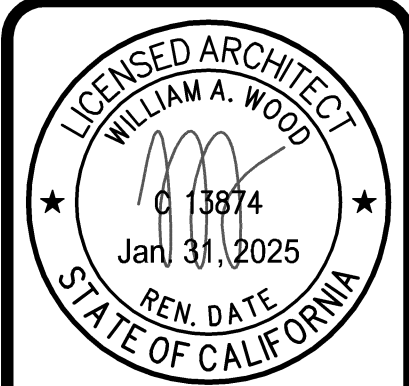


DANVILLE ACCESSORY
DWELLING UNITS
(ONE BEDROOM)
(CONTEMPORARY STYLE)

[illegible][illegible]

DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

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ARCHITECTS**
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JOB NO. 20.921R
SHEET CS1
OF SHEETS

GENERAL NOTES

2022 CALIFORNIA BUILDING CODE
2022 CALIFORNIA RESIDENTIAL CODE
2022 CALIFORNIA GREEN BUILDING STANDARDS CODE
2022 CALIFORNIA MECHANICAL CODE
2022 CALIFORNIA ELECTRICAL CODE
2022 CALIFORNIA PLUMBING CODE
2022 CALIFORNIA FIRE CODE
2022 CALIFORNIA ENERGY CODE
TOWN OF DANVILLE MUNICIPAL CODE

1. All materials, workmanship and methods of construction shall conform to the requirements of the 2022 codes listed above. The Contractor is required to incorporate whatever changes or additional work these codes or regulations require without additional cost to the Owner.
2. Do not scale drawings. Written dimensions shall always take precedence over scale dimensions. Dimensions shown on floor plans are to face of stud unless otherwise noted or indicated.
3. Notify the Architect of any discrepancies in the Documents or between the Documents and the existing conditions before proceeding with any work.
4. The Contractor shall obtain all required inspections for his work and give the Owner timely notice of his intent to have inspections.
5. The Contractor and/or Subcontractor shall at all times keep the premises free of accumulations of waste materials or rubbish caused by his employees or work. At the completion of the work, he shall remove all his rubbish, all of his tools, scaffolding, and surplus materials from and about the building and shall leave his work "broom clean" or its equivalent.
6. Contractor to coordinate and verify all sizes of kitchen appliances, to be selected by the Owner, and the style of the design, materials, hardware and finish of the cabinets with the Owner and cabinet-maker prior to manufacturing.
7. The escape opening has a minimum net clear opening of 5.7 square feet; minimum net clear opening height of 24 inches; and minimum net clear opening width of 20 inches. Maximum sill height to be 44" where windows are provided as means of escape or rescue.
8. All ICC evaluation reports and manufacturer specifications for fireplaces and skylights to be provided by Contractor and approved by the TOWN OF DANVILLE prior to plan installation.
9. Contractor to verify size and clearances of selected water heater and furnace for adequacy of space shown on the drawings prior to construction. Notify Architect immediately of any conflicts with space requirements for this equipment.
11. All shower and tub/shower enclosures to be provided with smooth, hard, nonabsorbent finish backing (e.g., tile over W.R. Board to 70" above drain).
12. Handrails & Guardrails: Stairways to have handrails on open side of stairs, not less than 34" nor more than 38" above the nosing treads. They should be continuous the full length of the stairs and need not extend 12" beyond top and 12" plus tread width beyond bottom tread. Ends shall be returned or shall terminate in newel posts or safety terminals. Handgrip shall not be less than $1\frac{1}{4}$ " or more than 2" in cross-sectional dimension w/ no sharp corners, height of 34 inches to 38 inches above nosing, extend continuously from top to bottom risers, and terminate at newel posts or return to walls; handgrip shall have a space not less than $1\frac{1}{2}$ " out from the wall. Open guardrail and stair railings shall have intermediate rails or an ornamental pattern such that a sphere 4" in diameter cannot pass through. All interior and exterior handrails and guardrails to conform to 2022 CRC. Guardrails shall be designed to withstand a 200-pound load applied in any direction at any point along the top rail per 2022 CRC.
13. Maximum 3.5-inch handrail projection into the required stair width. Stringers and other projections such as trim are limited to $1 - \frac{1}{2}$ inches on each side.
14. Handrail(s) is required for stairways with four or more risers.
16. Batt insulation shall have maximum flame spread of 25 and smoke development of 450. .
17. The Contractor will read and conform to the Title 24 requirements for this project. A copy of Title 24 documents can be obtained from the Owner.
18. Glass & glazing to comply with 2022 CRC.
19. Discrepancies: In the event of conflict or discrepancy in contract documents, larger quantity and higher quality shall govern unless Architect approval is given in writing.

Site & Grading Notes

1. The site plan drawing is not a property survey. All information shown in regard to boundaries of property was taken from the County Assessor's maps. The Architect does not accept responsibility for the accuracy of the data presented and drawn. The Contractor is to hire and pay for a licensed surveyor to locate the property line adjacent to the addition prior to the start of construction.
2. Non-removable backflow prevention to be provided at meter and provide non-removable vacuum breaker on all hose bibbs.
3. All utilities to be field verified by contractor. William Wood Architects assume no responsibility for accuracy of utility locations, as shown on plans.
4. Restore all existing building and site improvements, including sidewalks, curb and gutters, walks, patios, fences, landscaping, sprinklers, plumbing, etc., which have been altered or damaged by reason of the contractor's operations to a new condition, to the complete satisfaction of the owner.
5. Work to comply with TOWN OF DANVILLE ordinance for setbacks and grading requirements as shown on the site plan.
6. All downspouts, site area drains and catch basins as shown on site plan shall be collected with PVC tight line rigid wall, independent of all other drainage systems.
7. All retaining walls shall have drainage with 4" perforated PVC rigid wall and collected with PVC Tight line rigid wall.
8. A positive gradient shall be provided away from the foundation in order to provide rapid removal of the surface water runoff away from the foundation to an adequate discharge point. The grade shall fall a minimum of 6" within the first 10' measured perpendicular to the face of the foundation wall per CRC R401.3.

General Construction Notes:

1. Gypsum drywall systems with fire-resistance ratings where indicated or required are to comply with governing regulations. Provide materials and installations identical with applicable assemblies that have been tested and listed by recognized authorities, including UL. Comply with FM approval Guide where applicable.

Foundation Notes:

See structural drawing for foundation notes.

Framing Notes:

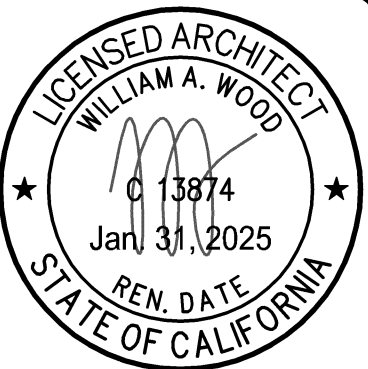
See Structural Drawings for framing notes:

Plumbing Notes

1. Showerheads shall be flow control of 1.8 G.P.M. or less.
2. All lavatory faucets shall be flow control of 1.2 G.P.M. or less.
3. Kitchen faucet to be 1.8 gallons per minute, maximum.
4. Water closet to be 1.28 gallon per flush maximum. or dual flush per CPC 411.2.
5. All piping and equipment shall be supported as per section 313 in the CPC.
6. Seismic restraint requirements for all piping and equipment (including gas lines and fuel fired equipment) shall be installed per CBC 1613.1 and Chapter 13 of ASCE7-16.
6. All backflow prevention devices shall be installed per CPC 603.3.
7. All potable and non-potable (closed loop fire sprinkler system) water piping is to be clearly identified as per the requirements of CPC 601.2.
8. Hose bibs shall be protected with a nonremovable hose-bib0-type backflow preventer, a non removable hose-bib-type vacuum breaker or by an atmospheric vacuum breaker installed not less than 6 inches above the highest point of usage located on the discharge side of the last valve. CPC 603.5.7.

Electrical and Mechanical Notes:

3. Electrical receptacles in bathrooms, outdoors and within 6'-0" of sink to be on ground fault protection (G.F.I.);
2. Provide 1/2-switched outlet under counter at sink for disposal.
4. Switch and duplex outlet, or multiple switches shown adjacent to each other on plan are to be under one plate.
5. New 110v smoke detector(s), with battery backup, and interconnected, which are audible in all sleeping area will be provided at the following locations:
(1) all new bedrooms; (2) in hallways and immediately adjacent to new bedrooms.
6. All appliances shall conform to the standards set forth by the California Energy Commission Title 24 requirements.
7. Gas fired cooking appliances and space-heating equipment shall have intermittent ignition devices.
8. All air ducts penetrating separation wall or ceiling between garage and living area shall be 26 GA. The ducts, plenums, A/C coil boxes must be insulated to R4 minimum and be of 26 ga. sheetmetal in the garage.
9. Contractor to verify space requirements for plenums and ducts prior to start of work.
10. Mechanical ventilation shall be capable of providing five air changes per hour in lieu of the natural ventilation in bathrooms, water closet compartments and laundry rooms and vented directly to the outside. All exhaust fans and fan systems to have damper controls.
11. Gaskets to be provided at exterior receptacles and switch plates.
12. Final locations of all mechanical and electrical equipment, panel boards, meters, fixtures, flues, vents, switches etc., shall be approved by Owner prior to installation.
13. Lights: All lights shall be high efficiency.
14. HVAC equipment, water heaters, showerheads and faucets to be certified by the CEC.
15. Provide a setback thermostat on all applicable heating systems.
16. Ducts penetrating the one-hour separation between garage and the house shall be not less than 26 gauge galvanized steel.
17. The HVAC equipment shown meets the assumed efficiency requirements of this compliance, however, it is recommended that the actual system to be installed be determined by an HVAC designer. The loads shown are only one of the criteria affecting the selection of HVAC equipment. Other relevant design factors such as airflow requirements, outdoor design temperatures, coil sizing, availability of equipment, oversizing safety margin, etc., must also be considered. It is the HVAC designer's responsibility to consider all factors when selecting the HVAC equipment.
18. Fused disconnect at A/C compressor unit or approved circuit breaker to be installed.
19. Clothes dryer, vented range hoods, and bathroom vent fans must be exhausted to outside per 2022 CMC.
20. Install new receptacle outlets as specified in NEC.
21. Provide a dedicated 20-amp branch circuit to supply the laundry receptacle outlet.
22. Contractor to provide additional electrical load as necessary to equip all new & existing elect. equipment.

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DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

WILLIAM WOOD
ARCHITECTS

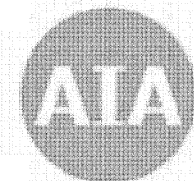
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SCALE AS SHOWN
JOB NO. 20.921R
SHEET GN1
OF SHEETS

Apr 26, 2023 - 10:36am GNI Gen Notes.dwg



California

2022 CALIFORNIA GREEN BUILDING STANDARDS CODE

RESIDENTIAL MANDATORY MEASURES, SHEET 1 (January 2023)

Y	N/A	RESPON. PARTY	CHAPTER 3 GREEN BUILDING SECTION 301 GENERAL	Y	N/A	RESPON. PARTY	CHAPTER 3 GREEN BUILDING SECTION 301 GENERAL	Y	N/A	RESPON. PARTY	CHAPTER 3 GREEN BUILDING SECTION 301 GENERAL	Y	N/A	RESPON. PARTY	CHAPTER 3 GREEN BUILDING SECTION 301 GENERAL																
			301.1 SCOPE. Buildings shall be designed to include the green building measures specified as mandatory in the application checklists contained in this code. Voluntary green building measures are also included in the application checklists and may be included in the design and construction of structures covered by this code, but are not required unless adopted by a city, county, or city and county as specified in Section 101.7.				4.106.4.2.1 New multifamily dwellings, hotels and motels and new residential parking facilities. When parking is provided, parking spaces for new multifamily dwellings, hotels and motels shall meet the requirements of Sections 4.106.4.2.1 and 4.106.4.2.2. Calculations for spaces shall be rounded up to the nearest whole number. A parking space served by electric vehicle supply equipment or designed as a future EV charging space shall count as at least one standard automobile parking space only for the purpose of complying with any applicable minimum parking space requirements established by a local jurisdiction. See Vehicle Code Section 22511.2 for further details.				4.106.4.2.1 Identification. The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code.				4.304 OUTDOOR WATER USE 4.304.1 OUTDOOR POTABLE WATER USE IN LANDSCAPE AREAS. Residential developments shall comply with a local water efficient landscape ordinance or the current California Department of Water Resources Model Water Efficient Landscape Ordinance (MWELO), whichever is more stringent.																
			301.1.1 Additions and alterations. [HCD] The mandatory provisions of Chapter 4 shall be applied to additions or alterations of existing residential buildings where the addition or alteration increases the building's conditioned area, volume, or size. The requirements shall apply only to and/or within the specific area of the addition or alteration. The mandatory provision of Section 4.106.4.2 may apply to additions or alterations of existing parking facilities or the addition of new parking facilities serving existing multifamily buildings. See Section 4.106.4.3 for application. Note: Repairs including, but not limited to, resurfacing, restriping and repairing or maintaining existing lighting fixtures are not considered alterations for the purpose of this section. Note: On and after January 1, 2014, residential buildings undergoing permitted alterations, additions, or improvements shall replace noncompliant plumbing fixtures with water-conserving plumbing fixtures. Plumbing fixture replacement is required prior to issuance of a certificate of final completion, certificate of occupancy or final permit approval by the local building department. See Civil Code Section 1101.1, et seq., for the definition of a noncompliant plumbing fixture, types of residential buildings affected and other important enactment dates.				4.106.4.2.2 Multifamily development projects with less than 20 dwelling units; and hotels and motels with less than 20 sleeping units or guest rooms. The number of dwelling units, sleeping units or guest rooms shall be based on all buildings on a project site subject to this section. 1.EV Capable. Ten (10) percent of the total number of parking spaces on a building site, provided for all types of parking facilities, shall be electric vehicle charging spaces (EV spaces) capable of supporting future Level 2 EVSE. Electrical load calculations shall demonstrate that the electrical panel service capacity and electrical system, including any on-site distribution transformer(s), have sufficient capacity to simultaneously charge all EVs at all required EV spaces at a minimum of 40 amperes. The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code. Exceptions: 1. When EV chargers (Level 2 EVSE) are installed in a number equal to or greater than the required number of EV capable spaces 2. When EV chargers (Level 2 EVSE) are installed in a number less than the required number of EV capable spaces, the number of EV capable spaces required may be reduced by a number equal to the number of EV chargers installed. Notes: a. Construction documents are intended to demonstrate the project's capability and capacity for facilitating future EV charging. b. There is no requirement for EV spaces to be constructed or available until receptacles for EV charging or EV chargers are installed for use. 2.EV Ready. Twenty-five (25) percent of the total number of parking spaces shall be equipped with low power Level 2 EV charging receptacles. For multifamily parking facilities, no more than one receptacle is required per dwelling unit when more than one parking space is provided for use by a single dwelling unit. Exception: Areas of parking facilities served by parking lifts.				4.303 INDOOR WATER USE 4.303.1 WATER CONSERVING PLUMBING FIXTURES AND FITTINGS. Plumbing fixtures (water closets and urinals) and fittings (faucets and showerheads) shall comply with the sections 4.303.1.1, 4.303.1.2, 4.303.1.3, and 4.303.4.4. Note: All noncompliant plumbing fixtures in any residential real property shall be replaced with water-conserving plumbing fixtures. Plumbing fixture replacement is required prior to issuance of a certificate of final completion, certificate of occupancy, or final permit approval by the local building department. See Civil Code Section 1101.1, et seq., for the definition of a noncompliant plumbing fixture, types of residential buildings affected and other important enactment dates.				4.304.2 CONSTRUCTION WASTE MANAGEMENT. Recycle and/or salvage for reuse a minimum of 65 percent of the non-hazardous construction and demolition waste in accordance with either Section 4.408.2, 4.408.3 or 4.408.4, or meet a more stringent local construction and demolition waste management ordinance. Exceptions: 1. Excavated soil and land-clearing debris. 2. Alternate waste reduction methods developed by working with local agencies if diversion or recycle facilities capable of compliance with this item do not exist or are not located reasonably close to the jobsite. 3. The enforcing agency may make exceptions to the requirements of this section when isolated jobsites are located in areas beyond the haul boundaries of the diversion facility.																
			301.2 LOW-RISE AND HIGH-RISE RESIDENTIAL BUILDINGS. [HCD] The provisions of individual sections of CALGreen may apply to either low-rise residential buildings high-rise residential buildings, or both. Individual sections will be designated by banners to indicate where the section applies specifically to low-rise only (LR) or high-rise only (HR). When the section applies to both low-rise and high-rise buildings, no banner will be used.				4.106.4.2.2 Multifamily development projects with 20 or more dwelling units, hotels and motels with 20 or more sleeping units or guest rooms. The number of dwelling units, sleeping units or guest rooms shall be based on all buildings on a project site subject to this section. 1.EV Capable. Ten (10) percent of the total number of parking spaces on a building site, provided for all types of parking facilities, shall be electric vehicle charging spaces (EV spaces) capable of supporting future Level 2 EVSE. Electrical load calculations shall demonstrate that the electrical panel service capacity and electrical system, including any on-site distribution transformer(s), have sufficient capacity to simultaneously charge all EVs at all required EV spaces at a minimum of 40 amperes. The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging purposes as "EV CAPABLE" in accordance with the California Electrical Code. Exception: When EV chargers (Level 2 EVSE) are installed in a number greater than five (5) percent of parking spaces required by Section 4.106.4.2.2, Item 3, the number of EV capable spaces required may be reduced by a number equal to the number of EV chargers installed over the five (5) percent required. Notes: a. Construction documents shall show locations of future EV spaces. b. There is no requirement for EV spaces to be constructed or available until receptacles for EV charging or EV chargers are installed for use. 2.EV Ready. Twenty-five (25) percent of the total number of parking spaces shall be equipped with low power Level 2 EV charging receptacles. For multifamily parking facilities, no more than one receptacle is required per dwelling unit when more than one parking space is provided for use by a single dwelling unit. Exception: Areas of parking facilities served by parking lifts. 3.EV Chargers. Five (5) percent of the total number of parking spaces shall be equipped with Level 2 EVSE. Where common use parking is provided, at least one EV charger shall be located in the common use parking area and shall be available for use by all residents or guests. When low power Level 2 EV charging receptacles or Level 2 EVSE are installed beyond the minimum required, an automatic load management system (ALMS) may be used to reduce the maximum required electrical capacity to space served by the ALMS. The electrical system and any on-site distribution transformers shall have sufficient capacity to deliver at least 3.3 kW simultaneously to each EV charging station (EVCS) served by the ALMS. The branch circuit shall have a minimum capacity of 40 amperes, and installed EVSE shall have a capacity of not less than 30 amperes. ALMS shall not be used to reduce the minimum required electrical capacity to the required EV capable spaces.				4.303.1.3.1 Single Showerhead. Showerheads shall have a maximum flow rate of not more than 1.8 gallons per minute at 80 psi. Showerheads shall be certified to the performance criteria of the U.S. EPA WaterSense Specification for Showerheads. 4.303.1.3.2 Multiple showerheads serving one shower. When a shower is served by more than one showerhead, the combined flow rate of all the showerheads and/or other shower outlets controlled by a single valve shall not exceed 1.8 gallons per minute at 80 psi, or the shower shall be designed to only allow one shower outlet to be in operation at a time. Note: A hand-held shower shall be considered a showerhead. 4.303.1.4 Faucets. 4.303.1.4.1 Residential Lavatory Faucets. The maximum flow rate of residential lavatory faucets shall not exceed 1.2 gallons per minute at 60 psi. The minimum flow rate of residential lavatory faucets shall not be less than 0.8 gallons per minute at 20 psi. 4.303.1.4.2 Lavatory Faucets in Common and Public Use Areas. The maximum flow rate of lavatory faucets installed in common and public use areas (outside of dwellings or sleeping units) in residential buildings shall not exceed 0.5 gallons per minute at 60 psi. 4.303.1.4.3 Metering Faucets. Metering faucets when installed in residential buildings shall not deliver more than 0.2 gallons per cycle. 4.303.1.4.4 Kitchen Faucets. The maximum flow rate of kitchen faucets shall not exceed 1.8 gallons per minute at 60 psi. Kitchen faucets may temporarily increase the flow above the maximum rate, but not to exceed 2.2 gallons per minute at 60 psi, and must default to a maximum flow rate of 1.8 gallons per minute at 60 psi. Note: Where complying faucets are unavailable, aerators or other means may be used to achieve reduction. 4.303.1.4.5 Pre-rinse spray valves. When installed, shall meet the requirements in the California Code of Regulations, Title 20 (Appliance Efficiency Regulations), Sections 1605.1 (h)(4) Table H-2, Section 1605.3 (h)(4)(A), and Section 1607 (d)(7) and shall be equipped with an integral automatic shutoff. FOR REFERENCE ONLY: The following table and code section have been reprinted from the California Code of Regulations, Title 20 (Appliance Efficiency Regulations), Section 1605.1 (h)(4) and Section 1605.3 (h)(4)(A).				4.303.1.4.5 Pre-rinse spray valves. When installed, shall meet the requirements in the California Code of Regulations, Title 20 (Appliance Efficiency Regulations), Sections 1605.1 (h)(4) Table H-2, Section 1605.3 (h)(4)(A), and Section 1607 (d)(7) and shall be equipped with an integral automatic shutoff. FOR REFERENCE ONLY: The following table and code section have been reprinted from the California Code of Regulations, Title 20 (Appliance Efficiency Regulations), Section 1605.1 (h)(4) and Section 1605.3 (h)(4)(A).																
			SECTION 4.102 DEFINITIONS 4.102.1 DEFINITIONS The following terms are defined in Chapter 2 (and are included here for reference) FRENCH DRAIN. A trench, hole or other depressed area loosely filled with rock, gravel, fragments of brick or similar pervious material used to collect or channel drainage or runoff water. WATTLES. Wattles are used to reduce sediment in runoff. Wattles are often constructed of natural plant materials such as hay, straw or similar material shaped in the form of tubes and placed on a downflow slope. Wattles are also used for perimeter and inlet controls.				4.106.4.2.2.1 Location. EVCS shall comply with at least one of the following options: 1. The charging space shall be located adjacent to an accessible parking space meeting the requirements of the California Building Code, Chapter 11A, to allow use of the EV charger from the accessible parking space, Chapter 2, to the building. 2. The charging space shall be located on an accessible route, as defined in the California Building Code, Chapter 11B, are not required to comply with Section 4.106.4.2.2.1.1 and Section 4.106.4.2.2.1.2, Item 3. 4.106.4.2.2.1.1 Accessible EV spaces. In addition to the requirements in Sections 4.106.4.2.2.1.1 and 4.106.4.2.2.1.2, all EVSE, when installed, shall comply with the accessibility provisions for EV chargers in the California Building Code, Chapter 11B. EV ready spaces and EVCS in multifamily developments shall comply with California Building Code, Chapter 11A, Section 1109A. 4.106.4.2.3 EV space requirements. 1. Single EV space required. Install a listed raceway capable of accommodating a 208/240-volt dedicated branch circuit. The raceway shall not be less than trade size 1 (nominal 1-inch inside diameter). The raceway shall originate at the main service or subpanel and shall terminate into a listed cabinet, box or enclosure in close proximity to the proposed location of an EV charger. Raceways are required to be continuous at enclosed, inaccessible or concealed areas and spaces. The service panel and/or subpanel shall provide capacity to install a 40-ampere 208/240-volt minimum dedicated branch circuit and space(s) reserved to permit installation of a branch circuit overcurrent protective device. Exception: A raceway is not required if a minimum 40-ampere 208/240-volt dedicated EV branch circuit is installed in close proximity to the location or the proposed location of the EV space, at the time of original construction in accordance with the California Electrical Code. 4.106.4.1.1 Identification. The service panel or subpanel circuit directory shall identify the overcurrent protective device space(s) reserved for future EV charging as "EV CAPABLE". The raceway termination location shall be permanently and visibly marked as "EV CAPABLE".				4.303.2 Submitters for multifamily buildings and dwelling units in mixed-used residential/commercial buildings. Submitters shall be installed to measure water usage of individual rental dwelling units in accordance with the California Plumbing Code. 4.303.3 Standards for plumbing fixtures and fittings. Plumbing fixtures and fittings shall be installed in accordance with the California Plumbing Code, and shall meet the applicable standards referenced in Table 1701.1 of the California Plumbing Code. NOTE: THIS TABLE COMPILES THE DATA IN SECTION 4.303.1, AND IS INCLUDED AS A CONVENIENCE FOR THE USER. <table><tr><th>FIXTURE TYPE</th><th>FLOW RATE</th></tr><tr><td>SHOWER HEADS (RESIDENTIAL)</td><td>1.8 GPM @ 80 PSI</td></tr><tr><td>LAVATORY FAUCETS (RESIDENTIAL)</td><td>MAX. 1.2 GPM @ 60 PSI MIN. 0.8 GPM @ 20 PSI</td></tr><tr><td>LAVATORY FAUCETS IN COMMON & PUBLIC USE AREAS</td><td>0.5 GPM @ 60 PSI</td></tr><tr><td>KITCHEN FAUCETS</td><td>1.8 GPM @ 60 PSI</td></tr><tr><td>METERING FAUCETS</td><td>0.2 GAL/CYCLE</td></tr><tr><td>WATER CLOSET</td><td>1.28 GAL/FLUSH</td></tr><tr><td>URINALS</td><td>0.125 GAL/FLUSH</td></tr></table>	FIXTURE TYPE	FLOW RATE	SHOWER HEADS (RESIDENTIAL)	1.8 GPM @ 80 PSI	LAVATORY FAUCETS (RESIDENTIAL)	MAX. 1.2 GPM @ 60 PSI MIN. 0.8 GPM @ 20 PSI	LAVATORY FAUCETS IN COMMON & PUBLIC USE AREAS	0.5 GPM @ 60 PSI	KITCHEN FAUCETS	1.8 GPM @ 60 PSI	METERING FAUCETS	0.2 GAL/CYCLE	WATER CLOSET	1.28 GAL/FLUSH	URINALS	0.125 GAL/FLUSH				4.408.3 WASTE MANAGEMENT COMPANY. Utilize a waste management company, approved by the enforcing agency, which can provide verifiable documentation that percentage of construction and demolition waste material diverted from the landfill complies with Section 4.408.1. Note: The owner or contractor may make the determination if the construction and demolition waste materials will be diverted by a waste management company. 4.408.4 WASTE STREAM REDUCTION ALTERNATIVE [LR]. Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 3.4 lbs./sq. ft. of the building area shall meet the minimum 65% construction waste reduction requirement in Section 4.408.1. 4.408.4.1 WASTE STREAM REDUCTION ALTERNATIVE. Projects that generate a total combined weight of construction and demolition waste disposed of in landfills, which do not exceed 2 pounds per square foot of the building area, shall meet the minimum 65% construction waste reduction requirement in Section 4.408.1. 4.408.5 DOCUMENTATION. Documentation shall be provided to the enforcing agency which demonstrates compliance with Section 4.408.2, Items 1 through 5, Section 4.408.3 or Section 4.408.4. Notes: 1. Sample forms found in "A Guide to the California Green Building Standards Code (Residential)" located at www.hcd.ca.gov/CALGreen.html may be used to assist in documenting compliance with this section. 2. Mixed construction and demolition debris (C & D) processors can be located at the California Department of Resources Recycling and Recovery (CalRecycle). 4.410 BUILDING MAINTENANCE AND OPERATION 4.410.1 OPERATION AND MAINTENANCE MANUAL. At the time of final inspection, a manual, compact disc, web-based reference or other media acceptable to the enforcing agency which includes all of the following shall be placed in the building: 1. Directions to the owner or occupant that the manual shall remain with the building throughout the life cycle of the structure. 2. Operation and maintenance instructions for the following: a. Equipment and appliances, including water-saving devices and systems, HVAC systems, photovoltaic systems, electric vehicle chargers, water-heating systems and other major appliances and equipment. b. Roof and yard drainage, including gutters and downspouts. c. Space conditioning systems, including condensers and air filters. d. Landscape irrigation systems. e. Water reuse systems. 3. Information from local utility water and waste recovery providers on methods to further reduce resource consumption, including recycle programs and locations. 4. Public transportation and/or carpool options available in the area. 5. Educational material on the positive impacts of an interior relative humidity between 30-60 percent and what methods an occupant may use to maintain the relative humidity level in that range. 6. Information about water-conserving landscape and irrigation design and controllers which conserve water. 7. Instructions for maintaining gutters and downspouts and the importance of diverting water at least 5 feet away from the foundation. 8. Information on required routine maintenance measures, including, but not limited to, caulking, painting, grading around the building, etc. 9. Information about state solar energy and incentive programs available. 10. A copy of all special inspectors' verifications required by the enforcing agency or this code. 11. Information from the Department of Forestry and Fire Protection on maintenance of defensible space around residential structures. 12. 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[illegible]

Professional Engineer Seal for William A. Wood, State of California, License No. C 13874, expires Jan 31, 2025.

DANVILLE CA 94526

LE, CALIFORNIA
(925) 820-8233

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GN3

OF SHEETS

BROAN

BROAN ERV100
Part no. **ERV100S**
Low speed: 50 CFM (0.2 in. w.g.), selectable 65 CFM
High speed: 100 CFM (0.4 in. w.g.), selectable 85 CFM



CONSTRUCTION

- 22 ga. galvanized steel housing and door
- One-piece molded insulation shell, expanded polystyrene (UL 94 HF-1 certified)
- Galvanized steel 5" diameter ports*
- Steel door hinges and latches
- Cold side ports with plastic ring; allow vapor barrier sealing
- No drain required
- Included installation brackets

*All unit ports were created to be connected to ducts having a minimum of 5" diameter, but if need be, they can be connected to bigger sized ducts by using an appropriate transition (e.g. 5" diameter to 6" diameter transition).

MOTORS

- Two high reliability, external rotor PSC motors, totally enclosed and thermally protected
- Backward blowers, can support high static pressure environment

FILTERS

- Two washable 20 gpi reticulated polyester urethane foam filters, UL 900 class 2 certified
- Optional MERV 8 filter kit, part no. V21030 (additional 0.1 in. w.g. static pressure to be considered) (sold separately)

ERV CORE

- Crossflow aluminum-polymerized paper core, UL723 certified
- Hydroscope polymer exchange water by direct vapor transfer using molecular transport without the need of condensation
- Constructed of alternate layers of corrugated aluminum material and polymeric bactericide desiccant impregnated media; will not promote growth of mold or bacteria
- Unique rectangular flute design to provide very low pressure drop values

ENERGY RECOVERY VENTILATOR

Controls

- Integrated push-button control for Low or High speed operation.
- For a complete list of optional main and auxiliary controls available, refer to the **Wall Control Compatibility Chart** on last pages of wall controls specification sheet, available at www.broan.com.
- For more details about controls, refer to the **Main and auxiliary wall controls** user guide, also available at www.broan.com.
- Removable terminal block included for quicker low voltage control connections
- LED indicator shows operating modes and error codes.
- Unit must be permanently energized, no control should be installed on power supply of unit.

Air Flow

- Pressure taps and balancing chart to allow easier balancing of the unit
- Integrated balancing/backdraft dampers in cold supply and cold exhaust ports

Defrost Cycles

- Choice of regular or extended defrost cycles, according to climatic conditions
- To set extended defrost cycles, refer to unit installation manual

Regular Defrost Cycles			
Outdoor Temperature*		Defrost in Minutes / Air Exchange in Minutes	
°C	°F	Low Speed	High Speed
Warmer than -10	Warmer than 14	No DEFROST	No DEFROST
FROM -10 to -15	FROM 14 to 5	6/40	9/40
FROM -15 to -27	FROM 5 to -17	6/20	9/20
-27 AND LESS	-17 AND LESS	8/12	11/12

* Outdoor temperature is read by a thermostat located inside the unit, next to fresh air from outdoor port.

Warranty

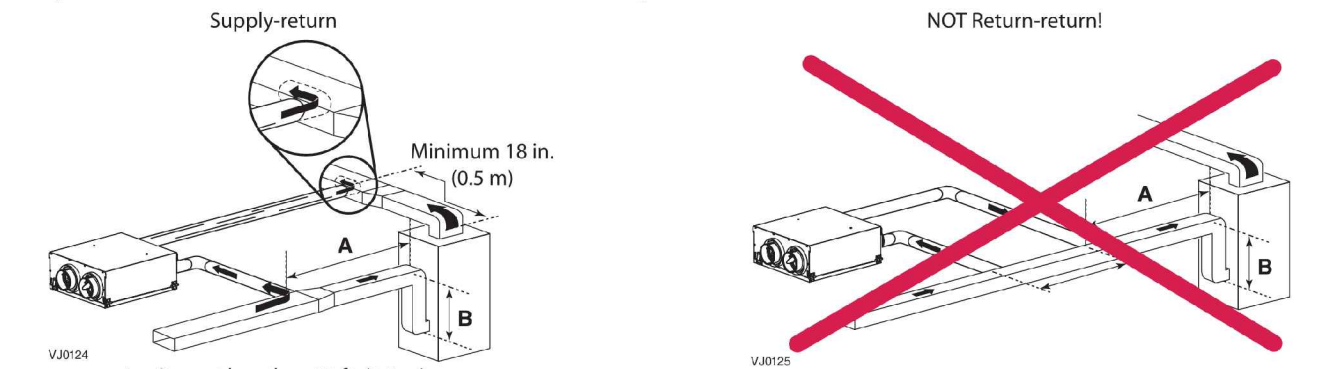
The Broan ERV100 is protected by a 5-year warranty on parts only, with the original proof of purchase.

Requirements and standards

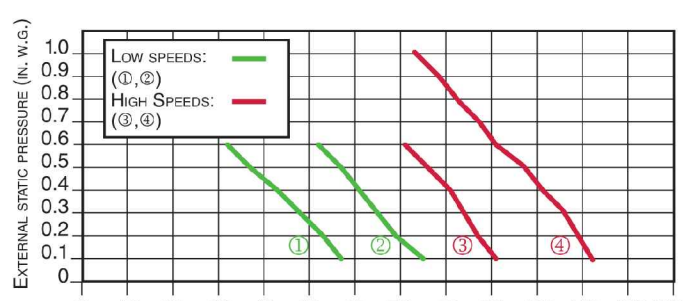
• HVI certified

- Complies with the UL 1812 requirements regulating the installation of Energy Recovery Ventilators
- Complies with the CSA C22.2 no. 113 Standard applicable to ventilators
- Complies with CSA C444 requirements regulating the installation of Energy Recovery Ventilators

Simplified Installation (connection to a forced air system)



VENTILATION PERFORMANCE



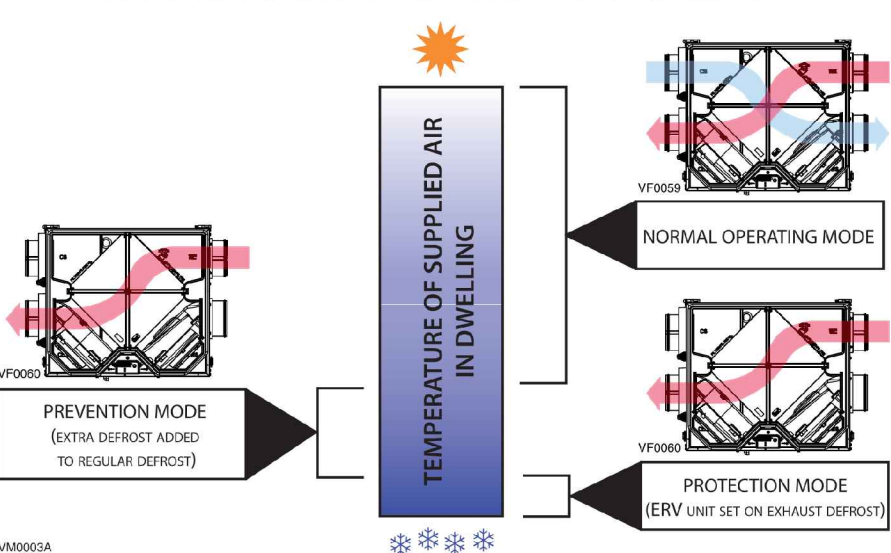
NOTE: Low speed 0.2 in. w.g. and high speed 0.4 in. w.g. are selectable.

Acoustic Noise Power Chart (dBA)

Airflow	Fresh air to building port	Exhaust air from building port
100 CFM at 0.4 in. w.g.	67.8 dBA	58.3 dBA
55 CFM at 0.1 in. w.g.	58.0 dBA	49.4 dBA

Coldshield™ Protection

ERV UNIT MODES BASED ON AIR DISTRIBUTION TEMPERATURE



Professional Prestige® ProTerra™ Hybrid Electric with LeakGuard™ is the most efficient water heater available

Efficiency

- Up to 4.0 UEF reduces operating cost
- ENERGY STAR® rated

Performance

- Delivers hot water faster than most standard electric water heaters
- Ambient operating range: 37°-145° F is widest in class, offering more days of HP operation annually; designed to meet Northern Climate Spec (Tier 4)

Easy Installation

- Easy access side connections
- Quick access to electrical junction box
- Easily replaces a standard electric water heater

Integration

- LED Screen with built-in water sensor alert with audible alarm!
- Integrated EcoNet® WiFi-connected technology and free mobile app gives users control over water heater, allowing for customizable temperature, vacation settings, energy savings and system monitoring at home or away. Visit Rheem.com/hybridolutions

LEAKGUARD™ WATER SHUT-OFF VALVE

- Integrated leak detection and prevention system with factory installed auto water shut-off valve limits leaks to no more than 20 ounces of water! *

- LeakSense™ Built-in Leak Detection System detects any leak large or small, internal or external! *

Operation Modes

- Energy Saver
- Heat Pump
- High Demand
- Electric
- Vacation/Away: 2-28 days (or placed on hold indefinitely)

Plus...

- Premium grade anode rod with resistor extends the life of the tank
- 3/4" NPT water inlet and outlet; 3/4" condensate drain connections
- Incooly stainless steel resistor elements
- Dry-fire protection
- Easy access, top mounted washable air filter
- 2" Non-CFC foam insulation
- Enhanced flow brass drain valve
- Temperature and pressure relief valve installed
- Design certified to NSF/ANSI 372 (Lead Content)

Warranty

- 10-Year limited tank and parts warranty
- Includes Residential Warranty Certificate for complete information

- Units meet or exceed ANSI requirements and have been tested according to D.O.E. procedures. Units meet or exceed the energy efficiency requirements of NAECA, ASHRAE standard 90, ICC Code and all state energy efficiency performance criteria.

- Units meet or exceed ANSI requirements and have been tested according to D.O.E. procedures. Units meet or exceed the energy efficiency requirements of NAECA, ASHRAE standard 90, ICC Code and all state energy efficiency performance criteria.



Professional Prestige® ProTerra Hybrid
40, 50, 65 and 80-Gallon Capacities
208-240 Volt / 1 PH Electric



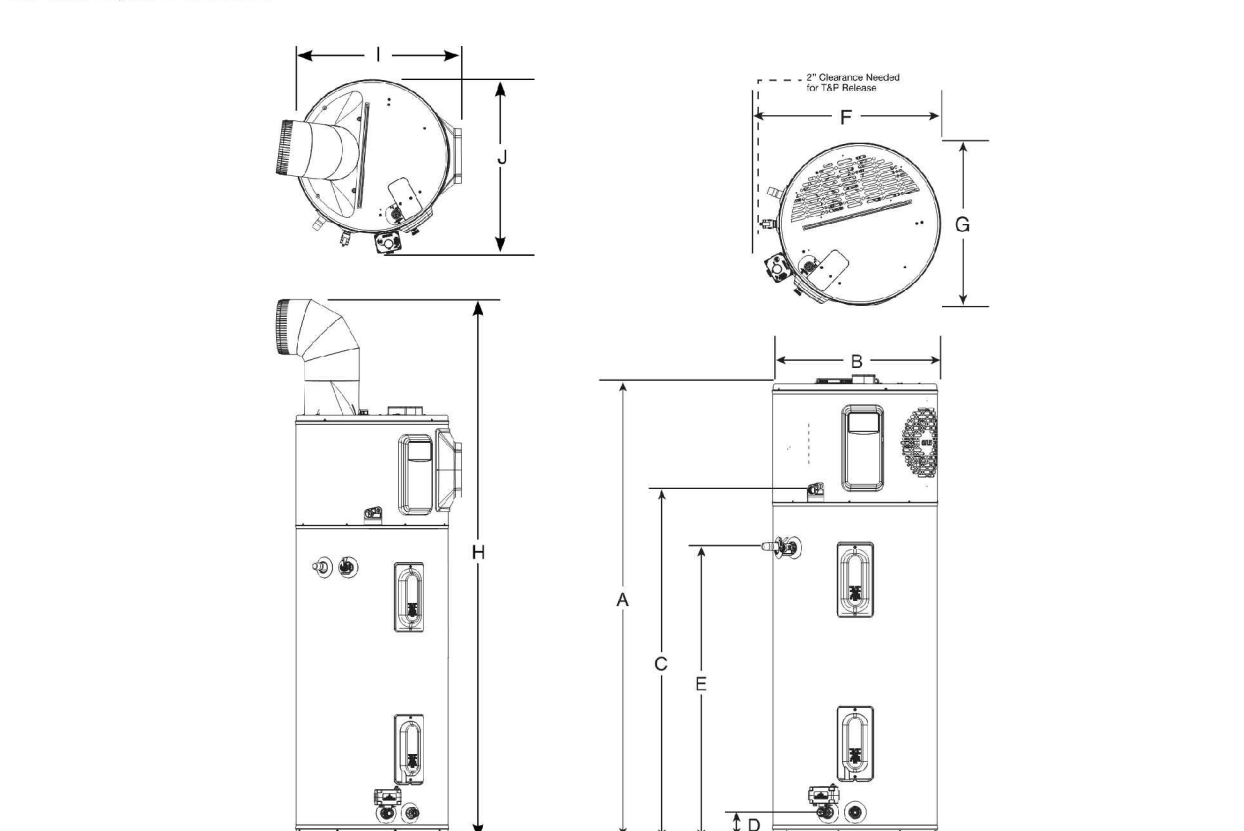
LEED Points = 3

See specifications chart on next page.

Professional Prestige® ProTerra Hybrid Specifications

DESCRIPTION			ENERGY INFO				FEATURES				SHIPPING WEIGHTS				
NOMINAL WATER CAPACITY	RATED GALLON CAPACITY	MODEL NUMBER	MODEL VARIANT	ELECTRIC RATING	PROFORM ENERGY CONSUMPTION	DEFROST CYCLES	DEFROST RATE	DEFROST RATE	DEFROST RATE	ELEMENT WEIGHT	TOTAL WEIGHT	MAX APPS	UNIT WEIGHT	APPROX. VOLUME	
ProTerra with LeakGuard™															
40	50	PROPH40 T2 RH-075-S0	700944	30	3.75	\$104	4,200	49	50	28	4,500	5,000	21	157	174
50	45	PROPH50 T2 RH-075-S0	700943	30	3.75	\$104	4,200	49	50	27	4,500	5,000	21	178	218
65	50	PROPH65 T2 RH-075-S0	700944	30	3.85	\$155	4,200	49	75	27	4,500	5,000	21	225	282
80	72	PROPH80 T2 RH-075-S0	700945	30	4.00	\$146	4,200	49	87	27	4,500	5,000	21	244	281
ProTerra 30 Amp without LeakGuard™															
40	50	PROPH40 T2 RH-075-S0	700947	30	3.75	\$104	4,200	49	50	27	4,500	5,000	21	157	174
50	45	PROPH50 T2 RH-075-S0	700947	30	3.75	\$104	4,200	49	50	27	4,500	5,000	21	178	218
65	50	PROPH65 T2 RH-075-S0	700948	30	3.85	\$155	4,200	49	75	27	4,500	5,000	21	225	282
80	72	PROPH80 T2 RH-075-S0	700949	30	4.00	\$146	4,200	49	87	27	4,500	5,000	21	244	281
ProTerra 15 Amp without LeakGuard™															
40	50	PROPH40 T2 RH-075-S15	700947	15	3.45	\$78	4,200	49	46	16	2,250	2,750	12	157	174
50	45	PROPH50 T2 RH-075-S15	700948	15	3.75	\$104	4,200	49	50	16	2,250	2,750	12	178	218
65	50	PROPH65 T2 RH-075-S15	700949	15	3.55	\$110	4,200	49	54	16	2,250	2,750	12	225	282
80	72	PROPH80 T2 RH-075-S15	700948	15	3.70	\$105	4,200	49	67	16	2,250	2,750	12	244	281

Estimated energy cost based on a national average electricity cost of \$0.12/kWh, Uniform Energy Factor and rated gallon capacity based on Department of Energy (DOE) requirements. All units have integrated Wi-Fi control option.



DESCRIPTION		DIMENSIONS (SHOWN IN INCHES)									
NOMINAL GALLON CAPACITY	MODEL NUMBER	A	B	C	D	E	F	G	H	I	J
40	PROPH40	62-5/16	20-1/4	47	3-5/8	39-5/8	23-3/8	20-1/2	18-7/8	23-3/8	23-1/4
50	PROPH50	61-3/4	22-1/4	47	3-5/8	39-5/8	25-3/8	22-1/2	18-5/8	24-3/8	25-9/16
65	PROPH65	64-3/16	24-1/4	48	3-7/8	42-3/8	27-1/2	24-5/8	21-1/8	25-1/2	27-3/8
80	PROPH80	74-3/16	24-1/4	50	3-7/8	42-3/8	27-1/2	24-5/8	21-1/8	25-1/2	27-3/8

2

Specifications are subject to change without notice.

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PEAD-A24AA7 & PUZ-A24NHA7-(BS)
MID STATIC HORIZONTAL-DUCTED INDOOR UNIT
24,000 BTU/H HEAT PUMP UNIVERSAL OUTDOOR

Job Name: _____ Date: _____
System Reference: _____

Indoor Unit: PEAD-A24AA7
Outdoor Unit: PUZ-A24NHA7
Standard Model: PUZ-A24NHA7
Seacoast Model: PUZ-A24NHA7-BS

INDOOR UNIT FEATURES

- Unobtrusive ceiling-concealed design for short-run ductwork
- Wide ranging external static pressure (0.14-0.60 in. WG)
- Built-in condensate lift mechanism (up to 27-9/16")
- Auto fan speed mode
- Optional FB Series filter boxes for easy access and service
- Ideal for residential homes, retail shopping centers, larger classrooms, office complexes, conference rooms, ballrooms, fitness centers, and more
- Multiple control options available:
 - Kurvo cloud® smart device app for remote access
 - Third-party interface options
 - Wired or wireless controllers

OUTDOOR UNIT FEATURES

- Variable speed INVERTER-driven compressor
- Power receiver pre-charged with refrigerant volume for piping length up to 70 ft
- Low ambient cooling down to 0°F providing 100% capacity
- 24-hour continuous operation (cooling mode)
- High pressure protection
- Fast restart
- Superior energy and operational efficiency

Specifications are subject to change without notice.

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SPECIFICATIONS: PEAD-A24AA7 & PUZ-A24NHA7-(BS)	
Cooling at 80°F	Maximum Capacity BTU/H 24,000
	Rated Capacity BTU/H 24,000
	Minimum Capacity BTU/H 10,000
	Maximum Power Input W 2,950
Heating at 47°F	Rated Power Input W 2,950
	Moisture Removal Pints/hr 6.9
	Sound Heat Factor dB(A) 6.8
	Power Factor % 92.892.8
Heating at 17°F	Maximum Capacity BTU/H 24,000
	Rated Capacity BTU/H 24,000
	Minimum Capacity BTU/H 10,000
	Maximum Power Input W 2,950
Heating at 0°F	Rated Power Input W 1,750
	Power Factor % 92.892.8
	Maximum Capacity BTU/H 12,200
	Rated Capacity BTU/H 14,800
Efficiency	Maximum Power Input W 1,700
	Rated Power Input W 1,630
	Maximum Capacity BTU/H 14,300
	Maximum Power Input W 1,600
Electrical	SEER 13.6
	EER 11.7
	HSPF (H) 10.8
	COP at 47°F 4.35
Indoor Unit	COP at 17°F at Maximum Capacity* 2.33
	COP at 0°F at Maximum Capacity* 2.33
	ENERGY STAR® Certified Voltage, Phase, Frequency 208/208 V, 60
	Guaranteed Voltage Range 198 - 253
Indoor Unit Operating Temperature Range	Voltage, Indoor - Outdoor, S1-S2 V AC 208/208
	Voltage, Indoor - Outdoor, S2-S3 V AC 24
	Short-circuit Current Rating (SCCR) kA 5
	Recommended Fuse/Breaker Size (Outdoor) A 14
Indoor Unit Operating Temperature Range	Power Supply A 240
	MCA A 2.1
	Fan Motor Full Load Amperage A 121
	Fan Motor Type DC Motor
Indoor Unit Operating Temperature Range	Airflow Rate at Cooling, Dry CFM 513-635-741
	Airflow Rate at Cooling, Moist CFM 445-572-667
	Airflow Rate at Heating, Dry CFM 513-635-741
	Sound Pressure Level (Cooling) dB(A) 30-33-37
Indoor Unit Operating Temperature Range	Sound Pressure Level (Heating) dB(A) 30-33-37
	External Static Pressure in. WG 0.14-0.24-0.26-0.4-0.6
	Drain Pipe Size in. (mm) 1-1/4 (32)
	Condensate Lift Mechanism, Maximum Distance in. (mm) 27-48 (700)
Indoor Unit Operating Temperature Range	Cooling on Heat Exchanger External Fresh Air Gal/min 4.5
	Unit Dimensions W x D x H, in. (mm) 43-5/16 x 28-7/8 x 9-7/8 (1100 x 732 x 250)
	Package Dimensions W x D x H, in. (mm) 53-3/16 x 34-1/16 x 13-7/16 (1355 x 880 x 341)
	Unit Weight Lbs. (kg) 69 (31)
Indoor Unit Operating Temperature Range	Package Weight Lbs. (kg) 69 (31)
	Cooling Intake Air Temp (Maximum / Minimum)* °F 80 DB, 67 WB (16 DB, 61 WB)
	Heating Intake Air Temp (Maximum / Minimum)* °F 77 DB (58 DB)
Indoor Unit Operating Temperature Range	Notes: AHRF Based Conditions (Rated data is determined at a fixed compressor speed)
	Cooling (Indoor / Outdoor) °F 80 DB, 67 WB (16 DB, 75 WB)
	Heating at 47°F (Indoor / Outdoor) °F 70 DB, 60 WB (17 DB, 44 WB)
	Heating at 17°F (Indoor / Outdoor) °F 70 DB, 60 WB (17 DB, 15 WB)
Indoor Unit Operating Temperature Range	Conditions: *Outdoor Unit Operating Temperature Range (Cooling Air Temp. Maximum / Minimum) • Wind baffles required to operate below 20°F DB cooling mode. • Heat pump system with wind baffles (PT, 110°F). • Refer to wind baffle documentation for further information.
	Heating at 0°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)
	Heating at 47°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)
	Heating at 17°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)
Indoor Unit Operating Temperature Range	*Outdoor Unit Operating Temperature Range (Cooling Air Temp. Maximum / Minimum) • Wind baffles required to operate below 20°F DB cooling mode. • Heat pump system with wind baffles (PT, 110°F). • Refer to wind baffle documentation for further information.
	Heating at 0°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)
	Heating at 47°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)
	Heating at 17°F (Indoor / Outdoor) °F 70 DB, 60 WB (15 DB, 4 WB)

Specifications are subject to change without notice.

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AHRI CERTIFIED®
www.ahrirectory.org

Certificate of Product Ratings

AHRI Certified Reference Number : 201754461 Date : 03-30-2020 Model Status : Active

AHRI Type : HRCU-A-CB (Mini-Split System): Heat Pump with Outdoor Unit-Air-Source, Ducted)

Series Name : P-Series

Outdoor Unit Brand Name : Mitsubishi Electric

Outdoor Unit Model Number : PUZ-A24NHA7***

Indoor Type : Mini-Splits (Ducted)

Indoor Model Number(s) : PEAD-A24AA*

Rated as follows in accordance with the latest edition of AHRI 210/240 - 2017 with Addendum 1, Performance Rating of Unitary Air-Conditioning & Air-Source Heat Pump Equipment and subject to rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (A2) - Single or High Stage (95F), btuh : 24000

SEER : 19.60

EER (A2) - Single or High Stage (95F) : 11.70

Heating Capacity (H12) - Single or High Stage (47F) : 26000

HSPF (Region IV) : 10.80

Rated as follows in accordance with the latest edition of AHRI 210/240 - 2020, Performance Rating of Unitary Air-Conditioning & Air-Source Heat Pump Equipment and subject to rating accuracy by AHRI-sponsored, independent, third party testing:

Cooling Capacity (A1a) - Single or High Stage (95F), btuh : 24000

SEER2 : 19.20

EER2 (A1a) - Single or High Stage (95F) : 12.00

Heating Capacity (H1a) - Single or High Stage (47F), btuh : 26000

HSPF2 (Region IV) : 9.10

Sold In : USA, Canada

*Active Model Status are those that an AHRI Certification Program Participant is currently producing AND selling or offering for sale; OR new models that are being marketed but are not yet being produced. "Production Stopped" Model Status are those that an AHRI Certification Program Participant is no longer producing BUT is still selling or offering for sale.

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GENERAL NOTES

1. ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE 2022 EDITION OF THE CALIFORNIA BUILDING CODE (CBC); THE MOST RECENT VERSIONS OF THE CMC, CPC AND CEC; ALL APPLICABLE LOCAL CODES AND ORDINANCES; AND LOCALLY ACCEPTED STANDARDS OF PRACTICE.
2. THESE DRAWINGS AND SPECIFICATIONS HAVE BEEN PREPARED EXCLUSIVELY FOR USE ON THIS PROJECT ONLY. THE DRAWINGS AND SPECIFICATIONS, OR PORTIONS THEREOF, SHALL NOT BE USED ON OTHER PROJECTS OR ADDITIONS TO THIS PROJECT EXCEPT BY AGREEMENT IN WRITING AND WITH APPROPRIATE COMPENSATION OF THE ENGINEER.
3. WRITTEN INFORMATION AND DIMENSIONS SHALL TAKE PRECEDENCE OVER GRAPHIC INFORMATION.
4. STRUCTURAL DRAWINGS SHOW ONLY THE BASIC STRUCTURAL SYSTEMS. REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND CIVIL DRAWINGS FOR ITEMS WHICH REQUIRE SPECIAL PROVISIONS DURING CONSTRUCTION.
5. SEE DRAWINGS OTHER THAN STRUCTURAL FOR: TYPES OF FINISH MATERIALS AND THEIR LOCATIONS, FOR DEPRESSIONS IN FLOOR SLABS, FOR OPENINGS IN WALLS AND FLOORS REQUIRED BY ARCHITECTURAL AND MECHANICAL FEATURES, FOR STAIRS, CURBS, ETC.
6. ALL DETAIL REFERENCES SHALL BE CONSIDERED "TYPICAL". THE INTENT OF TYPICAL DETAILS SHALL BE APPLIED TO SIMILAR CONDITIONS ELSEWHERE IN THE PROJECT. WHEN DETAILS LABELED "SIMILAR" ARE GIVEN ON DRAWINGS, THE CONTRACTOR SHALL APPLY THE GENERAL INTENT OF THE DETAIL TO THE REFERENCED CONDITION.
7. **THE CONTRACTOR SHALL REVIEW ALL DRAWINGS IMMEDIATELY UPON RECEIPT AND SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCIES.**
8. STRUCTURAL DRAWINGS AND SPECIFICATIONS FOR THIS WORK HAVE BEEN PREPARED IN ACCORDANCE WITH GENERALLY ACCEPTED ENGINEERING STANDARDS OF PRACTICE TO MEET THE MINIMUM REQUIREMENTS OF THE APPLICABLE EDITION OF THE CBC. ANY OMISSIONS OR DISCREPANCIES ON THE PLANS OR ANY DEVIATIONS FROM THE PLANS THAT ARE NECESSITATED BY FIELD CONDITIONS OR ANY CONDITION DIFFERENT FROM THOSE INDICATED ON THE PLANS SHOULD BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO CONTINUING CONSTRUCTION. ALL WORK SHALL BE COORDINATED SO COOPERATION BETWEEN THE TRADES IS ACCOMPLISHED.
9. CONNECTIONS AND IMPLIED CONSTRUCTION ASSEMBLIES THAT ARE NOT SPECIFICALLY DESCRIBED OR DETAILED SHALL BE CONSTRUCTED USING STANDARD CONSTRUCTION PRACTICES IN COMPLIANCE WITH THE GOVERNING CODES AND LOCAL ORDINANCES.
10. THE STRUCTURAL SYSTEMS HAVE BEEN DESIGNED TO CARRY THE SUPERIMPOSED LIVE LOADS AS PRESCRIBED BY THE CALIFORNIA BUILDING CODE AND IN ACCORDANCE WITH STANDARD ENGINEERING PRACTICES, WITH NO SPECIAL PROVISIONS TO CARRY CONCENTRATED LOADS FROM STORAGE AND HANDLING OF CONSTRUCTION MATERIALS OR FROM OPERATION OF CONSTRUCTION EQUIPMENT.
11. DRAWINGS AND SPECIFICATIONS REPRESENT FINISHED STRUCTURE. CONTRACTOR SHALL BE RESPONSIBLE FOR MEANS AND METHODS OF CONSTRUCTION, INCLUDING BUT NOT LIMITED TO SHORING AND TEMPORARY BRACING. THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURE TO INSURE SAFETY OF ALL PERSONS AND STRUCTURES AT THE SITE AND ADJACENT TO THE SITE. OBSERVATION VISITS TO THE SITE BY THE ARCHITECT, ENGINEER OR CONSTRUCTION MANAGER SHALL NOT RELIEVE THE CONTRACTOR OF SUCH RESPONSIBILITY.
12. THE CONTRACTOR IS RESPONSIBLE FOR AND SHALL MAINTAIN THE INTEGRITY OF ALL SCAFFOLDING, BRACING, AND SHORING SYSTEMS AS REQUIRED FOR INSTALLATION, STABILITY AND SAFETY OF NEW WORK AND EXISTING STRUCTURES, PIPING, AND FOUNDATION SYSTEMS. CONTRACTOR SHALL ALSO PROVIDE FOR THE SAFETY OF PEDESTRIANS AND JOB SITE PERSONNEL. AT ALL TIMES, THE CONTRACTOR SHALL BE SOLELY AND COMPLETELY RESPONSIBLE FOR THE CONDITION OF THE JOB SITE, INCLUDING SAFETY OF PERSONS AND PROPERTY. THE CONTRACTOR SHALL PROTECT NEW AND EXISTING CONSTRUCTION FROM INCLIMENT WEATHER AND PHYSICAL DAMAGE.
13. CONTRACTOR SHALL COORDINATE WITH THE CITY TO ENSURE ALL INSPECTIONS (INCLUDING SPECIAL INSPECTIONS) ARE COMPLETED PER THE LOCAL BUILDING DEPARTMENT REQUIREMENTS. APPROVALS BY BUILDING INSPECTORS SHALL NOT CONSTITUTE AUTHORITY TO DEVIATE FROM THE PLANS AND SPECIFICATIONS.
14. IF PROVIDED, OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER IS INTENDED TO IMPROVE THE PROBABILITY THAT THE WORK IS COMPLETED IN GENERAL CONFORMANCE WITH THE ENGINEERING INTENT OF THE DESIGN. OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY FOR COMPLETING THE CONSTRUCTION IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS, GENERALLY ACCEPTED STANDARDS OF PRACTICE, AND CITY/COUNTY REQUIRED INSPECTIONS.
15. ALL FRAMING HARDWARE SHALL BE MANUFACTURED BY SIMPSON STRONGTIE, OR EQUAL. ALTERNATE FRAMING HARDWARE MANUFACTURERS SHALL NOT BE PROVIDED UNLESS SPECIFICALLY AUTHORIZED BY THE ENGINEER AND THE BUILDING OWNER. IF ALTERNATE HARDWARE SYSTEMS ARE AUTHORIZED, THE CONTRACTOR SHALL FORWARD COMPLETE SHOP DRAWINGS FOR REVIEW AND APPROVAL.

FOUNDATION NOTES

1. FOOTING DEPTH DIMENSION ARE INTO UNDISTURBED SOIL OR ENGINEERED FILL APPROVED BY THE GEOTECHNICAL ENGINEER, IF APPLICABLE. FINAL FOOTING DEPTH SHALL BE MEASURED FROM LOWEST ADJACENT GRADE OR BOTTOM OF UNAPPROVED FILL.
2. REMOVE LOOSE SOIL AND STANDING WATER FROM FOUNDATION EXCAVATIONS PRIOR TO PLACING CONCRETE. THE GEOTECHNICAL ENGINEER (IF APPLICABLE) SHALL INSPECT AND APPROVE ALL EXCAVATIONS, SOIL COMPACTION WORK PRIOR TO PLACEMENT OF ANY REBAR OR CONCRETE, SHORING INSTALLATIONS, BACKFILL MATERIALS AND BACK FILLING PROCEDURES.
3. MINIMUM CLEARANCE OF 8" SHALL BE PROVIDED BETWEEN EARTH AND WOOD IN ALL LOCATIONS PER CBC 2304.12
4. **DRAINAGE AND SURFACE RUNOFF:**
PERIMETER GRADES SHOULD BE POSITIVELY SLOPED AT ALL TIMES TO PROVIDE FOR RAPID REMOVAL OF SURFACE WATER RUNOFF AWAY FROM THE FOUNDATION SYSTEMS AND TO PREVENT PONDING OF WATER UNDER FOUNDATIONS OR SEEPAGE TOWARD THE FOUNDATION SYSTEMS AT ANY TIME DURING OR AFTER CONSTRUCTION. PONDED WATER MAY CAUSE UNDESIRABLE SOIL SWELL OR LOSS OF STRENGTH.
- A. AS A MINIMUM REQUIREMENT, FINISHED GRADES SHOULD HAVE SLOPES OF AT LEAST 5 PERCENT WITHIN 10 FEET FROM THE EXTERIOR WALLS AND AT RIGHT ANGLES TO ALLOW SURFACE WATER TO DRAIN POSITIVELY AWAY FROM THE STRUCTURE. FOR PAVED AREAS, THE SLOPE GRADIENT CAN BE REDUCED TO 2 PERCENT.
- B. ALL SURFACE WATER SHOULD BE COLLECTED AND DISCHARGED INTO APPROVED DRAINAGE FACILITIES. APPROVED DRAINAGE FACILITIES SHALL BE PACIFIED BY THE THE CIVIL ENGINEER, IF APPLICABLE.
- C. ALL ROOF STORMWATER SHOULD BE COLLECTED AND DIRECTED TO DOWNSPOUTS. STORMWATER FROM ROOF DOWNSPOUTS SHOULD NOT BE ALLOWED TO DISCHARGE DIRECTLY ONTO THE GROUND SURFACE IN CLOSE PROXIMITY TO THE FOUNDATION SYSTEM. RATHER, STORMWATER FROM ROOF DOWNSPOUTS SHOULD BE DIRECTED BY AN IMPERMEABLE SURFACE INTO THE STREET OR TO AN APPROVED DRAINAGE FACILITY. IF THIS IS NOT ACCEPTABLE, WE RECOMMEND DOWNSPOUTS DISCHARGE AT LEAST 5 FEET AWAY FROM FOUNDATIONS.
5. **OVER-OPTIMUM SOIL MOISTURE CONDITIONS:**
THE CONTRACTOR SHOULD ANTICIPATE ENCOUNTERING EXCESSIVELY OVER-OPTIMUM (WET) SOIL MOISTURE CONDITIONS DURING WINTER OR SPRING GRADING, OR DURING OR FOLLOWING PERIODS OF RAIN. IN ADDITION, WET SOIL CONDITIONS MAY BE ENCOUNTERED NEAR THE BOTTOM OF EXCAVATIONS. WET SOIL CAN MAKE PROPER COMPACTION DIFFICULT OR IMPOSSIBLE. WET SOIL CONDITIONS SHALL BE MITIGATED BY APPROVED MEANS.
6. **OVER-OPTIMUM SOIL MOISTURE CONDITIONS:**
WHERE MOISTURE VAPOR TRANSMISSION IS A CONCERN, CONSULT A WATERPROOFING EXPERT. MOISTURE TRANSITION IS OUTSIDE THE SCOPE OF THESE PLANS.
- A. A TIGHT, WATER VAPOR RETARDING MEMBRANE SHOULD BE INSTALLED BELOW ALL SLAB FOUNDATIONS. SYSTEMS TO REDUCE MOISTURE CONDENSATION UNDER FLOOR COVERINGS. THE VAPOR RETARDER SHOULD MEET ASTM E 1745 CLASS A REQUIREMENTS FOR WATER VAPOR PERMEANCE, TENSILE STRENGTH, AND PUNCTURE RESISTANCE. VAPOR TRANSMISSION THROUGH THE SLAB FOUNDATIONS CAN ALSO BE REDUCED BY USING HIGH STRENGTH CONCRETE WITH A LOW WATER-CEMENT RATIO.
7. DEEPEN PERIMETER FOOTINGS AS REQUIRED WHERE FOOTING, OR EDGE OF SLAB, IS WITHIN 3'-0" OF EDGE OF BIO-SWALE, BIO-RETENTION FACILITIES, TRENCHES, ETC. DEEPEN FOOTING SUCH THAT A 1:1 PLANE IS MAINTAINED BETWEEN BOTTOM OF FOOTING AND BOTTOM OF ADJACENT EXCAVATION. SEE DTL. 207 FOR MORE INFORMATION.
8. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO LOCATE AND PROTECT EXISTING UTILITIES TO REMAIN DURING AND/OR AFTER CONSTRUCTION.
9. CONTRACTOR TO REMOVE ABANDONED FOOTINGS, UTILITIES, ETC. WHICH INTERFERE WITH NEW CONSTRUCTION, UNLESS OTHERWISE INDICATED. NOTIFY THE OWNER'S REPRESENTATIVE IF ANY BURIED STRUCTURES NOT INDICATED, SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC., ARE FOUND.
10. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING, UNDERPINNING AND PROTECTION OF EXISTING CONSTRUCTION.
11. PLACE BACKFILL BEHIND RETAINING WALLS AFTER CONCRETE OR MASONRY HAS ATTAINED FULL DESIGN STRENGTH. BRACE BUILDING AND PIT WALLS BELOW GRADE FROM LATERAL LOADS UNTIL ATTACHED FLOORS AND SLABS ON GRADE ARE COMPLETE AND HAVE ATTAINED FULL DESIGN STRENGTH.

SYMBOLS LEGEND

- NOT ALL SYMBOLS USED IN THE DRAWING ARE LISTED BELOW. REFER TO AMERICAN WELDING SOCIETY PUBLICATIONS FOR WELDING SYMBOLS.
- (LENGTH)
SW-# INDICATES SHEARWALL NUMBER AND MINIMUM DESIGN LENGTH. SHEARWALLS SHALL BE AS NOTED IN THE SHEARWALL SCHEDULE 210
SD1
- 103
INDICATES HOLDOWN TYPE AS MANUFACTURED BY THE "SIMPSON STRONGTIE COMPANY." HOLDOWNS SHALL BE AS NOTED IN THE HOLDOWN SCHEDULE, PER 112
SD1
- INDICATES INTERIOR BEARING WALL BELOW

SHEARWALL NOTES

1. WHERE A STRUCTURAL SHEARWALL IS INDICATED ON PLANS THE ASSEMBLY SHALL RUN HORIZONTALLY AND CONTINUOUSLY TO THE NEAREST WALL OPENING OR END OF THE WALL; THE ASSEMBLY SHALL RUN VERTICALLY CONTINUOUSLY FROM THE BOTTOM OF THE NEAREST SOLE OR BOTTOM PLATE UP TO THE TOP OF THE NEAREST DOUBLE TOP PLATE (OR BEAM). ALL PLYWOOD PANEL EDGES SHALL BE BLOCKED AND EDGE NAILED.
2. WHERE HOLDOWN POSTS OR STUDS ARE INDICATED AT THE END OF A SHEARWALL, THE SHEAR PLYWOOD SHALL BE EDGE NAILED AND THE POST SHALL RUN CONTINUOUSLY FROM THE SOLE PLATE TO THE DOUBLE TOP PLATE. HOLDOWNS SHALL BE ATTACHED TO POSTS AT THE ENDS OF SHEARWALLS AND SHALL EXTEND TO EITHER FRAMING BELOW OR TO FOUNDATION AS SHOWN ON PLANS.
3. SEE SHEARWALL SCHEDULE ON PLANS FOR REQUIRED SHEARWALL NAILING, ANCHOR BOLTS, SILL NAILS, AND OTHER SHEAR TRANSFER HARDWARE.
4. SHEARWALL PLYWOOD SHALL NOT BE CUT FOR PIPE, DUCTS, SLEEVES, ETC., U.N.O. OR DETAILED.
5. UNLESS OTHERWISE DETAILED, ALL INTERIOR SHEARWALLS SHALL BE CONTINUOUS TO THE ROOF OR FLOOR PLYWOOD IN ACCORDANCE WITH THE TYPICAL SHEAR TRANSFER DETAILS
6. PLYWOOD SHEETS LOCATED AT SHEARWALL EDGES SHALL BE AT LEAST 12" WIDE. PLYWOOD EDGES SHALL BE EDGE NAILED TO ALL SHEARWALL FRAMING MEMBERS. SEE SHEARWALL SCHEDULE FOR FIELD NAILING REQUIREMENTS.
7. SEE SHEARWALL SCHEDULE FOR SHEARWALLS THAT REQUIRE 3x MUDDSILLS AND 3x FRAMING AT ADJOINING PLYWOOD PANEL EDGES. SILL PLATES, TOP PLATES AND MEMBERS IN THE FIELD OF INDIVIDUAL PLYWOOD PANELS DO NOT TYPICALLY BACK ADJOINING PANEL EDGES AND THUS MAY BE 2x.

OBSERVATION OF CONSTRUCTION

1. IN ADDITION TO OBSERVATIONS BY THE SOILS ENGINEER(IF APPLICABLE), AND CITY INSPECTOR, OBSERVATION OF THE CONSTRUCTION BY THE PROJECT ENGINEER IS RECOMMENDED FOR THIS PROJECT. THE CONTRACTOR SHALL PHASE THE PROJECT AND COORDINATE WITH THE ENGINEER TO ENSURE THAT THE PRIMARY STRUCTURAL ELEMENTS OF THE CONSTRUCTION ARE OBSERVED PRIOR TO COVERING WITH FINISHES OR OTHER MATERIALS. AS A MINIMUM, OBSERVATION BY THE ENGINEER IS RECOMMENDED AS FOLLOWS:
- A. REINF. STEEL AND HARDWARE EMBEDDED IN THE FNDTION SHALL BE OBSERVED PRIOR TO CONCRETE PLACEMENT
B. SHEARWALLS AND FRAMING ELEMENTS SHALL BE OBSERVED PRIOR TO INSTALLATION OF FINISHES.
2. OBSERVATION OF THE CONSTRUCTION BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR FROM RESPONSIBILITY TO COMPLETE THE CONSTRUCTION IN CONFORMANCE WITH THE PROJECT DOCUMENTS AND GENERALLY ACCEPTED STANDARDS OF PRACTICE. THE PURPOSE OF OUR VISITS WILL BE TO BECOME GENERALLY FAMILIAR WITH THE PROGRESS AND QUALITY OF THE CONTRACTOR'S WORK AND DETERMINE IF THE WORK IS PROGRESSING IN GENERAL CONFORMANCE WITH OUR DESIGN INTENT. DURING OUR VISITS, ADVANCED ENGINEERING WILL NOT BE MAKING DETAILED INSPECTIONS, OR VERIFYING DIMENSIONS. THE CONTRACTOR IS RESPONSIBLE FOR FOLLOWING APPLICABLE CODES AND THE APPROVED CONSTRUCTION DOCUMENTS.

SPECIAL INSPECTIONS PER 2022 CBC 1701

1. IN ADDITION TO OBSERVATIONS BY THE CITY INSPECTOR AND THE PROJECT ENGINEER, SPECIAL INSPECTIONS BY AN ICC CERTIFIED SPECIAL INSPECTOR IS REQUIRED AS FOLLOWS:
- A. EPOXY ANCHORS INSTALLED IN CONCRETE SHALL HAVE SPECIAL INSPECTION OF HOLE DEPTH & DIAMETER, MATERIALS, CLEANING PROCEDURES, AND INSTALLATION PER ICC-ES/ESR-2508 SECTION 4.4.3.
B. TITEN HD ANCHORS INSTALLED IN CONCRETE SHALL HAVE SPECIAL INSPECTION OF DRILLING AND INSTALLATION PER ICC-REPORT ICC-ES/ESR-2713 SECTION 4.4.
C. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SEISMIC-FORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS AND HOLDOWNS PER SECTION 1705.12.2 OF THE CURRENT EDITION OF THE C.B.C.
D. CONTINUOUS INSPECTIONS OF CAST-IN-PLACE DEEP FOUNDATION (PIERS) OR DRILLING OPERATIONS. COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT SHALL BE PROVIDED PER SECTION 1705.8 OF THE CURRENT EDITION OF THE C.B.C.
E. CONTINUOUS INSPECTIONS OF CAST-IN-PLACE DEEP FOUNDATION (PIERS) TO VERIFY: PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, LENGTHS, AND EMBEDMENTS PER SECTION 1705.8 OF THE CURRENT EDITION OF THE C.B.C.
2. THE CONTRACTOR SHALL COORDINATE WITH THE CITY TO ENSURE SPECIAL INSPECTION IS PROVIDED PER CITY REQUIREMENTS. THE SPECIAL INSPECTOR SHALL BE EMPLOYED BY THE OWNER AND MUST DEMONSTRATE HIS QUALIFICATIONS TO THE ARCHITECT/ENGINEER OF RECORD AND THE BUILDING OFFICIAL.

FASTENING SCHEDULE (C.B.C. TABLE 2304.10.2)

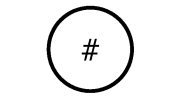
ITEM	DESCRIPTION OF CONNECTION	FASTENING (NOTE 1, 4, 8)		
		COMMON / BOX	SMOOTH SHANK	STAPLES
1	JOIST TO SILL OR GIRDER, TOE-NAIL	(3) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
2	BRIDGING TO JOIST, TOE-NAIL EA. END	(2) 8d	(2) 3" x 0.131"	(2) 3"-14 GA.
3	1" x 6" SUBFLOOR OR LESS, TO EACH JOIST, FACE NAIL	(2) 8d	N.A.	N.A.
4	WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	(3) 8d	N.A.	N.A.
5	2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	(2) 16d	N.A.	N.A.
6	SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL	16d AT 16"	3" x 0.131" AT 8" O.C.	3"-14 GA. AT 12" O.C.
	SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS	(3) 16d AT 16"	(4) 3" x 0.131" AT 16" O.C.	(4) 3"-14 GA. PER 16" O.C.
7	TOP PLATE TO STUD, END NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
8	STUD TO SOLE PLATE, END NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
	STUD TO SOLE PLATE, TOE-NAIL	(4) 8d	(4) 3" x 0.131"	(3) 3"-14 GA.
9	DOUBLE STUDS, FACE NAIL	16d AT 24"	3" x 0.131" AT 8" O.C.	3"-14 GA. AT 8" O.C.
10	DOUBLED TOP PLATES, TYP. FACE NAIL	16d AT 16"	3" x 0.131" AT 12" O.C.	3"-14 GA. AT 12" O.C.
	DOUBLED TOP PLATES, LAP SPLICE	(8) 16d	(12) 3" x 0.131"	(12) 3"-14 GA.
11	BLOCKING BETWEEN JOISTS OR RAFTERS, TO TOP PLATE, TOE-NAIL	(3) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
12	RIM JOIST TO TOP PLATE, TOE-NAIL	8d AT 6"	3" x 0.131" AT 6" O.C.	3"-14 GA. AT 6" O.C.
13	TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
14	CONTINUOUS HEADER, TWO PIECES	16d AT 16" ALONG EDGE	N.A.	N.A.
15	CEILING JOIST TO PLATE, TOE-NAIL	(3) 8d	(5) 3" x 0.131"	(5) 3"-14 GA.
16	CONTINUOUS HEADER TO STUD, TOE-NAIL	(4) 8d	N.A.	N.A.
17	CEILING JOISTS: LAPS OVER PARTITIONS, FACE NAIL (SEE C.B.C. TABLE 2308.7.3.1)	(3) 16d MIN.	(4) 3" x 0.131"	(4) 3"-14 GA.
18	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL (SEE C.B.C. TABLE 2308.7.3.1)	(3) 16d MIN.	(4) 3" x 0.131"	(4) 3"-14 GA.
19	RAFTER TO PLATE, TOE-NAIL (SEE C.B.C. TABLE 2308.7.9)	(2) 8d	(3) 3" x 0.131"	(3) 3"-14 GA.
20	1" DIAGONAL BRACE TO EACH STUD AND PLATE, FACE NAIL	(3) 8d	(2) 3" x 0.131"	(3) 3"-14 GA.
21	1" x 8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL	(3) 8d	N.A.	N.A.
22	WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	(3) 8d	N.A.	N.A.
23	BUILT-UP CORNER STUDS	16d AT 24"	3" x 0.131" AT 16" O.C.	3"-14 GA. AT 16" O.C.
24	BUILT-UP GIRDERS AND BEAMS, FACE NAIL TOP AND BTM STAGG. ON OPP. SIDE	20d AT 32"	3" x 0.131" AT 24" O.C.	3"-14 GA. AT 24" O.C.
	BUILT-UP GIRDERS AND BEAMS, FACE NAIL AT ENDS AND AT EACH SPLICE	(2) 20d	(3) 3" x 0.131"	(3) 3"-14 GA.
25	2" PLANK TO EACH BEARING	16d	N.A.	N.A.
26	COLLAR TIE TO RAFTER, FACE NAIL	(3) 10d	(4) 3" x 0.131"	(4) 3"-14 GA.
27	JACK RAFTER TO HIP, TOE-NAIL	(3) 10d	(4) 3" x 0.131"	(4) 3"-14 GA.
	JACK RAFTER TO HIP, FACE NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
28	ROOF RAFTER TO 2x RIDGE BEAM, TOE-NAIL OR FACE-NAIL	(2) 16d	(3) 3" x 0.131"	(3) 3"-14 GA.
29	JOIST TO BAND JOIST, FACE NAIL	(3) 16d	(4) 3" x 0.131"	(4) 3"-14 GA.
30	LEDGER STRIP, FACE NAIL	(3) 16d	(4) 3" x 0.131"	(4) 3"-14 GA.
31	WOOD STRUCTURAL PANELS AND PARTICLE BOARD (NOTE 2): SUBFLOOR, ROOF + WALL SHEATHING TO FRAMING:			
	1/2" AND LESS	6d (NOTE 3, 7)	2.375" x 0.113" (NOTE 9)	1.75" x 16 GA. (NOTE 10)
	19/32" TO 3/4"	8d (NOTE 4)	2.375" x 0.113" (NOTE 11)	2" x 16 GA. (NOTE 11)
	7/8" TO 1"	8d (NOTE 3)	N.A.	N.A.
	1-1/8" TO 1-1/4"	10d OR 8d (NOTE 4)	N.A.	N.A.
	SINGLE FLOOR (COMBINATION SUBFLOOR- UNDERLAYMENT TO FRAMING):			
	3/4" AND LESS	6d (NOTE 5)	N.A.	N.A.
	7/8" TO 1"	8d (NOTE 5)	N.A.	N.A.
	1-1/8" TO 1-1/4"	10d (NOTE 4)	N.A.	N.A.
	PANEL SIDING TO FRAMING (NOTE 2):			
32	1/2" AND LESS	6d (NOTE 6)	N.A.	N.A.
	5/8"	8d (NOTE 6)	N.A.	N.A.

NOTES:

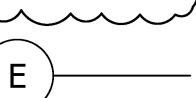
- FOR S1: 1 inch = 25.4 mm
1. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
2. NAILS SPACED AT 6" O.C. AT EDGES, 12" O.C. AT INTERMEDIATE SUPPORTS EXCEPT 6" O.C. AT ALL SUPPORTS WHERE SPANS ARE 4'-0" OR MORE. NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO C.B.C. SECTION 2305. WALL SHEATHING NAILS ARE PERMITTED TO BE COMMON, BOX OR CASING. COMMON OR DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148")
3. COMMON (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148"; 16d - 3.5" x 0.162")
4. DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2.5" x 0.131"; 10d - 3" x 0.148")
5. CORROSION-RESISTANT SIDING (6d - 1.875" x 0.106"; 8d - 2.375" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 2.5" x 0.113") NAILS.
7. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2.5" x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS.
8. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16".
9. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS.
10. FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3" O.C. AT EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.
11. FASTENERS SPACED 4" O.C. AT EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS.

NAIL SCHEDULE

FASTENER PER PLAN	SHANK DIA.	HEAD DIA.	MIN. LENGTH
8d	0.131"	0.281"	2-1/2"
10d	0.148"	0.312"	3"
16d	0.148"	0.344"	3-1/4"
16d COMMON	0.162"	0.344"	3-1/2"
20d	0.192"	0.406"	4"



INDICATES POST BELOW



INDICATES POST ABOVE



DETAIL NUMBER

SHEET NUMBER

INDICATES ELEVATION

INDICATES REVISION NUMBER AND PLAN REVISIONS INSIDE CLOUD

REFERENCE TO STRUCTURAL CALCULATIONS FOR THE BENEFIT OF THE BUILDING OFFICIAL

REFERENCE TO STRUCTURAL CALCULATIONS FOR THE BENEFIT OF THE BUILDING OFFICIAL

ABBREVIATION LEGEND

A.B.	ANCHOR BOLT	EXT.	EXTERIOR	OPNG	OPENING
ABV	ABOVE	FDN	FOUNDATION	O.S.B.	ORIENTED STRAND BOARD
ACI	AMERICAN CONCRETE	F.F.	FINISH FLOOR	O.W.S.J.	OPEN WEB STEEL JOIST
ADD'L	ADDITIONAL	FIN.	FINISH	O.W.J.G.	OPEN WEB JOIST GIRDER
A.F.F.	ABOVE FINISHED FLOOR	FLR	FLOOR	P.A.	POST ABOVE
AGGREG.	AGGREGATE	FLG	FLANGE	P.A.F.	POWDER-ACTUATED FASTER
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	F.LR	FLOOR	P.C.F.	POUNDS PER CUBIC FOOT
ATTC	AMERICAN INSTITUTE OF TIMBER CONSTRUCTION	F.N.	FACE NAIL	PEN.	PENETRATION
ALT.	ALTERNATE	F.O.C.	FACE OF CONCRETE	PERF.	PERFORATED
ARCH.	ARCHITECT(URAL)	F.O.M.	FACE OF MASONRY	PERP.	PERPENDICULAR
ASTM	AMERICAN SOCIETY FOR TESTING & MATERIALS	F.O.S.	FACE OF STUD	PLR	PLATE
B/	BOTTOM OF	F.P.	FIREPLACE - OR - FULL PENETRATION	PLY.	PLYWOOD
B.C.	BOTTOM CHORD	FRMD	FRAMED	PLYWD	PLYWOOD
BLDG	BUILDING	FRMG	FRAMING	PR	PAIR
BLK	BLOCK	F.S.	FAR SIDE	P.S.F.	POUNDS PER SQUARE FOOT
BLKG	BLOCKING	FT	FOOT	P.S.L.	PARALLEL-STRAND LUMBER
BLW	BELOW	FTG	FOOTING	PST	POST
BR	BEAM	G & N	GLUE AND NAIL	PT	POINT
B.N.	BOUNDARY NAILING	GA.	GAGE	P.T.	PRESSURE TREATED - OR - POST-TENSION
B.O.	BY OTHERS - OR - BOTTOM OF	GALV.	GALVANIZED	R	RADIUS
B.O.C.	BOTTOM OF CONCRETE	GEN.	GENERAL	REC'D	RECOMMEND(ATION)
B.O.F.	BOTTOM OF FOOTING	GLB	GLUED-LAMINATED BEAM	REF.	REFERENCE
B.O.S.	BOTTOM OF STEEL	GR.	GRADE	REINF.	REINFORCED - OR - REINFORCING - OR - REINFORCEMENT
B.RG	BEARING	G.T.	GIRDER TRUSS	REQD	REQUIRED
B.S.	BOTH SIDES	G.W.B.	GYPSON WALL BOARD	RET.	RETAINING
BTM	BOTTOM	H.D.G.	HOT-DIP GALVANIZED	RETAIN.	RETAINING
BTWN	BETWEEN	HDR	HEADER	R.J.	ROUGH JOIST
CALCS	CALCULATIONS	HGR	HANGER	R.O.	ROUGH OPENING
CANT.	CANTILEVER	H.N.H.	HEAVY HEX NUT	R.P.	REAL POST
C.B.	CEILING BEAM	H.N.	HEX NUT	R/S.	ROOF RAFTER
C.B.C.	CALIFORNIA BUILDING CODE	HORIZ.	HORIZONTAL	R/R	RESAWN (ROUGH-SAWN)
C.C.	CENTER TO CENTER	H.S.B.	HIGH STRENGTH BOLT	R.T.	ROOF TRUSS
C.C.J.	CRACK CONTROL JOINT	HSS	STRUCTURAL TUBE	R.W.	RETAINING WALL
C.C.F.	CUBIC FOOT	HT	HEIGHT	RWD	REDWOOD
C.J.	CONTROL JOINT - OR - CEILING JOIST	I.D.	INSIDE DIAMETER	S.A.D.	SEE ARCHITECTURAL
Ç	CENTERLINE	INFO.	INFORMATION	DRAWINGS	
CLG	CEILING	INT.	INTERIOR	S.B.	SOLID BLOCK
CLR	CLEAR	INV.	INVERT(ED)	S.B.T.	SET-BACK TRUSS
C.M.U.	CONCRETE MASONRY UNIT	JNT	JOINT	SCHED.	SCHEDULE
COL.	COLUMN	JST	JOIST	SEP.	SEPARATION
COLL.	COLLECTOR	J.T.	JACK TRUSS	S.G.T.	SH-GRIDER TRUSS
CONC.	CONCRETE	K.D.	KIPS	SHT	SHEET
COND.	CONDITION	K.D.	KILN-DRIED	SIM.	SIMILAR
CONN.	CONNECTION	KLF	KIPS PER LINEAL FOOT	S.M.S.	SHEET METAL SCREW
CONSTR.	CONSTRUCTION	K.P.	KING POST	S.J.	SOFFIT JOIST
CONT.	CONTINUOUS	KBS	KIPS	SPEC.	SPECIFICATION
CONTR.	CONTRACTOR	L.B.	LAG BOLT	SQ.	SQUARE
C.P.	COMPLETE PENETRATION	L.F.	LINEAL FOOT	S.S.	SELECT STRUCTURAL
C.S.	COUNTERSUNK	LONGIT.	LONGITUDINAL	S4S	SURFACED 4 SIDES
C.T.	COLLECTOR TRUSS	LG	LONG	STAGG.	STAGGER - OR - STAGGERED
CTR	CENTER	L.L.	LIVE LOAD	STD	STANDARD
CTRD	CENTERED	LSL	LAMINATED STRAND LUMBER	STIFF.	STIFFENER
C.Y.	CUBIC YARD	LT	LIGHT	STL	STEEL
D.	DOUBLE	LTWT	LIGHTWEIGHT	STRUCT.	STRUCTURAL
D.F.	DETAIL	LVL	LAMINATED VENEER LUMBER	SW	SHEARWALL
D.F.	DOUGLAS FIR	MATL	MATERIAL	SYMM.	SYMMETRICAL
DIA.	DIAMETER	MAX.	MAXIMUM	T + B	TOP AND BOTTOM
DIM.	DIMENSION	M.B.	MACHINE BOLT	T + G	TONGUE AND GROOVE
DJD.	DECK JOIST	MECH.	MECHANICAL	T.C.	TOP CHORD
D.T.	DRAG TRUSS	MEMB.	MEMBRANE <td>T.K.</td> <td>THICK</td>	T.K.	THICK
DEP	DEEP	MID.	MIDDLE	THND	THICKENED
DTL	DETAIL	MIN.	MINIMUM	T.N.	TO NAIL
DWG	DRAWING	M.I.W.	MALLEABLE IRON WASHER	T.O.B.	TOP OF BLOCK
(E)	EXISTING	M.M.	MASONRY OPENING	T.O.C.	TOP OF CONCRETE
E.A.	EACH	MOIST.	MOISTURE	T.O.P.	TOP OF PLATE
E.B.	EXPANSION BOLT	MFR	MANUFACTURER	T.O.S.	TOP OF STEEL - OR - TOP OF SHEATHING
E.F.	EACH FACE	MFRD	MANUFACTURED	T.O.W.	TOP OF WALL
E.F.	EXPANSION JOINT	(N)	METAL	T/	TOP OF
ELV.	ELEVATION	N.A.	NOT APPLICABLE	T.P.	TOP PLATE
EMB.	ELEVATOR	N.B.	NON-BEARING	TRANSV.	TRANSVERSE
EMB.	EMBEDMENT	N.D.T.	NON-DESTRUCTIVE TESTING	TYP.	TYPICAL
EMB.	EMBEDMENT	N.I.C.	NOT IN CONTRACT	U.N.O.	UNLESS NOTED OTHERWISE
E.O.R.	ENGINEER OF RECORD	NLG	NAILING	VERT.	VERTICAL
E.Q.	EQUIPMENT	No.	NUMBER	V.I.F.	VERIFY IN FIELD
EQUIP.	EQUIPMENT	N.S.	NEAR SIDE	W/	WOOD
E.S.	EACH SIDE	N.S.G.	NON-SHRINK GROUT	W.P.	WORK POINT - OR - WATERPROOF(ING)
E.W.	EACH WAY	N.T.S.	NOT TO SCALE	WT	WEIGHT
		O.	OVER	W.F.F.	WELDED WIRE FABRIC
		O.C.	ON CENTER	W.M.	WIRE MESH
		O.D.	OUTSIDE DIAMETER	#	NUMBER (PREFIX)
		O.F.S.	OUTSIDE FACE OF STUD	#	POUNDS (SUFFIX)
		O.H.	OVERHANG - OR - OPPOSITE HAND		

GENERAL FRAMING NOTES

1. NO STRUCTURAL MEMBER SHALL BE CUT OR NOTCHED UNLESS SPECIFICALLY SHOWN, NOTED, OR APPROVED BY THE ENGINEER. NOTCH DETAILS, IF PROVIDED, ARE FOR GENERAL GUIDANCE ONLY. THE ENGINEER SHALL BE CONTACTED TO APPROVE LOCATIONS OF PROPOSED NOTCHES. STUDS IN EXTERIOR WALLS AND BEARING PARTITIONS MAY BE CUT OR NOTCHED TO A DEPTH NOT EXCEEDING 25% OF STUD WIDTH. CUTTING OR NOTCHING OF STUDS IN NON-BEARING PARTITIONS SHALL NOT EXCEED 40% OF THE WIDTH. SEE DETAIL 404 FOR MORE INFORMATION.
2. **TYPICAL FRAMING AND STRUCTURAL MATERIALS:**
ALL FRAMING LUMBER SHALL BE DOUGLAS FIR, AND SHALL BE STAMPED WITH A GRADE MARK WITH THE FOLLOWING GRADES. FRAMING LUMBER SHALL CONFORM TO GRADING RULES OF WVPA AND COMPLIES WITH DOC PS 20.
MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19% AT THE TIME OF CONSTRUCTION.
- | FRAMING MATERIAL | GRADE | FRAMING MATERIAL | GRADE |
|------------------------------------|------------------------|--|----------------------|
| TYPICAL WALL STUDS | PER NOTE 6 BELOW | RAFTERS (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. |
| SOLE PLATES (2x MIN.) ¹ | STUD GRADE, MINIMUM | BUILT UP HEADERS (2-2x) | D.F. #2, MIN. U.N.O. |
| MUDSILLS (2x MIN.) ² | P.T. D.F. STD & BETTER | 4x POSTS/BEAMS/RAFTERS | D.F. #2, MIN. U.N.O. |
| TOP PLATES (2x MIN.) ³ | D.F. #2, MIN. U.N.O. | 6x POSTS/BEAMS/RAFTERS | D.F. #1, MIN. U.N.O. |
| RIM JOIST (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. | STRUCTURAL PLYWOOD | APA RATED SHGT |
| FLR JOISTS (2x DIMENSIONAL) | D.F. #2, MIN. U.N.O. | LUMBER EXPOSED TO WEATHER ⁴ | P.T. D.F. #2 U.N.O. |
1. SOLE PLATES, AS NOTED ABOVE, ARE ALL INTERIOR SILL PLATES NOT IN CONTACT WITH CONCRETE. MATERIAL SHALL BE UC1 INTERIOR/DRY CATEGORY AS DEFINED BY AWPA STANDARD U1.
2. MUDSILLS, AS NOTED ABOVE, ARE ALL INTERIOR SILL PLATES IN DIRECT CONTACT WITH CONCRETE. MATERIAL SHALL BE UC2 INTERIOR/DAMP CATEGORY AS DEFINED BY AWPA STANDARD U1.
3. LUMBER EXPOSED TO WEATHER, AS NOTED ABOVE, IS ALL EXTERIOR LUMBER ABOVE GROUND AND EXPOSED TO WEATHER. MATERIAL SHALL BE UC3B ABOVE GROUND EXPOSED CATEGORY AS DEFINED BY AWPA STANDARD U1.
3. **PRESERVATIVE TREATED OR NATURALLY DURABLE MATERIALS:**
LUMBER SHALL BE TREATED WITH TYPICAL WATERBORNE PRESERVATIVES: ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), COPPER AZOLE (CA-B & CA-C, MCA-C) AND SODIUM BORATES (SBX/DOT). THESE TREATMENTS ARE OFTEN REFERRED TO BY TRADE NAMES SUCH AS: WOLMANIZED NATURAL SELECT™ (COPPER AZOLE), PRESERVE AND NATUREWOOD® (ACQ), MICROPRO™, SMART SENSE™ (MCQ), AND ADVANCE GUARD® (BORATE).
- A. ALL INTERIOR/DAMP CONSTRUCTION, UC2 CATEGORY, SHALL BE TREATED WITH SODIUM BORATE SBX/DOT.
1. PLAIN CARBON STEEL FASTENERS, INCLUDING NUTS AND WASHERS, IN CONTACT WITH SODIUM BORATE SBX/DOT PRESERVATIVE-TREATED WOOD IN AN INTERIOR, DRY ENVIRONMENT SHALL BE PERMITTED.
- B. ALL EXTERIOR ABOVE GROUND CONSTRUCTION (INCL. DECK FRAMING), UC3B CATEGORY, SHALL BE TREATED WITH ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), OR COPPER AZOLE (CA-B & CA-C, MCA-C)
1. ALL FASTENERS AND CONNECTORS IN CONTACT WITH PRESERVATIVE-TREATED TREATED WOOD USING ALKALINE COPPER QUAT (ACQ-C, ACQ-D, ACQ-D CARBONATE), OR COPPER AZOLE (CA-B & CA-C, MCA-C) SHALL BE HOT DIPPED GALVANIZED OR STAINLESS STEEL IN ACCORDANCE WITH GOVERNING CBC SEC. 2304.10 AND SHALL BE IN ACCORDANCE WITH ASTM A 153.
- C. WOOD JOISTS OR WOOD STRUCTURAL FLOORS THAT ARE CLOSER THAN 18 INCHES OR WOOD GIRDERS THAT ARE CLOSER THAN 12 INCHES TO EXPOSED GROUND IN CRAWL SPACES OR UNEXCAVATED AREAS LOCATED WITHIN THE PERIMETER OF THE BUILDING FOUNDATION SHALL BE PRESERVATIVE-TREATED WOOD IN ACCORDANCE CBC.
4. **GLUE-LAMINATED STRUCTURAL MATERIALS:**
STANDARD SPECIFICATIONS FOR GLUE-LAMINATED STRUCTURAL MEMBERS, ANSI/AITC A 190.1 AND ASTM D3737. GLUE-LAMINATED BEAMS SHALL BE INSPECTED AND A CERTIFICATE PROVIDED TO THE FILED INSPECTOR AT THE TIME OF FRAMING INSPECTION. FABRICATION SHALL BE PERFORMED IN ACCORDANCE WITH CBC 1705.5. ALL GLUE-LAMINATED BEAMS THAT ARE CONTINUOUS OVER SUPPORTS OR CANTILEVERED SHALL HAVE TENSION LAMINATIONS ON TOP OF BEAMS.
MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 16% AT THE TIME OF CONSTRUCTION.
- A. GLULAM MEMBERS IN DRY SERVICE USE SHALL BE DOUGLAS-FIR 24F-V4 (SINGLE SPAN) OR COMBINATION 24F-V8 (MULTI-SPAN & CANTILEVER) U.N.O. BEAMS SHALL BE ARCHITECTURAL GRADE WHEN EXPOSED TO VIEW, S.A.D.
1. GLULAM MEMBERS, SEE PLAN SPECIFICATIONS FOR CAMBER WHERE OCCURS.
- | MATERIAL GRADE | E (x10 ³ psi) | Fb (psi) | Fv (psi) | Fc (psi) | RADIUS, U.N.O. |
|----------------|--------------------------|----------|----------|----------|----------------|
| DF/DF 24F-V4 | 1.8 | 2400 | 265 | 650 | 3500' RADIUS |
| DF/DF 24F-V8 | 1.8 | 2400 | 265 | 650 | NO CAMBER |
- B. GLULAM MEMBERS IN WET SERVICE USE SHALL BE ALASKAN CEDAR 20F-V12 (SINGLE SPAN) OR COMBINATION 20F-V13 (MULTI-SPAN & CANTILEVER) U.N.O.
1. GLULAM MEMBERS, SEE PLAN SPECIFICATIONS FOR CAMBER WHERE OCCURS.
- | MATERIAL GRADE | E (x10 ³ psi) | Fb (psi) | Fv (psi) | Fc (psi) | RADIUS, U.N.O. |
|----------------|--------------------------|----------|----------|----------|----------------|
| AC/AC 20F-V12 | 1.5 | 2000 | 265 | 560 | 3500' RADIUS |
| AC/AC 20F-V13 | 1.5 | 2000 | 265 | 560 | NO CAMBER |
- C. DESIGN, FABRICATION AND CONSTRUCTION OF STRUCTURAL GLULAM MEMBERS SHALL CONFORM TO THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION STANDARD, MANUAL No. 301 AND THE COMMERCIAL STANDARD FOR STRUCTURAL GLUED LAMINATED TIMBER, CS 253.
- D. STRUCTURAL PLYWOOD SHALL BE GRADED PER APA PSI-83AND SHALL BE INTERIOR TYPE SHEATHING C-D GRADE WITH EXTERIOR GLUE. EQUIVALENT O.S.B. WOOD STRUCTURAL PANEL MAY BE USED AS AN ALTERNATE TO PLYWOOD. HOWEVER, IN ACCORDANCE WITH THE TILE COUNCIL OF AMERICA RECOMMENDATIONS, O.S.B. SHALL NOT BE USED BELOW TILE MORTAR. ALL HORIZONTAL PLYWOOD SHALL BE LAID WITH FACE GRAIN PERPENDICULAR TO FRAMING MEMBERS, WITH STAGGERED JOINTS.
5. **STRUCTURAL COMPOSITE LUMBER (SCL) MATERIALS:**
ALL STRUCTURAL COMPOSITE LUMBER (SCL) SHALL BE MANUFACTURED PER ASTM D5055 AND ASTM D5456. STRUCTURAL COMPOSITE LUMBER SHALL BE MANUFACTURED BY BOISE CASCADE, OR EQUAL. ALTERNATE MANUFACTURERS ARE PERMITTED AND CONSIDERED AN EQUIVALENT SUBSTITUTION IF THE SUBSTITUTED MATERIAL MEETS, OR EXCEEDS, ALL THE DESIGN PROPERTIES LISTED BELOW.
- | SPECIFICATION | MATERIAL | WIDTH (in.) ^{1,2} | E (x10 ³ psi) | Fb (psi) | Fv (psi) | F _c (psi) | F _{cu} (psi) |
|---------------|------------------|----------------------------|--------------------------|----------|----------|----------------------|-----------------------|
| RIM JOIST | VERSA-STRAND LVL | 1-5/16 | 1.4 | 1800 | 225 | 525 | 2500 |
| LVL COLUMN | VERSA-LAM LVL | 3.5, 5.25, 7 | 1.7 | 2650 | 285 | 750 | 3000 |
| LVL STUD | VERSA-STUD LVL | 1.5 | 1.7 | 2617 | 285 | 450 | 3000 |
| LVL BEAM | VERSA-LAM 2.0E | 1.75 | 2.0 | 2800 | 285 | 750 | 3000 |
| LVL BEAM | VERSA-LAM 2.0E | 3.5, 5.25, 7 | 2.0 | 3100 | 285 | 750 | 3000 |
1. BEAM DEPTH IS ASSUMED TO MATCH FLOOR FRAMING DEPTH U.N.O.
2. MFR BEAM MEMBERS MAY BE BUILT UP TO ACHIEVE SPECIFIED WIDTHS PER DETAIL 609 U.N.O. ON PLANS.
6. ALL STUD WALLS SHOWN ON STRUCTURAL DRAWINGS SHALL BE FRAMED AS FOLLOWS (U.O.N.):
- EXTERIOR WALL STUD HEIGHT TABLE (L/360 DEFL.)
- | MAX. PLATE HT | STUD TYPE | SPACING |
|---------------|---------------------|----------|
| 21'-5" | (2) 2x6 DOUG-FIR #1 | 12" O.C. |
| 19'-5" | (2)-2x6 DOUG-FIR #1 | 16" O.C. |
| 15'-1" | 2x6 DOUG-FIR #2 | 16" O.C. |
| 12'-1" | (2)-2x4 DOUG-FIR #1 | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
| | 2x4 DOUG-FIR #1 | 16" O.C. |
| 10'-1" | 2x6 STUD GRADE | 16" O.C. |
| 9'-1" | 2x4 DOUG-FIR #2 | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
| 8'-1" | 2x4 STUD GRADE | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
- INTERIOR WALL STUD HEIGHT TABLE (L/240 DEFL.)
- | MAX. PLATE HT | STUD TYPE | SPACING |
|---------------|----------------|----------|
| 21'-5" | 2x6 STUD GRADE | 12" O.C. |
| 19'-5" | 2x6 STUD GRADE | 16" O.C. |
| 15'-1" | 2x6 STUD GRADE | 16" O.C. |
| 12'-1" | 2x4 STUD GRADE | 16" O.C. |
| | 2x6 STUD GRADE | 16" O.C. |
- NOTE:
- A. STUD HEIGHTS EXCEEDING 10'-1" SHALL BE BRACED MID-HEIGHT USING FULL DEPTH 2x BLOCKING.
7. TOP PLATES SHALL BE DOUBLED ON ALL STUD WALLS. LAP 4'-0" MINIMUM AT TOP PLATE SPLICES, WITH (12) 16d NAILS EACH SIDE OF SPLICE, U.N.O. SPLICES IN UPPER AND LOWER PLATES SHALL BE STAGGERED 4'-0" MINIMUM.
8. POSTS IN WALLS MAY BE MADE WITH MULTIPLE STUDS OF EQUIVALENT WIDTH AND DEPTH, U.O.N. SECURE MULTIPLE STUDS WITH 16d NAILS AT 8" O.C. STAGGERED.
9. PROVIDE KING STUDS AT THE ENDS OF ALL HEADERS OR OTHER BEAMS INSTALLED IN WALLS, SEE DETAIL 401 ADJACENT, STACKING WINDOWS SHALL BE SEPARATED BY KING STUDS THAT ARE CONTINUOUS FROM SILL TO TOP PLATE, CRIPPLE STUDS UNDER HEADERS SHALL BE CONTINUOUS TO SILL PLATE. END NAIL KING STUDS TO HEADERS.
10. ALL MEMBERS IN BEARING SHALL BE ACCURATELY CUT AND ALIGNED SO THAT FULL BEARING IS PROVIDED WITHOUT THE USE OF SHIMS.
11. BLOCK ALL STUD WALLS AS REQUIRED FOR SHEATHING AND FINISHES. BALLOON FRAME ALL WALLS WITH SLOPING CEILING OR WITH RAISED CEILINGS.
12. INSTALL HORIZONTAL MEMBERS WITH CROWN UP. WHERE KNOTS EXIST NEAR THE TOP OR BOTTOM OF HORIZONTAL MEMBERS, INSTALL MEMBER WITH KNOTS UP. CANTILEVERED DECK JOISTS SHALL BE CAREFULLY NOTCHED AND TRIMMED (IF NECESSARY) TO PROVIDE SLOPE WITHOUT OVER-CUTTING.
13. PROVIDE FULL DEPTH BLOCKING OR CONTINUOUS RIM JOIST AT ALL FLOOR AND ROOF FRAMING SUPPORTS. FRAMING MEMBERS SHALL HAVE A MINIMUM OF 2" BEARING AT SUPPORTS. LAPPING JOISTS SHALL HAVE 6" MINIMUM OVERLAP CENTERED OVER INTERIOR SUPPORTS.
14. ALL BOLTED WOOD CONNECTIONS SHALL HAVE A WASHER UNLESS A STEEL PLATE IS SPECIFIED. HOLES SHALL BE PROPERLY ALIGNED. OVERSIZED HOLES ARE NOT ALLOWED. NUTS SHALL BE SNUG TIGHTENED. BOLT HOLES SHALL BE NOMINAL DIAMETER OF BOLT PLUS 1/16".
15. NAILED WOOD CONNECTIONS SHALL USE COMMON WIRE NAILS, U.N.O. MIN. NAILING REQUIREMENTS FOR STANDARD CONNECTIONS SHALL BE IN ACCORDANCE WITH THE GOVERNING C.B.C. FASTENING SCHEDULE (TABLE 2304.10.1).
16. ALL MANUFACTURED CONNECTION HARDWARE SHALL BE AS DESIGNATED ON DRAWINGS AND INSTALLED (WITH ALL NAIL HOLES FILLED) IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS AND APPLICABLE ICC APPROVALS.
17. INSTALL LAG SCREWS IN DRILLED LEAD HOLES WITH A DIA. EQUAL TO 3/4" OF THE SHANK DIAMETER. LAG SCREWS SHALL NOT BE HAMMERED IN. PROVIDE WASHERS UNDER HEADS BEARING ON WOOD. HOLES SHALL BE PROPERLY ALIGNED.

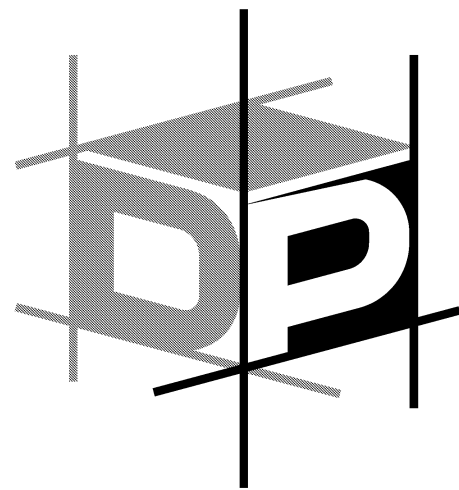
CONCRETE NOTES

- CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF THE 2022 BUILDING CODE (C.B.C.) AND THE LATEST EDITION OF THE AC1318-14, UNLESS NOTED OTHERWISE.
1. CONCRETE REQUIREMENTS: BASED ON NORMAL WEIGHT CONCRETE (UNIT WEIGHT OF 145 TO 150 pcf).
- | | MINIMUM COMPRESSIVE STRENGTH (psi) (AT 28 DAYS) | SLUMP (+/- 1/2") | AGGREG. SIZE | SPEC. INSP. REQD | MAXIMUM WATER TO CEMENT RATIO (NOTE 1F) | CEMENT TYPE ASTM C150 |
|--------------------------|---|------------------|--------------|------------------|---|-----------------------|
| GRADE BEAMS & STEM WALLS | 3000 | 4" | 3/4" | NO ** | .55 | II OR V |
| CONCRETE PIERS | 3000 | 4" | 3/4" | NO ** | .55 | II OR V |
| FOOTINGS ON GRADE | 3000 | 4" | 1" | NO ** | .55 | II OR V |
| FLOOR SLABS ON GRADE | 3000 | 4" | 1" | NO ** | .45 | II OR V |
- ** **SPECIAL INSPECTION IS NOT REQUIRED, DESIGN COMPRESSIVE STRENGTH IS 2500 psi, HIGHER STRENGTHS HAVE BEEN SPECIFIED FOR QUALITY CONTROL.**
- A. COARSE AGGREGATE SHALL BE HARD, DURABLE CRUSHED STONE OR GRAVEL GRADED PER ASTM C33. MAXIMUM SIZE OF AGGREGATE SHALL BE AS NOTED IN SCHEDULE ABOVE AND DEFINED BELOW. SAND SHALL BE CLEAN, HARD, DURABLE, WASHED FREE FROM SILT, LOAM OR CLAY.
1. GRADE BEAMS, STEM WALLS, RETAINING WALLS AND OTHER CONCRETE FRAMEWORK LESS THAN 10" WIDE SHALL USE 3/4" AGGREGATE CONFORMING TO ASTM C33 WITH 100% PASSING 1" SIEVE AND 90% (MINIMUM) PASSING 3/4" SIEVE. TO ENSURE PROPER CONCRETE COVER AND CONSOLIDATION.
2. FOOTING AND SLABS ON GRADE, INCLUDING POST-TENSION AND MAT SLABS, SHALL USE 1" AGGREGATE CONFORMING TO ASTM C33 WITH 95% (MINIMUM) PASSING 1" SIEVE.
- B. MIXING WATER SHALL BE CLEAN AND FREE FROM INJURIOUS AMOUNTS OF OIL, ACIDS, ALKALIES, ORGANIC MATERIALS OR OTHER DELETERIOUS SUBSTANCES.
- C. CONCRETE EXPOSED TO SULFATE SHALL USE TYPE V CEMENT WITH POZZOLAN.
- D. FLY ASH OR POZZOLANS, IF USED, SHALL CONFORM WITH ASTM C618, COAL FLY ASH AND RAW OR CALCINED NATURAL POZZOLAN FOR USE AS A MINERAL ADMXTURE IN CONCRETE. USAGE SHALL NOT EXCEED 25 PERCENT, BY WEIGHT OF THE TOTAL CEMENTITIOUS MATERIALS. WHEN POZZOLANS ARE USED TO MITIGATE THE EFFECT OF SULFATE CONTAINING SOILS THEY SHALL BE OF A TYPE THAT HAS DEMONSTRATED SUCH ABILITY BY TEST OR SERVICE RECORD.
- E. ADMXTURES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- F. CEMENT SHALL CONFORM WITH ASTM C150 & C 595, PORTLAND CEMENT AND BLENDED HYDRAULIC CEMENTS
- G. TRANSIT MIX SHALL BE PER ASTM C-94.
- H. MIX DESIGN SHALL MEET THE RECOMMENDED SPECIFICATION UNLESS AN ALTERNATE MIX IS SUBMITTED AND REVIEWED BY THE ENGINEER.
2. SPECIAL INSPECTION OF CONCRETE WORK IS REQD WHERE NOTED ABOVE. WHEN REQD, SPECIAL INSPECTION SHALL INCLUDE THE INSPECTION OF THE PLACEMENT OF REINFORCEMENT, AND THE INSPECTION OF THE CONCRETE PLACEMENT OPERATIONS AS WELL AS CONCRETE CYLINDER TESTS, PER C.B.C. SECTION 1705.3. THE FOLLOWING THREE CRITERIA ESTABLISH THE REQD MINIMUM SAMPLING FREQUENCY FOR EACH CLASS OF CONCRETE:
- A. ONE EACH DAY A GIVEN CLASS IS PLACED, NOR LESS THAN
- B. ONCE FOR EACH 150 yd OF EACH CLASS PLACED EACH DAY, NOR LESS THAN
- C. ONCE FOR EACH 5000 ft OF SLAB OR WALL SURFACE AREA PLACED EACH DAY.
3. SPLICES OF CONTINUOUS REINFORCEMENT SHALL HAVE A MINIMUM LAP PER DETAILS 202 AND 203 UNLESS NOTED OTHERWISE. ALL REINFORCING STEEL SHALL BE SECURELY WIRED AND PROPERLY SUPPORTED ABOVE GROUND, AND AWAY FROM FORMS. REINFORCING BAR FABRICATION, LAPS AND PLACEMENT SHALL CONFORM TO THE MANUAL OF STANDARD PRACTICE OF THE CONCRETE REINFORCING STEEL INSTITUTE.
4. REINFORCING SHALL BE NEW STOCK, DEFORMED BARS, NO. 3 AND SMALLER: GRADE 40 CONFORMING TO ASTM A-615 AS FOLLOWS (U.N.O.) : NO. 4 AND LARGER: GRADE 60
- A. ALL BARS TO BE WELDED SHALL MEET THE REQUIREMENTS OF ASTM A706, GRADE 60.
- B. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, FLAT SHEETS ONLY. LAP FABRIC 6" MINIMUM.
- C. REINFORCEMENT PLACEMENT SHALL CONFORM TO DETAIL A
- D. ANCHOR BOLTS SHALL BE ASTM A307, U.N.O. AND SHALL CONFORM TO DETAIL 201 SD2
5. REINFORCEMENT COVER: ALL DIMENSIONS SHOWING THE LOCATIONS OF REINFORCEMENT STEEL NOT NOTED AS "CLEAR", ARE TO THE CENTER OF THE STEEL. MINIMUM CLEAR COVERAGE OF REINFORCEMENT SHALL BE AS FOLLOWS:
- A. CONCRETE CAST AGAINST EARTH, EXCEPT SLABS ON GRADE: 3" 1-1/2" U.N.O.
- B. CONCRETE CAST IN FORMS, BUT EXPOSED TO EARTH OR WEATHER: No. 5 REINFORCING AND SMALLER: 1-1/2" No. 6 REINFORCING AND LARGER: 2"
- C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND: SLABS, WALLS AND JOISTS: 3/4" BEAMS AND COLUMNS: 1-1/2"
6. UNFORMED CONCRETE SURFACE CURING:
- A. CURE FOR ONE TO SEVEN DAYS BY MAINTAINING TEMPERATURE ABOVE 50 DEGREES FAHRENHEIT, AND IN A MOIST CONDITION.
- B. APPLY MEMBRANE-FORMING CURING COMPOUND TO DAMP CONCRETE IMMEDIATELY AFTER COMPLETION OF THE MOIST-CURING PERIOD.
7. THE CONTRACTOR SHALL INFORM THE ENGINEER AT LEAST TWO DAYS PRIOR TO POURING ANY STRUCTURAL CONCRETE SO THAT OBSERVATION OF THE WORK MAY BE PERFORMED AS REQUIRED BY THE ENGINEER'S CONTRACT OR THE CODE.
8. FOOTING/GRADE BEAM CONSTRUCTION JOINTS SHALL CONFORM TO DETAIL 204 SD2
9. CRACK CONTROL JOINTS SHALL BE PLACED IN CONCRETE SLABS ON GRADE AT A SPACING OF 12'-0" MAX. O.C. EACH WAY (U.N.O. ON PLAN) PER DETAIL 205A SD2
- LOCATION OF SLAB ON GRADE CONSTRUCTION JOINTS SHALL BE APPROVED BY THE ENGINEER PRIOR TO CONSTRUCTION. SLAB ON GRADE CONSTRUCTION JOINTS SHALL CONFORM TO 205B SD2
10. ALL PIPES AND DUCTS THROUGH CONCRETE SHALL BE SLEEVED. VERIFY OPENINGS WITH PLUMBER AND ELECTRICIAN. SEE DETAIL 206 SD2
11. IF SPECIFIED, WELDED WIRE FABRIC SHALL BE 6x6-W1.4xW1.4. WIRE FABRIC SHALL BE ELECTRICALLY WELDED STEEL PER ASTM A185. LAP 6" MINIMUM AT ALL EDGES AND TIE AT THREE PLACES TO REINFORCING DOWELS (WHERE OCCUR) EXCEPT LOCATIONS WHERE SLAB IS INDEPENDENT OF FOUNDATION. CONTRACTOR SHALL PROVIDE SUPPORT CHAIRS TO ENSURE FABRIC IS LOCATED IN THE CENTER OF THE SLAB.
12. WELDING OF REINFORCING BARS SHALL CONFORM TO AWS D1.4 USING ASTM A706 REINFORCING BAR SPECIFICATIONS.

STRUCTURAL STEEL NOTES

1. THE FOLLOWING SECTION APPLIES TO ALL STRUCTURAL STEEL 1/8" THICK OR LARGER.
2. ALL WORK SHALL BE IN ACCORDANCE WITH THE CBC CHAPTER 22, AISC 15th EDITION, AND THE 2015 A.W.S. D1.1.
3. STEEL MATERIALS SHALL CONFORM TO THE FOLLOWING:
- A. PLATES AND SHAPES, U.N.O. ASTM A992 (fy = 50 ksi), OR ASTM A572, GRADE 50
- B. STRUCTURAL TUBE COLUMNS AND BEAMS: ASTM A500, GRADE B.
- C. PIPE COLUMNS: ASTM A53, TYPE E OR S, GRADE B
- D. ROD ASTM A36 (fy = 36 ksi)
4. FASTENERS SHALL CONFORM TO THE FOLLOWING:
- A. ANCHOR BOLTS: ASTM A307, U.N.O.
- B. STEEL TO WOOD CONNECTIONS: ASTM A307, U.N.O. USE CUT WASHERS (IF CONCEALED) AND MALLEABLE IRON WASHERS (IF EXPOSED)
5. WELDING ELECTRODES SHALL CONFORM TO 2015 A.W.S. D1.1 AND SHALL BE LOW HYDROGEN MATCHING FILLER METAL.
6. SURFACES OF STEEL TO BE FIELD WELDED SHALL BE FREE AND CLEAR OF ALL PAINT, DIRT, GREASE, OR OTHER DELETERIOUS COATINGS.
7. STEEL FRAMING, EXCEPT THOSE PORTIONS TO BE EMBEDDED IN CONCRETE, CONCEALED IN FRAMING, FIELD WELDED, OR HIGH STRENGTH BOLTED SHALL BE SHOP-PAINTED PER THE SPECIFICATIONS.
8. FIELD PAINT ALL EXPOSED STEEL SURFACES AFTER INSTALLATION, PER THE SPECIFICATIONS.
9. ALL WELDERS SHALL BE QUALIFIED BY A.W.S. PROCEDURES FOR THE REQUIRED WELDING.
10. SUBMIT CERTIFICATION OF COMPLIANCE FOR ALL STEEL MATERIALS.

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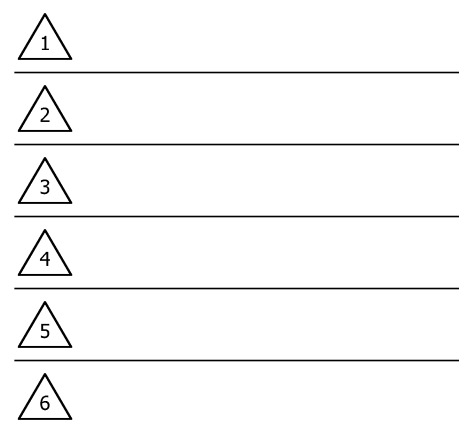
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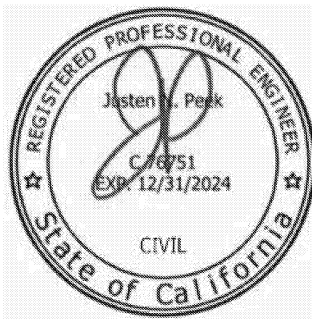
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Engineering Seal:



Sheet Description:

GENERAL STRUCTURAL
SPECIFICATIONS AND NOTES

No Scale

Initial Issue Date: March 24, 2023

Drawn By: E. Bennett

Project Designer: J. Peek

Project Manager: J. Peek Ext. 23

Job No. W021420

Sheet No.

SN2

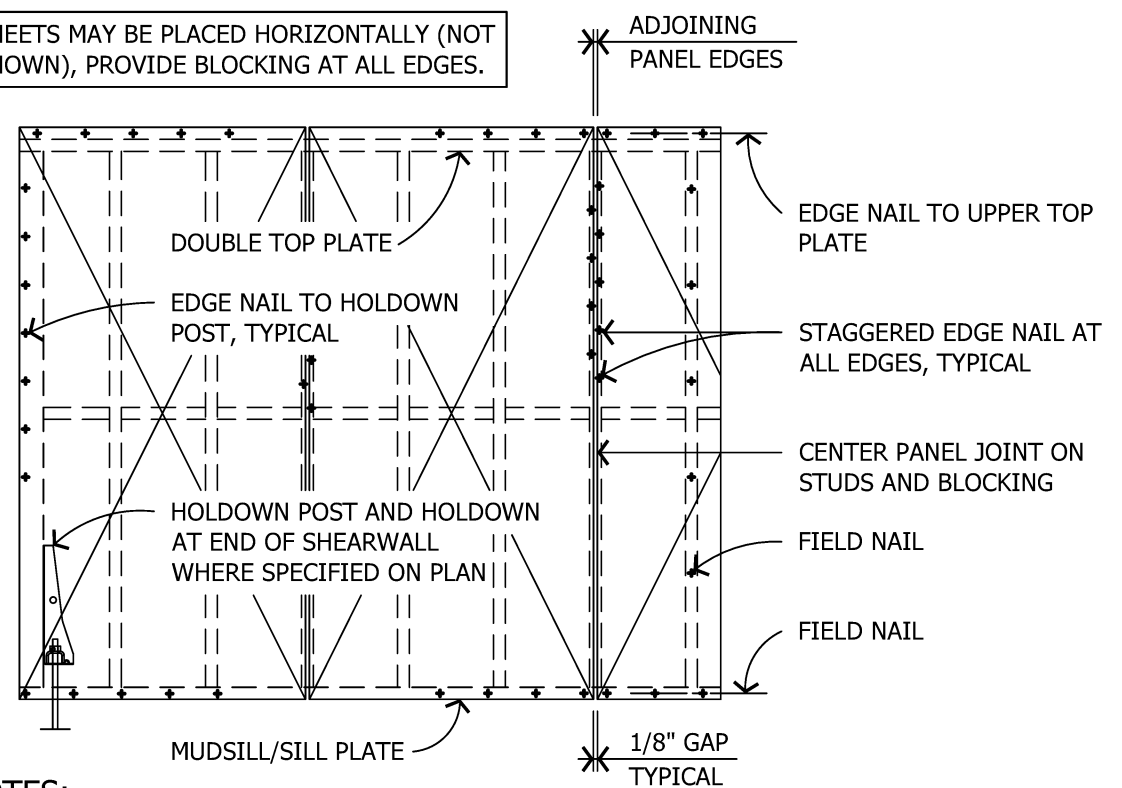


1. SEE ARCHITECTURAL DRAWINGS FOR STUD SIZES AND OTHER REQUIREMENTS.
2. SEE ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, ELEVATIONS, ETC. THE PROJECT ARCHITECT IS RESPONSIBLE FOR SPECIFYING DIMENSIONS TO ALL PROJECT ELEMENTS. THE DIMENSIONS SHOWN ON THE FOUNDATION PLANS SHALL NOT BE USED FOR CONSTRUCTION. THE FOUNDATION SHALL BE CONSTRUCTED USING THE DIMENSIONS SHOWN ON THE ARCHITECTURAL PLANS.
3. DETAIL KEYS AND OTHER INFORMATION ARE TYPICAL.
4. THE FOUNDATION SHALL CONSIST OF A UNIFORM THICKNESS REINFORCED CONCRETE STRUCTURAL SLAB PER DETAIL (A) SUPPORTED BY DRILLED CONCRETE PIERS PER (B) SD3.
 - A. STEP FOUNDATION AS SD3 REQUIRED FOR FLOOR ELEVATION CHANGES AND COMPLIANCE WITH DETAILS. THE CONTRACTOR SHALL GRADE THE SITE TO PROVIDE PROPER SURFACE DRAINAGE AWAY FROM THE FOUNDATION AT ALL LOCATIONS. ROOF GUTTER DOWNSPOUTS SHALL NOT DISCHARGE NEAR THE FOUNDATION.
5. CONCRETE TIE BEAMS SHALL BE 12" x 18" AND SHALL BE REINFORCED WITH (2) #4 HORIZ. REINFORCING, TOP AND BOTTOM. TIE BEAM REINFORCEMENT SHALL CONTINUE THROUGH FOUNDATION ELEMENTS OR SHALL BE PROPERLY TIED TO FOUNDATION REINFORCING.
6. LANDSCAPE SLABS SHALL BE INDEPENDENT OF THE FOUNDATION. THE CONTRACTOR SHALL INSTALL ASPHALT-IMPREGNATED EXPANSIVE JOINT MATERIAL BETWEEN SLABS AND THE FOUNDATION.
7. HOLDOWNS:
HOLDOWNS NOTED OCCUR AT THE LEVEL OF FRAMING SHOWN FOR CONNECTION OF WALLS ABOVE TO FOOTING.
 - A. ALL HOLDOWN MEMBERS IN CONTACT WITH P.T. MATERIAL SHALL BE HOT-DIP GALVANIZED. USE HOT-DIPPED GALVANIZED ANCHOR BOLTS WITH 3" x 3" x 1/4" PLATE WASHERS. PLAIN CARBON STEEL FASTENERS IN SBX/DOT AND ZINC BORATE PRESERVATIVE-TREATED WOOD SHALL BE PERMITTED.
 - B. ALL HOLDOWNS SHALL BE INSTALLED WITH ANCHORS PER DETAIL (112) SD1.
 - C. CONTRACTOR SHALL REVIEW HOLDOWN ANCHOR REQUIREMENTS FOR EMBEDMENT CONDITIONS THAT REQUIRE DEEPENED STEM WALL. REFER TO (112) SD1 FOR REQUIREMENTS.
 - D. PROVIDE POSTS AT ALL HOLDOWNS AS SHOWN IN DETAIL (112) SD1 WHERE DOUBLE 2x POSTS ARE USED, SISTER WITH 16d FACE NAILS AT SILL NAILING PER SHEARWALL SCHEDULE. WHERE 3x MEMBERS AT PANEL EDGES ARE REQUIRED, USE 4x MINIMUM POST FOR HOLDOWN PER FRAMER SHALL CONFIRM LAYOUT BEFORE CONCRETE IS POURED. NOTIFY ENGINEER OF CONFLICTS PRIOR TO PROCEEDING.
8. CONTRACTOR SHALL REVIEW HOLDOWN ANCHOR REQUIREMENTS FOR EMBEDMENT CONDITIONS THAT REQUIRE DEEPEMED FTG. REFER TO (112) SD1 FOR REQUIREMENTS.
9. REFER TO THE PROJECT SOILS REPORT IF APPLICABLE. FOR ADDITIONAL FOUNDATION AND SITE CONSTRUCTION REQUIREMENTS. ALL FOUNDATION ELEMENTS SHALL COMPLY WITH ALL CONDITIONS CONTAINED WITHIN APPROPRIATE SOILS REPORT AND REQUIREMENTS OF 2016 CBC CHAPTER 18. THE RESPONSIBLE SOILS ENGINEER IF APPLICABLE, SHALL OBSERVE ALL LAB AND FOUNDATION SUBGRADE PRIOR TO PLACING CONCRETE. SEE SOILS REPORT FOR SPECIFIC INSPECTION REQUIREMENTS.
10. ALL FOUNDATION PLATES AND MUDDSILLS SHALL BE PRESSURE-TREATED DOUGLAS FIR MARKED OR BRANDED BY AN APPROVED AGENCY. SEE SHEARWALL SCHEDULE FOR SHEARWALL LOCATIONS THAT REQUIRE 3x MINIMUM MUDDSILLS. FOUNDATION PLATES AND MUDDSILLS SHALL BE BOLTED TO THE FOUNDATION WITH NOT LESS THAN 5/8" DIA. HOT-DIPPED GALVANIZED ANCHOR BOLTS WITH 3" x 3" x 1/4" PLATE WASHERS. PLAIN CARBON STEEL FASTENERS IN SBX/DOT AND ZINC BORATE PRESERVATIVE-TREATED WOOD SHALL BE PERMITTED. EMBED ANCHOR BOLTS 2" INTO CONCRETE FOUNDATION, WALL OR 12" INTO GROUTED MASONRY, AND SPACE NOT MORE THAN 4'-0" APART, UNLESS NOTED OTHERWISE ON THE SHEARWALL SCHEDULE. THERE SHALL BE A MINIMUM OF TWO BOLTS PER BOARD WITH ONE BOLT LOCATED 12" MAXIMUM AND 4-3/8" MINIMUM OF EACH END. FOR ADDITIONAL INFORMATION SEE DETAIL (201) SD2.
11. SEE GENERAL NOTES FOR ADDITIONAL REQUIREMENTS. ALL DETAIL CALLOUTS SHALL BE CONSIDERED TYPICAL. CONTRACTOR SHALL REVIEW THE DETAIL SHEETS FOR DETAILS NOT SPECIFICALLY REFERENCED.
12. SHEARWALLS:
LENGTHS OF SHEARWALLS ARE SHOWN ABOVE OR BELOW SHEARWALL NUMBER (SEE PLAN). THE NUMBERS INDICATE THE MINIMUM DESIGN LENGTH OF THE SHEARWALL. SEE DETAIL (103) SD1.

1. WALLS SHOWN AS SOLID ARE BELOW FRAMING. SEE ARCHITECTURAL DRAWINGS FOR STUD JUMPS AND OTHER REQUIREMENTS.
2. SEE ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS, ELEVATIONS, ETC. THE PROJECT ARCHITECT IS RESPONSIBLE FOR SPECIFYING DIMENSIONS TO ALL PROJECT ELEMENTS.
3. DETAIL KEYS AND OTHER INFORMATION ARE TYPICAL.
4. **ROOF FRAMING TERMS:**
ALL BEAMS SHALL BE CONSIDERED FLUSH, (TOP OF BEAM EQUAL TO TOP OF ROOF FRAMING AND BOTTOM OF ROOF FRAMING) UNLESS NOTED OTHERWISE ON PLANS AS ONE OF THE FOLLOWING:
 - A. **DROPPED:** BEAM IS SET BELOW ROOF FRAMING WITH TOP OF BEAM SET AT BOTTOM OF ROOF FRAMING.
 - B. **DOWNSSET:** BEAM IS DEEPER THAN THE ROOF FRAMING AND SET WITH TOP OF BEAM EQUAL TO TOP OF ROOF FRAMING. BOTTOM OF BEAM EXTENDS BELOW ROOF FRAMING.
 - C. **UPSET:** BEAM IS DEEPER THAN THE ROOF FRAMING AND SET WITH BOTTOM OF BEAM EQUAL TO BOTTOM OF ROOF FRAMING. TOP OF BEAM EXTENDS ABOVE ROOF FRAMING.
 - D. **"HDR":** BOTTOM OF HEADER IS SET AT ROUGH OPENING (R.O.). SEE ARCH. FOR HEIGHT. IF HEADER BREAKS, TOP PLATES INSTALL C514 x 36" STRAP FROM FACE OF PLATE TO FACE OF HEADER, TYPICAL AT EACH END OF HEADER.
5. **ROOF FRAMING:**
ROOF SHALL BE FRAMED WITH PRE-MANUFACTURED ROOF TRUSSES AT 24" O.C. INSTALLED PER APPROVED ROOF TRUSS SHOP DRAWINGS. ROOF TRUSS MANUFACTURER SHALL DESIGN THE ROOF TRUSSES AS SPECIFIED IN THE GENERAL NOTES (SHEET SN-2, U.N.O.). SEE ROOF TRUSS DETAILS DETAILS ON SHEET SD4 FOR TYPICAL DETAILING.
 - A. ROOF TRUSSES WITH FLAT BOTTOM CHORDS SHALL BE SECURED TO END BEARING POINTS WITH "H1" CLIP TO TOP PLATE. ROOF TRUSSES WITH SLOPED BOTTOM CHORDS SHALL BE SECURED TO BEARING POINTS WITH SIMPSON "TC" CLIPS OR EQUIVALENT CONNECTORS THAT ALLOW HORIZONTAL MOVEMENT IN ACCORDANCE WITH THE ROOF TRUSS DESIGNER'S REQUIREMENTS.
 - B. PROVIDE 2x BLOCKING BELOW ALL HIPs, VALLEYS, AND RIDGES. CONTRACTOR SHALL INSTALL STRONGBACKS, CROSS BRIDGING, AND/OR BRACING AS SPECIFIED BY THE ROOF TRUSS SHOP DRAWINGS.
6. **MANUFACTURED BEAM FRAMING:**
ALL MANUFACTURED WOOD BEAMS SHALL BE MANUFACTURED BY "TRUS JOIST" WEYERHAEUSER. REFER TO "GENERAL FRAMING NOTES" ON SHEET SN1 FOR MATERIAL SPECIFICATIONS.
7. **ROOF SHEATHING:**
TYPICAL ROOF SHEATHING SHALL BE 15/32" THICK (24/0 OR 32/16) CDX GRADE PLYWOOD (OR EQUIVALENT-RATED ORIENTED STRAND BOARD (O.S.B.) WITH GAP PER MANUFACTURER. ROOF SHEATHING SHALL BE NAILED WITH 8d AT 6" O.C. (EDGES) AND 8d AT 12" O.C. (FIELD), UNLESS NOTED OTHERWISE ON THE PLAN. ORIENT SHEATHING WITH FACE GRAIN PERPENDICULAR TO FRAMING MEMBERS, AND STAGGER END JOINTS.
 - A. PLYWOOD SHEETS LOCATED AT ROOF EDGES OR CHANGES IN FRAMING SHALL BE AT LEAST 24 INCHES WIDE OR SHALL BE EDGE BLOCKED AND NAILED. EDGE NAIL PLYWOOD TO ALL GABLE AND SHEAR TRUSSES (TRUSSES WITH DRAG LOADS).
 - B. EDGE NAIL ROOF PLYWOOD TO ALL DRAG MEMBERS (RAFTERS OR BEAMS w/ HORIZ. STRAPS).
8. **SUPPORT OF BEAMS, HIPs, GIRDERS, ETC.:**
INSTALL POSTS (POST SIZE TO MATCH BEAM AND WALL SIZE), MINIMUM BELOW ALL ROOF BEAMS AND GIRDERS. CONTRACTOR SHALL VERIFY FRAMING LAYOUT TO ENSURE CONTINUOUS AND SOLID BLOCKING UNDER ALL CONCENTRATED LOADS.
9. **CRICKETS AND/OR CALIFORNIA (BUILT-UP FRAMING):**
CRICKETS AND/OR CALIFORNIA FRAMING SHALL BE CONSTRUCTED WITH 2x6 MEMBERS AT 24" O.C. SUPPORTED TO THE ROOF BELOW AT 48 INCHES ON CENTER. ROOF PLY SHALL CONTINUE BELOW CRICKETS AND/OR CALIFORNIA FRAMING.
10. **HEADERS:**
WINDOW AND DOOR HEADERS SHALL CONFORM TO THE HEADER SCHEDULE, U.N.O. ON THE PLAN 210
SD2
11. **TOP PLATE SPLICES:**
TOP PLATE SPLICES OF ALL WALLS SHALL CONFORM TO DETAIL 209
SD2
INSTALL C514 x 3'-0" LONG STRAPS AT TOP PLATE SLOPE CHANGES AND/OR CONTINUOUS TOP PLATE CONDITIONS.
12. **TOP CONNECTORS SHALL BE INSTALLED AT EXTERIOR WALL TOP PLATES AT 4'-0" O.C. MINIMUM WHERE SHEAR IS NOT NOTED ON PLANS.**
13. SEE GENERAL NOTES FOR ADDITIONAL REQUIREMENTS. ALL DETAIL CALLOUTS SHALL BE CONSIDERED TYPICAL. THE CONTRACTOR SHALL REVIEW THE DETAIL SHEETS FOR DETAILS NOT SPECIFICALLY REFERENCED.
14. **SHEARWALLS AT OPENING TRIMMERS:**
WHERE SHEARWALL LENGTH SPECIFIED ON PLANS REQUIRES EDGE 108
SD1
NAILING TO WINDOW AND/OR DOOR TRIMMERS, REFER TO DETAIL.
15. **SHEARWALLS:**
LENGTHS OF SHEARWALLS ARE SHOWN ABOVE OR BELOW SHEARWALL NUMBER (SEE PLAN). THE NUMBERS INDICATE THE MIN. DESIGN LENGTH OF THE SHEARWALL. SEE 103
SD1
16. **HOLD-DOWN STRAPS**
WHERE HOLD-DOWN STRAPS ARE SPECIFIED ON PLANS, INSTALL PER DETAIL 112
SD1 113
SD1
- A. **ALL NAIL HOLES SHALL BE FILLED ON HOLD-DOWN STRAPS**



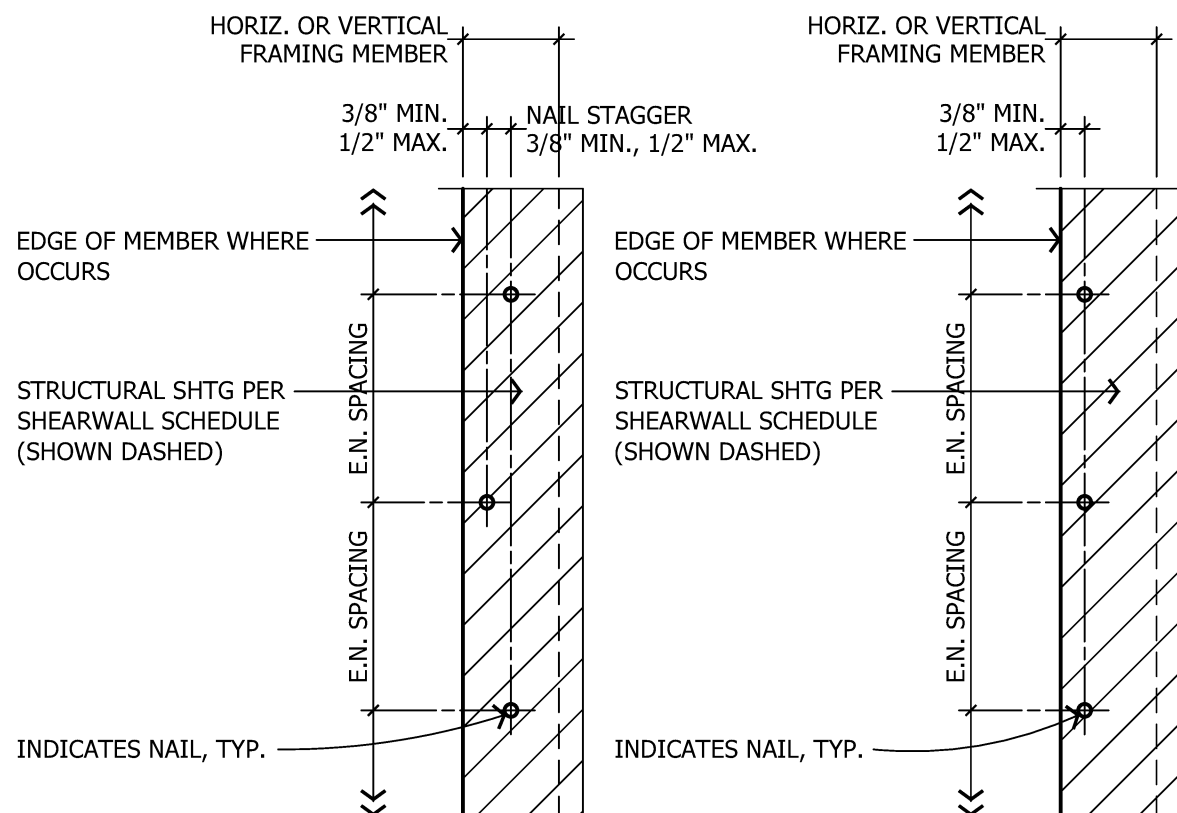
SHEETS MAY BE PLACED HORIZONTALLY (NOT SHOWN), PROVIDE BLOCKING AT ALL EDGES.



NOTES:

- SEE SHEARWALL SCHEDULE FOR REQUIRED SHEATHING, NAILING, AND SILL CONNECTIONS.
- FOR LOCATION OF TIEDOWNS AND TD POSTS, SEE TYPICAL DETAILS ON SHEET SD2.

101 TYP. SHEARWALL ELEVATION

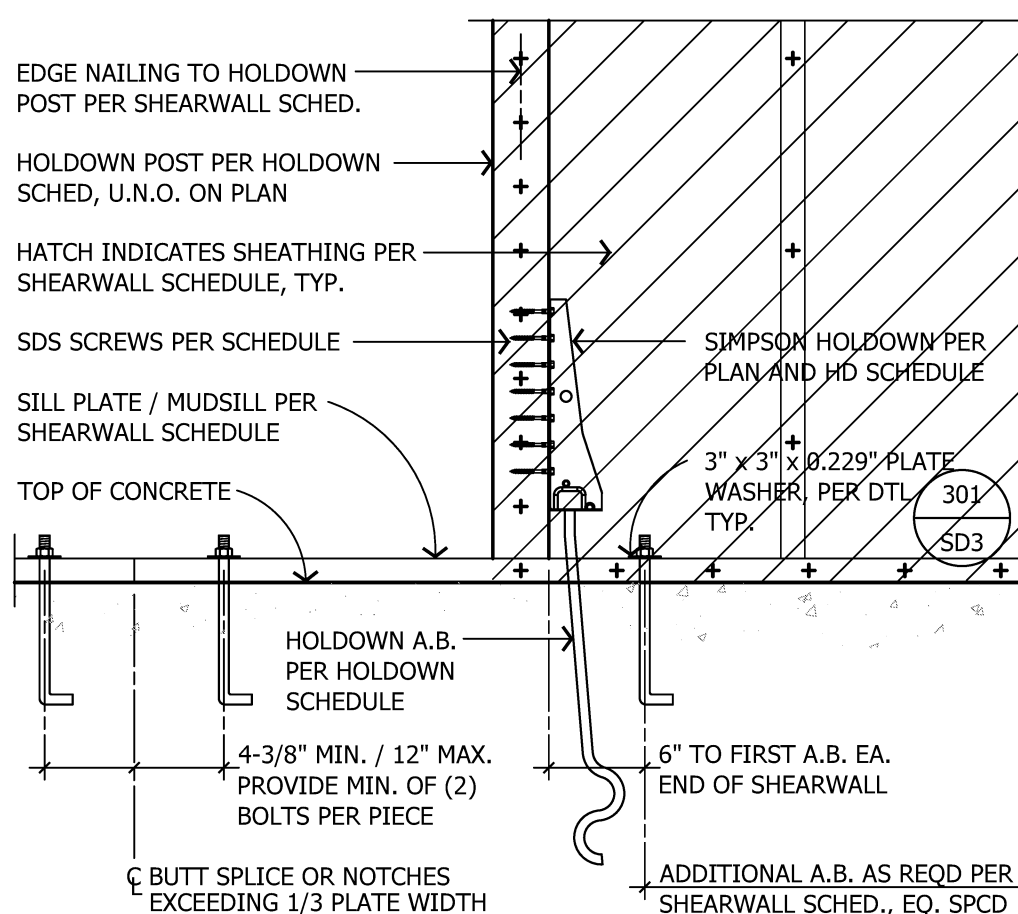


104 TYP. SHEARWALL EDGE NAIL PATTERN

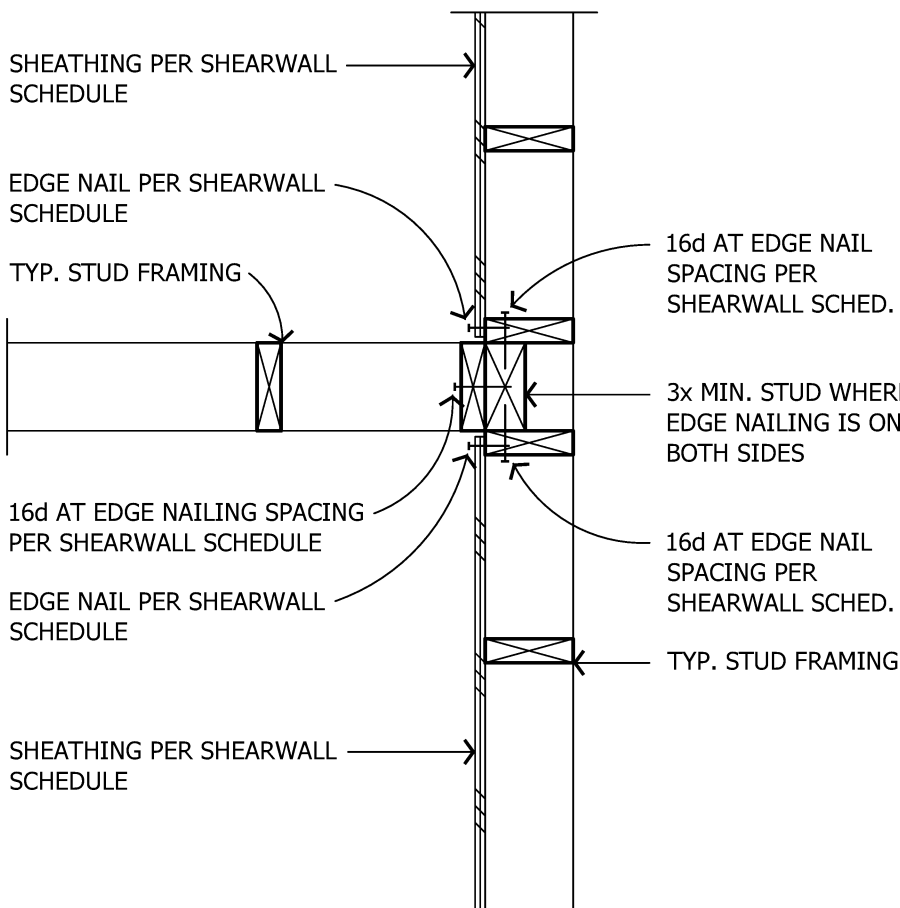
ALTERNATIVE MUDSILL ANCHORAGE				
THE FOLLOWING ALTERNATIVE ANCHORAGE MAY BE INSTALLED IN LIEU OF 5/8\"/>				
SHEARWALL DESIGNATION	"MASA" (NOTE 1.)	"TITEN HD" (NOTE 2.)		
X\"/>	32\"/>	48\"/>		
X\"/>	22\"/>	36\"/>		
X\"/>	16\"/>	28\"/>		
X\"/>	10\"/>	24\"/>		
X\"/>	9\"/>	24\"/>		
X\"/>	8\"/>	24\"/>		
X\"/>	NOT ALLOWED	16\"/>		

- SIMPSON "MASA" ANCHORS SHALL BE INSTALLED PER ICC REPORT No. ESR-2555.
- SIMPSON TITEN HD ANCHORS SHALL BE INSTALLED PER ICC REPORT No. ESR-2713.

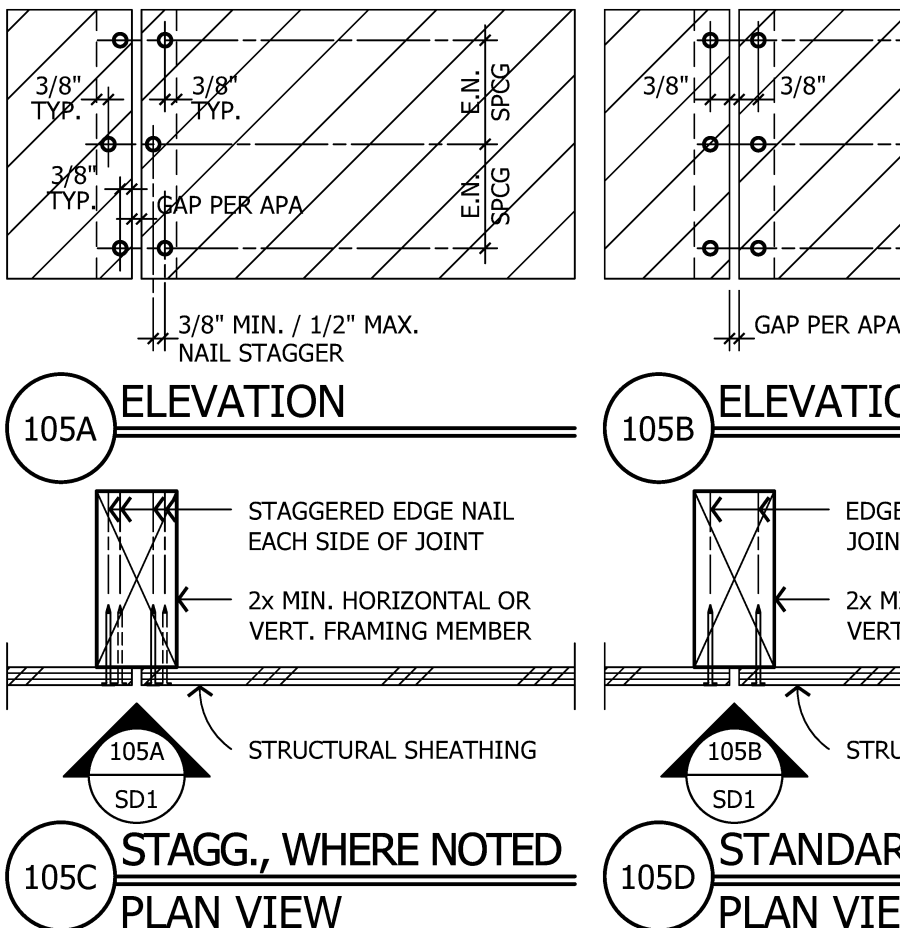
109 ALTERNATIVE MUDSILL ANCHORAGE



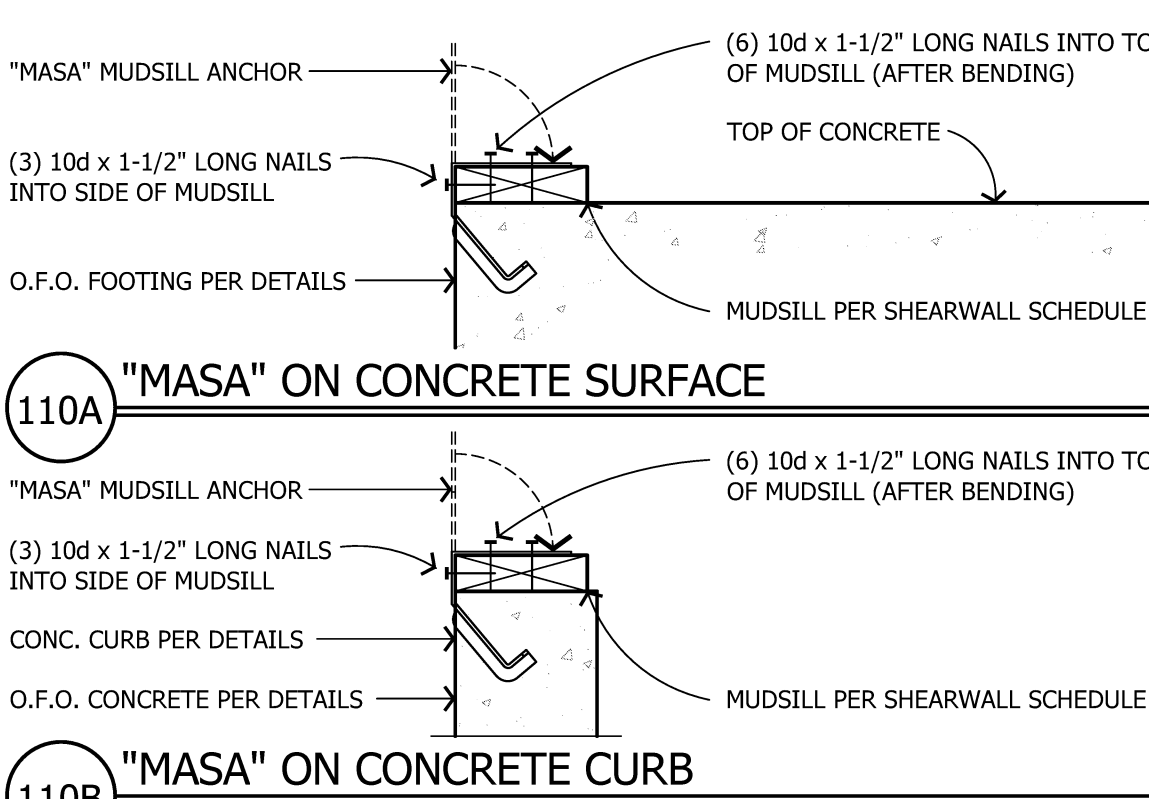
113 TYPICAL HOLDOWN AND HOLDOWN ANCHOR BOLT INSTALLATION



102 SHEARWALL INTERSECTION PLAN

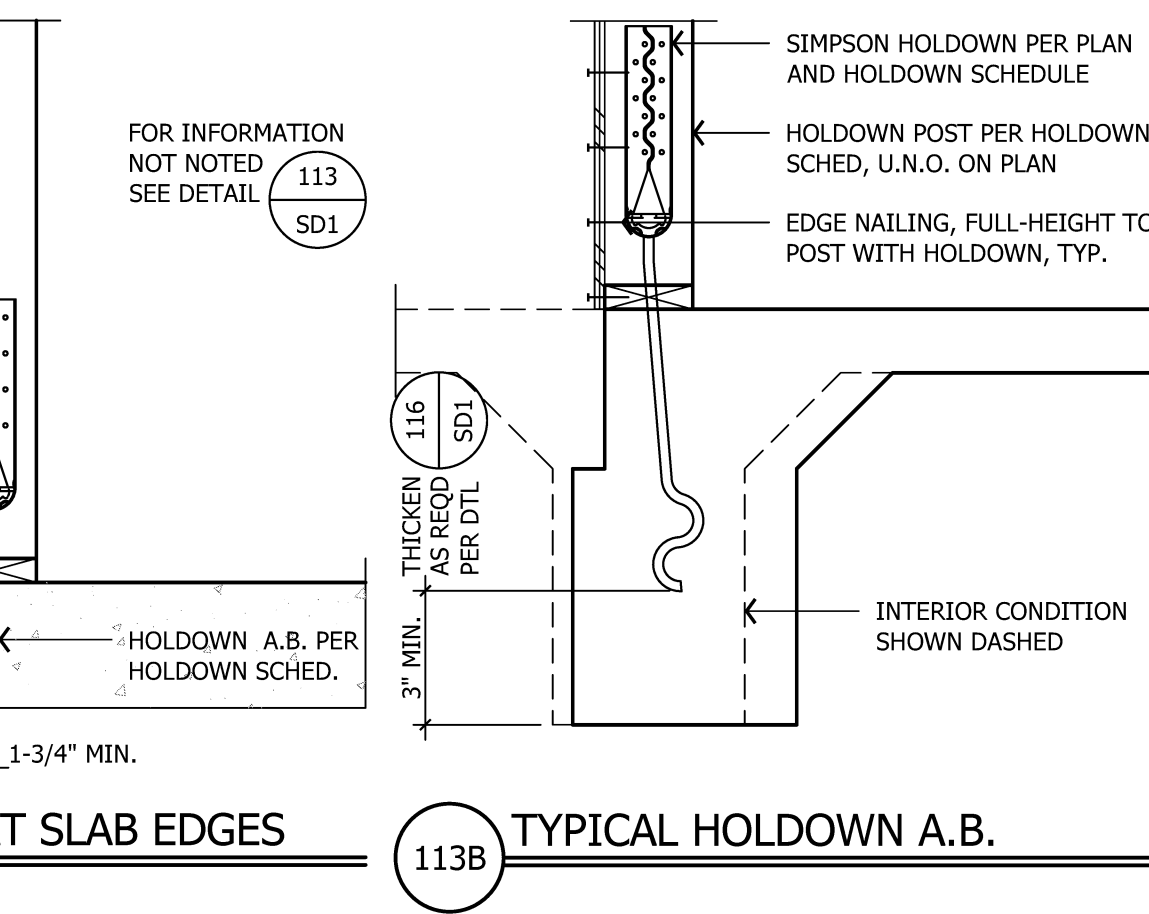


105 SHEARWALL SHEATHING AT JOINT



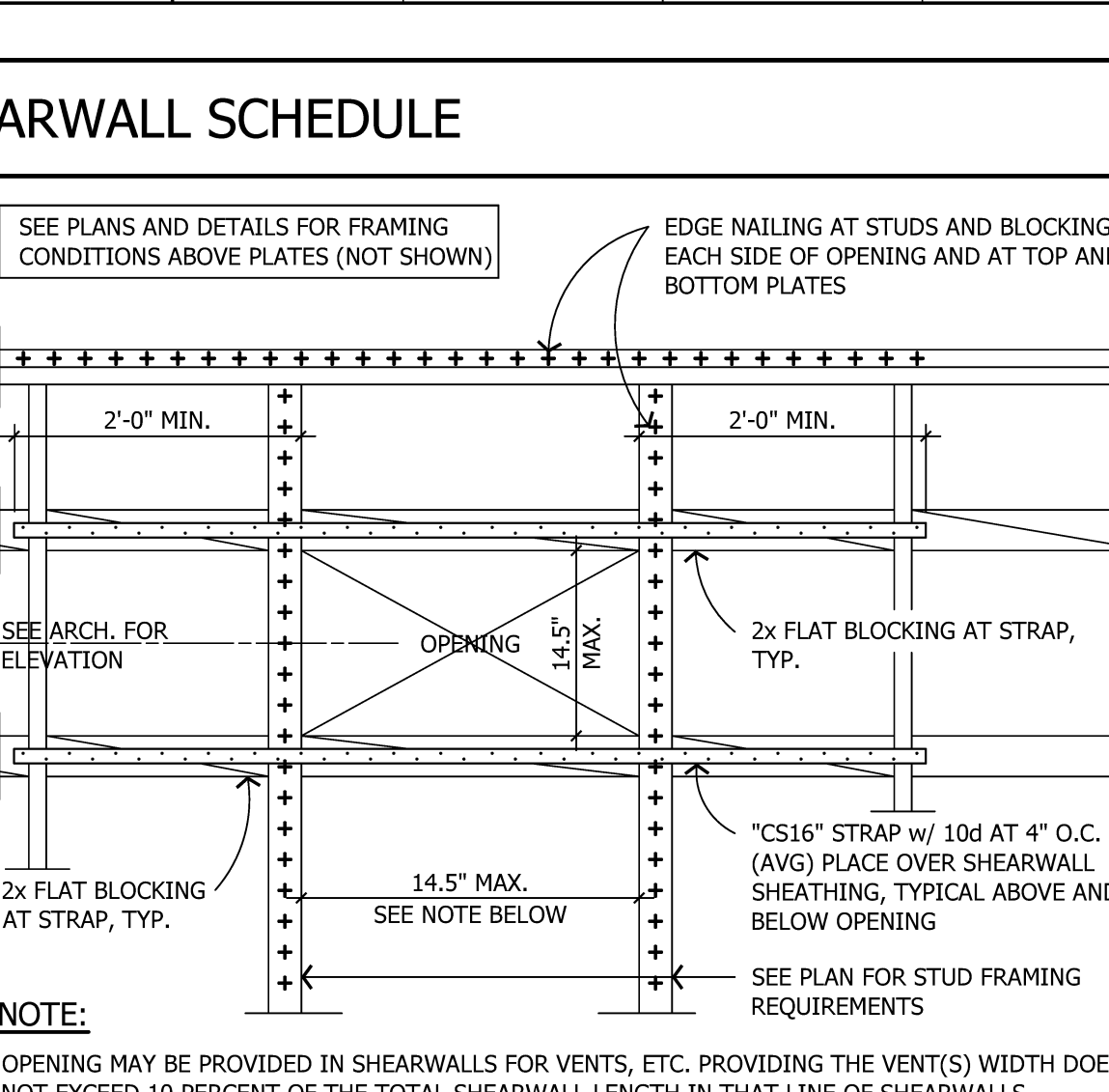
- FULL LOADS APPLY FOR SPALLS UP TO A MAX OF 1-1/4\"/>
- MINIMUM "MASA" END DISTANCE IS 4\"/>

110 MASA ANCHORAGES

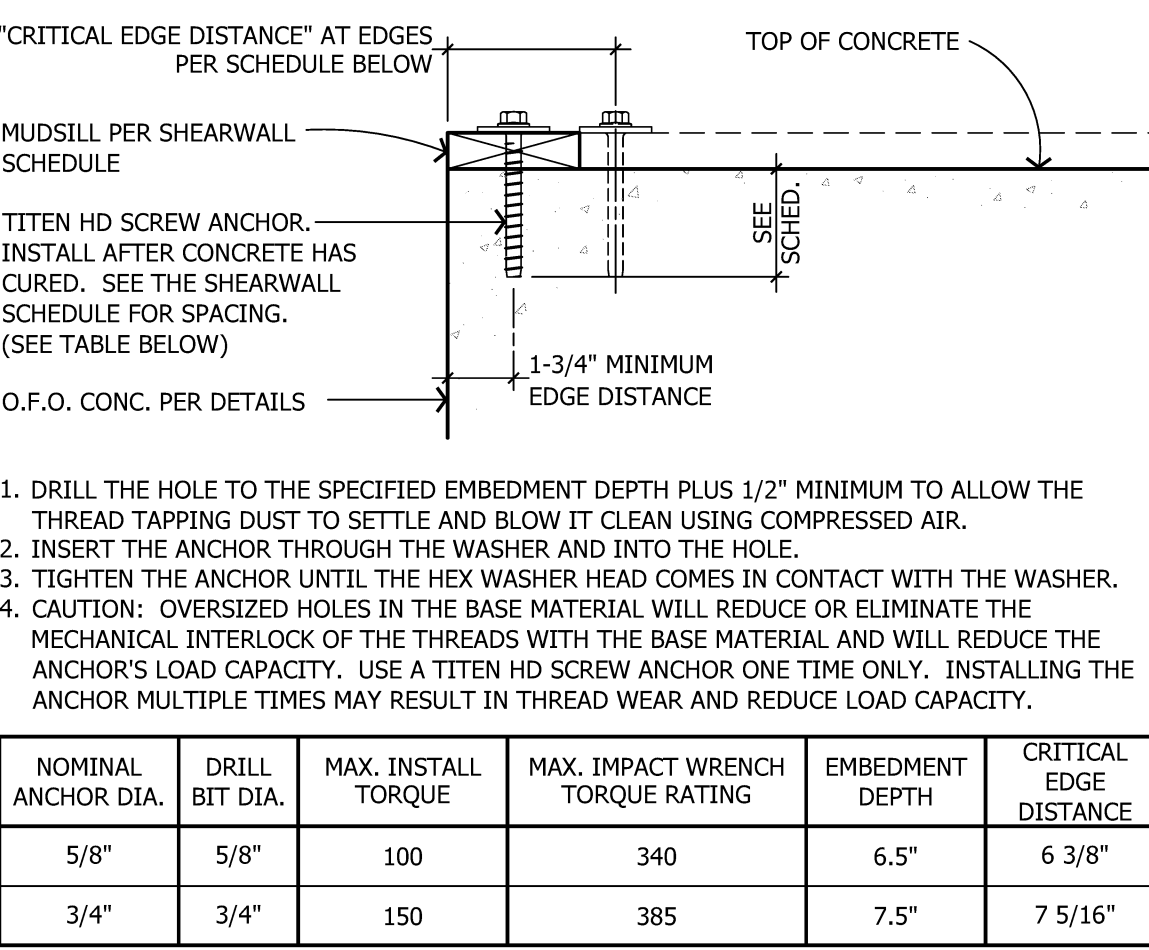


113A AT SLAB EDGES

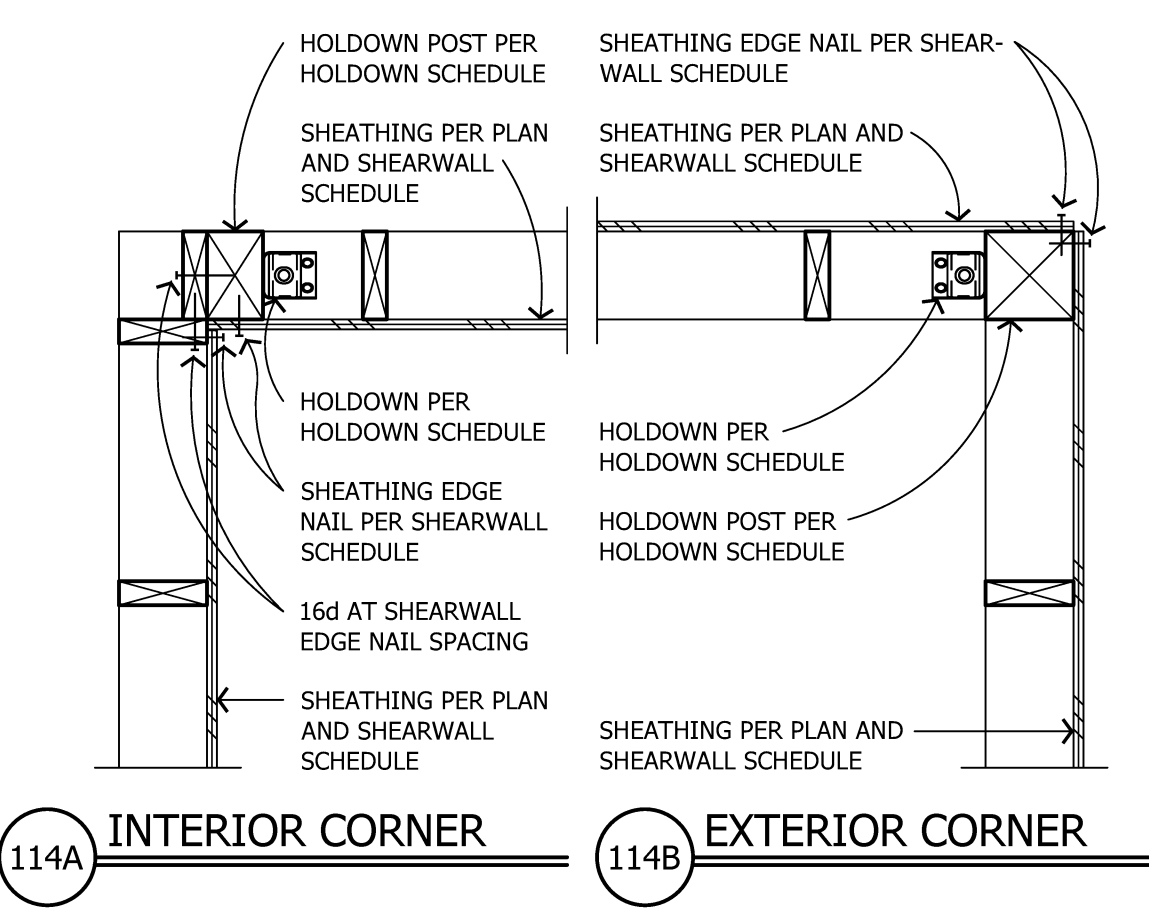
1/2\"/>	SHEARWALL LENGTH: SHEARWALL DESIGNATION:	X\"/>	X\"/>	X\"/>	X\"/>	X\"/>
		SW-1	SW-2	SW-3	SW-4	SW-5
	PLYWOOD OR O.S.B. SHEATHING THICKNESS:	15/32\"/>	15/32\"/>	15/32\"/>	15/32\"/>	15/32\"/>
	10d EDGE NAIL (0.148 DIA.):	10d @ 6\"/>	10d @ 4\"/>	10d @ 3\"/>	10d @ 2\"/>	10d @ 2\"/>
	10d FIELD NAIL (0.148 DIA.):	10d @ 12\"/>	10d @ 12\"/>	10d @ 12\"/>	10d @ 12\"/>	10d @ 12\"/>



106 SMALL OPENING IN SHEARWALL

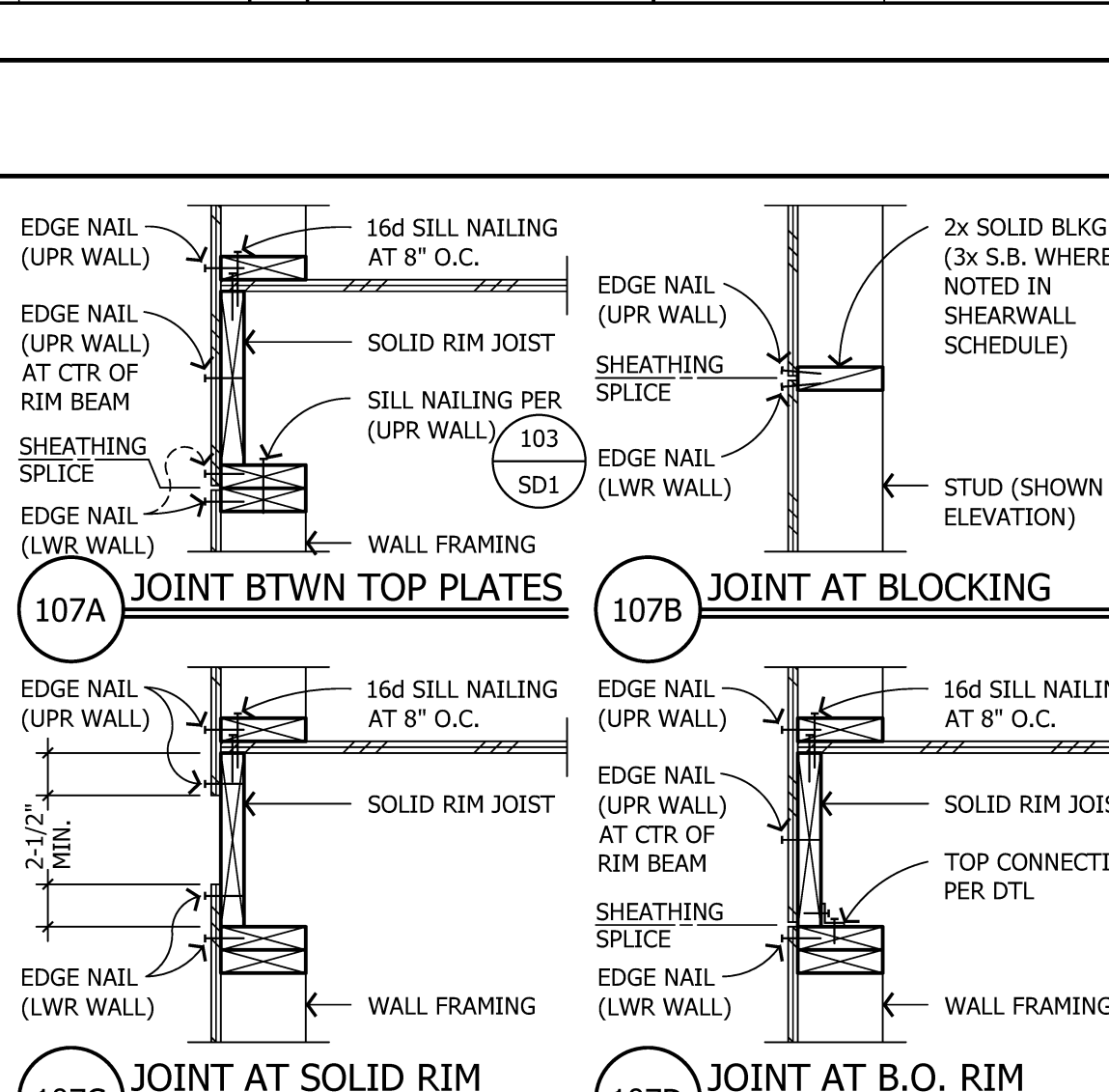


111 \"TITEN HD\" ANCHORAGE & NOTES



114 HOLDOWN AT SHEARWALL CORNER (PLAN)

1/2\"/>	SHEARWALL LENGTH: SHEARWALL DESIGNATION:	X\"/>	X\"/>	X\"/>
		SW-6	SW-7	SW-8
	PLYWOOD OR O.S.B. SHEATHING THICKNESS:	15/32\"/>	15/32\"/>	15/32\"/>
	10d EDGE NAIL (0.148 DIA.):	10d @ 4\"/>	10d @ 3\"/>	10d @ 2\"/>
	10d FIELD NAIL (0.148 DIA.):	10d @ 12\"/>	10d @ 12\"/>	10d @ 12\"/>



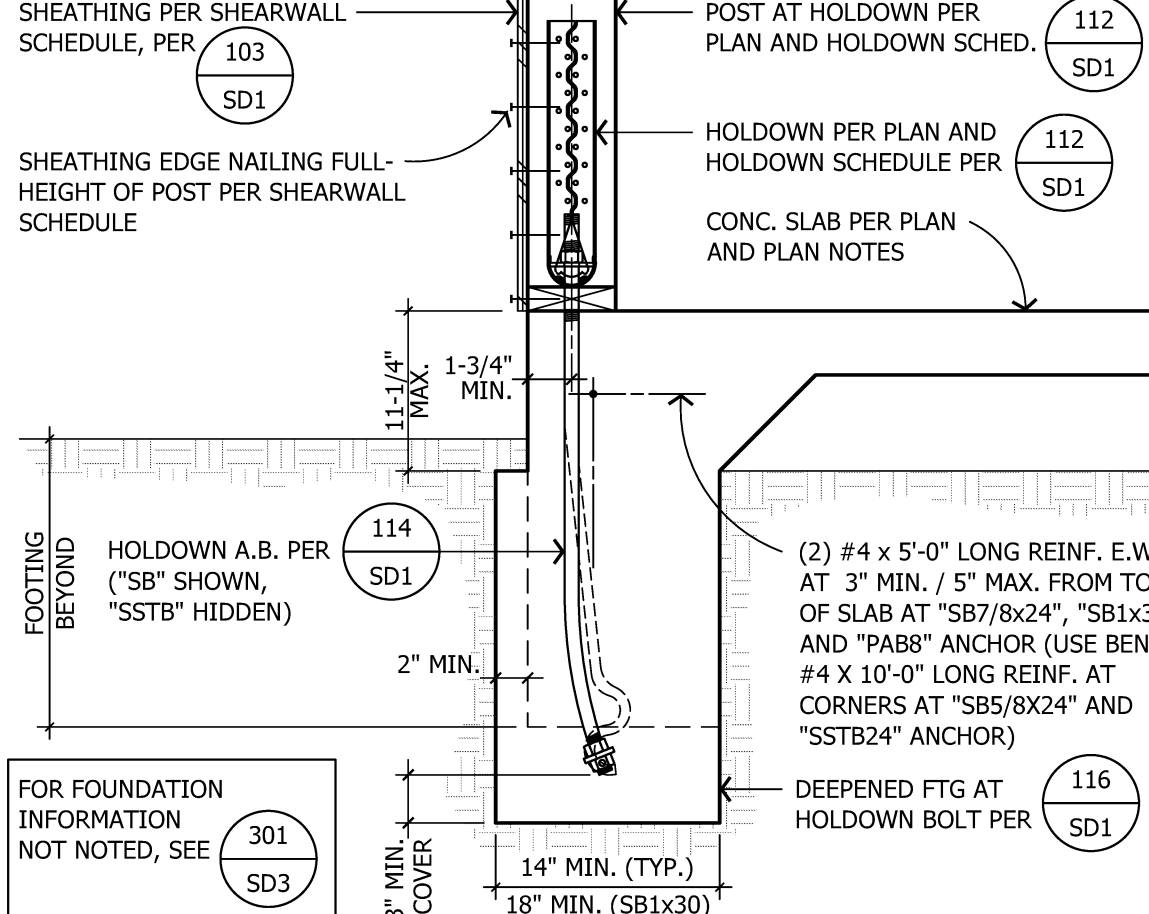
107 SHEATHING JOINT AT RIM OR BLKG

STRAP HOLDOWN SCHEDULE				
STRAP	END LENGTH	FASTENERS EA. END	MIN. POST SIZE	ALLOWABLE LOAD
\"CS14\"	15\"	(13) 10d (0.148 DIA. x 2-1/2\")	(2) 2x	2490 lbs
(2) \"CS14\"	15\"	(13) 10d (0.148 DIA. x 2-1/2\") EACH STRAP	4x4 / (2) 2x6	4980 lbs
\"CMST14\"	30\"	(33) 10d (0.148 DIA. x 2-1/2\")	4x6 / 6x6	6490 lbs
\"CMST12\"	39\"	(43) 10d (0.148 DIA. x 2-1/2\")	4x8 / 6x6	9215 lbs

- FILL ALL NAIL HOLES PER MANUFACTURER'S REQUIREMENTS.
- STRAPS SHALL BE INSTALLED OVER PLYWOOD SHEATHING.
- STRAPS MUST BE INSTALLED FLAT TO THE FACE OF THE POST. NO BENDS, NICKS OR SLACK OF ANY KIND ARE ALLOWED. STRAPS INSTALLED WITH ANY BENDS, NICKS OR SLACK SHALL BE CONSIDERED AS INADEQUATE AND MUST BE RE-INSTALLED.
- STRAPS MUST BE INSTALLED VERTICALLY WHEN USED AS A HOLDOWN. STRAPS INSTALLED ON A SLOPE OR AT AN ANGLE SHALL BE CONSIDERED AS INADEQUATE AND MUST BE RE-INSTALLED.
- LOADS INCLUDE A 60% LOAD DURATION INCREASE ON THE FASTENERS FOR WIND OR SEISMIC.
- CMST STRAPS ONLY - USE EVERY OTHER ROUND HOLE IF THE WOOD TENDS TO SPLIT. INCREASE STRAP LENGTH AS REQUIRED TO ACHIEVE MINIMUM NUMBER OF FASTENERS LISTED IN TABLE ABOVE.
- PRE-DRILL HOLES TO PREVENT WOOD SPLITTING AS REQUIRED.

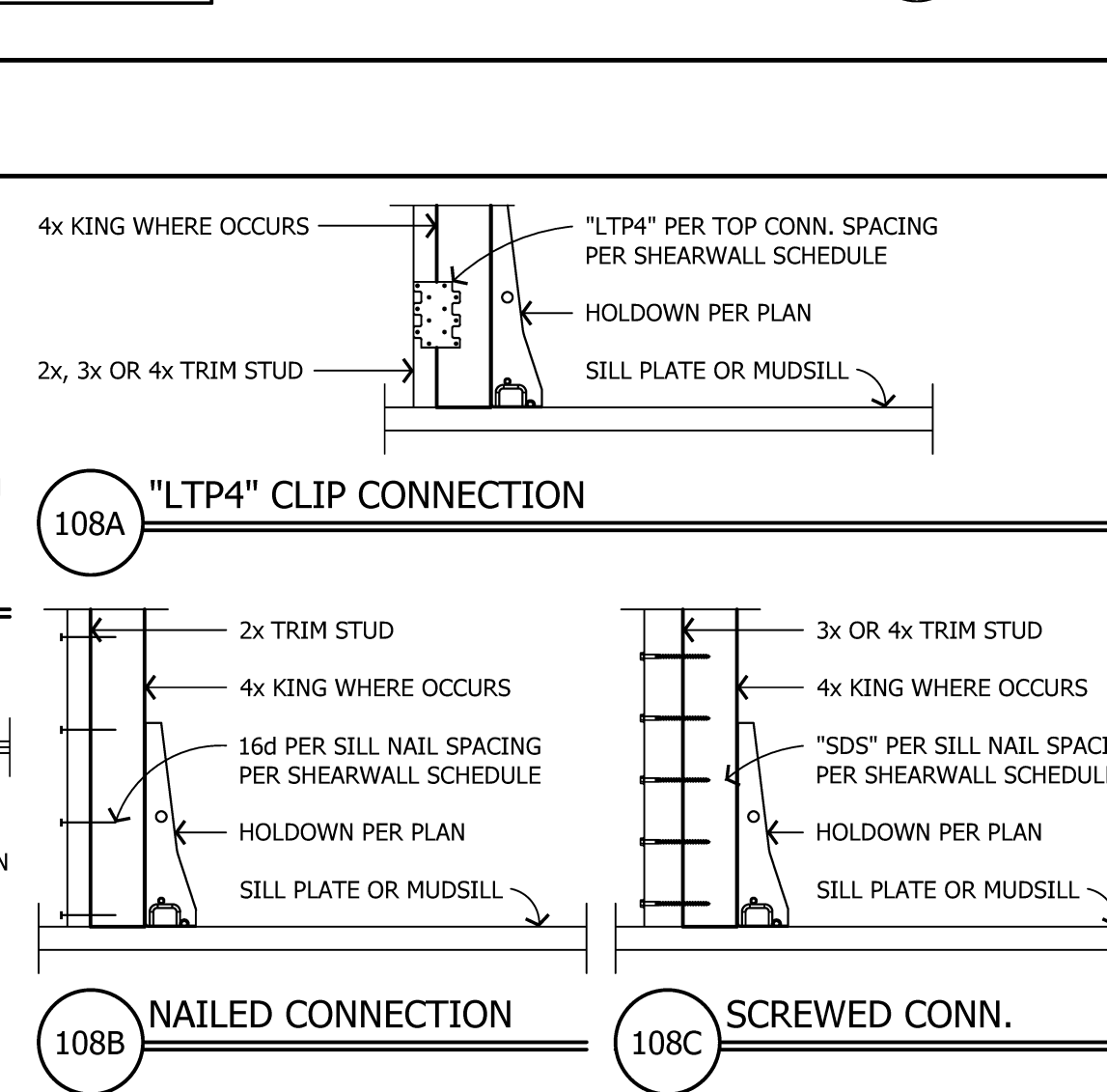
NOTE: A. WHERE NOTED, SB1x30 REQUIRES FOOTING WIDTH TO BE 18\"/>

112 SLAB HOLDOWN SCHEDULES



115 HOLDOWN A.B. TO EXTERIOR FOOTING

- ALL NAILS SHALL HAVE MINIMUM SHANK DIAMETER AS SPECIFIED IN SCHED. A. GALVANIZED NAILS SHALL BE HOT DIPPED OR TUMBLED.
- ALL FASTENERS THAT ARE INSTALLED INTO OR IN CONTACT WITH PRESSURE TREATED LUMBER ARE TO BE HOT DIPPED GALVANIZED.
- PLYWOOD AND OSB SHALL BE TYPE CDX GRADE OR BETTER (EXCEPT WHERE STRUCTURAL 1 GRADE IS NOTED).
- STR. 1 = STRUCTURAL 1 GRADE PLYWOOD / O.S.B.
- WHEN FASTENERS ARE SPACED LESS THAN 4\"/>
- ALL DOUBLE SIDED WALLS REQUIRE 3x SILL, MIN.
- PREDRILL SILL CONNECTIONS WHERE NEEDED TO AVOID WOOD SPLITTING. USE DRILL BIT SIZE = 0.75 x NAIL (OR SCREW) DIAMETER.
- 3x SILL PLATES MAY BE OMITTED WHEN ALTERNATE SHEATHING JOINT AT RIM OR BLOCKING IS PROVIDED PER DETAIL.
- INSTALL OBL 2x POST MIN. AT END OF SHEARWALLS. IF 3x FRAMING MEMBERS ARE REQUIRED USE 4x POST MINIMUM.
- ALL ANCHOR BOLTS MUST BE INSTALLED WITH 3\"/>
- GALVANIZED PLATE WASHERS PER THE CURRENT CBC.
- WHERE SHEAR MATERIAL IS APPLIED ON BOTH FACES OF A SHEARWALL, AND NAIL SPACING IS LESS THAN 6\"/>
- USE 3x SILLS AND 3x TOP PLATES.
- THE VERTICAL SHEAR PANEL JOINTS ON OPPOSITE FACES SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, UNLESS SUCH FRAMING MEMBERS ARE 4x OR THICKER.
- INSTALL 4x RIM / BLOCKING MINIMUM BELOW ALL DOUBLE SIDED SHEARWALLS.
- FOR ALTERNATE MUDSILL ANCHORAGE, SEE DETAIL 109 SD1.

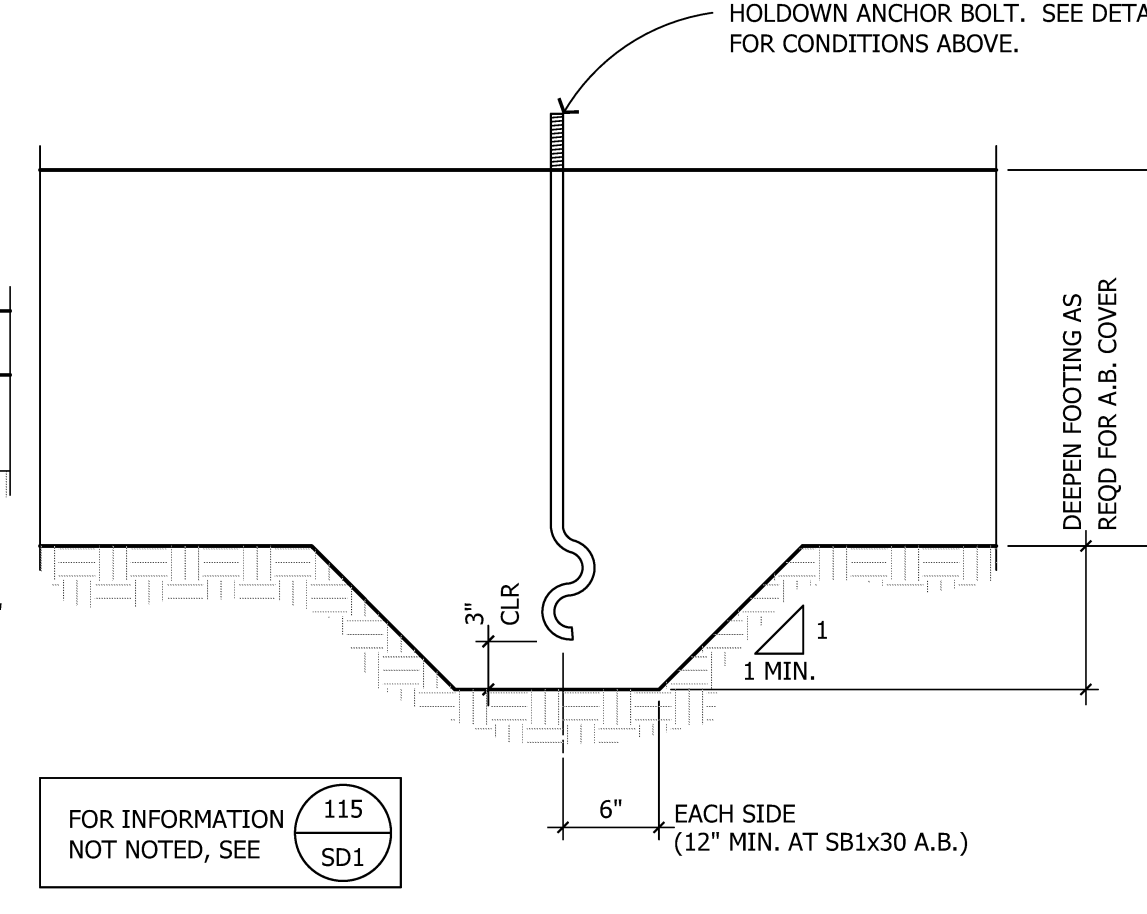


108 TRIM STUD TO HOLDOWN POST

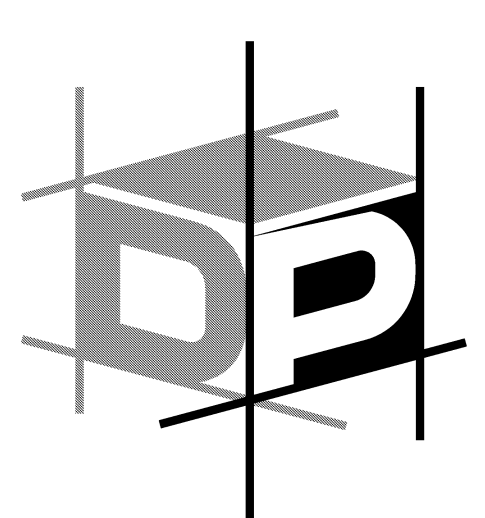
SCREWED / BOLTED HOLDOWN SCHEDULE				
ANCHOR	POST CONNECTORS	HOLDOWN ANCHOR	MIN. POST	ALLOW. LOAD
\"HDU2\"	(6) SD525212 SCREWS	SSTB24 OR SB5/8x24	(2) 2x	2940 lbs
\"HDU4\"	(10) SD525212 SCREWS	SSTB24 OR SB5/8x24	(2) 2x	3815 lbs
\"HDU5\"	(14) SD525212 SCREWS	SSTB24 OR SB5/8x24	(2) 2x	5645 lbs
\"HDU8\"	(20) SD525212 SCREWS	SSTB28 OR SB7/8x24	4x6 / 6x6	7855 lbs
\"HDU11\"	(30) SD525212 SCREWS	SB1x30 (A)	4x8 / 6x6	11175 lbs
\"HDU14\"	(36) SD525212 SCREWS	PAB8 (OR EQUIV.)	4x8 / 6x6	14390 lbs
\"HD19\"	(5) 1\" DIA. A307 BOLTS	PAB10 (OR EQUIV.)	4x8 / 6x6	19070 lbs

- RE-TIGHTEN ALL BOLTS PRIOR TO SHEARWALL CLOSE-IN.
- DOUBLE 2x HOLDOWN POSTS SHALL BE SISTERED TOGETHER PER SILL NAILING REQUIREMENTS FOR SHEARWALL TYPE. SEE DETAIL FOR MORE INFORMATION.
- CONTRACTOR'S OPTION TO SUBSTITUTE \"STHD\" TYPE HOLDOWNS:

ANCHOR	\"STHD\" OPTION	POST CONNECTORS	MIN. POST	ALLOW. LOAD
\"HDU2\"	\"STHD10\"	(20) 16d SINKERS	(2) 2x	2940 lbs
\"HDU4\"	\"STHD14\"	(24) 16d SINKERS	(2) 2x	3815 lbs



116 DEEPENED FOUNDATION (FOR A.B.)

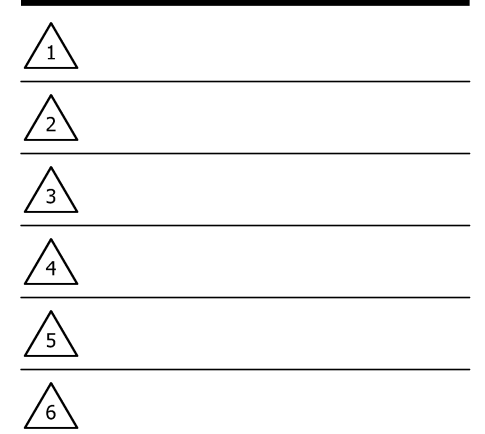


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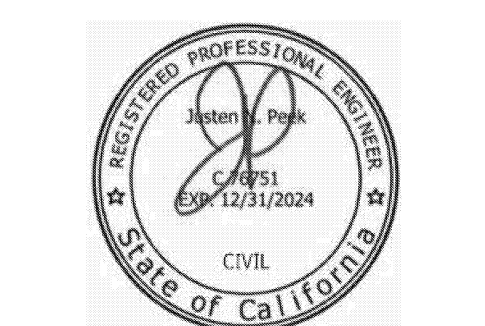
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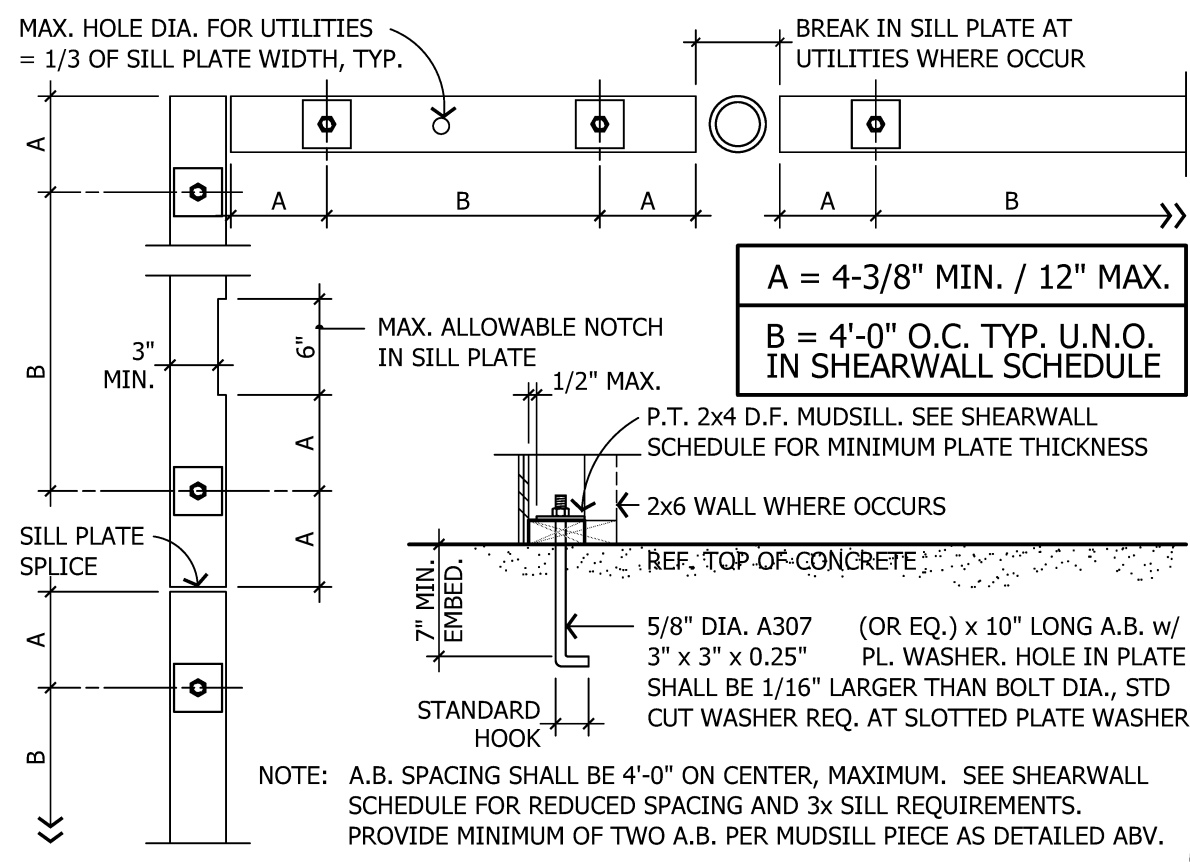
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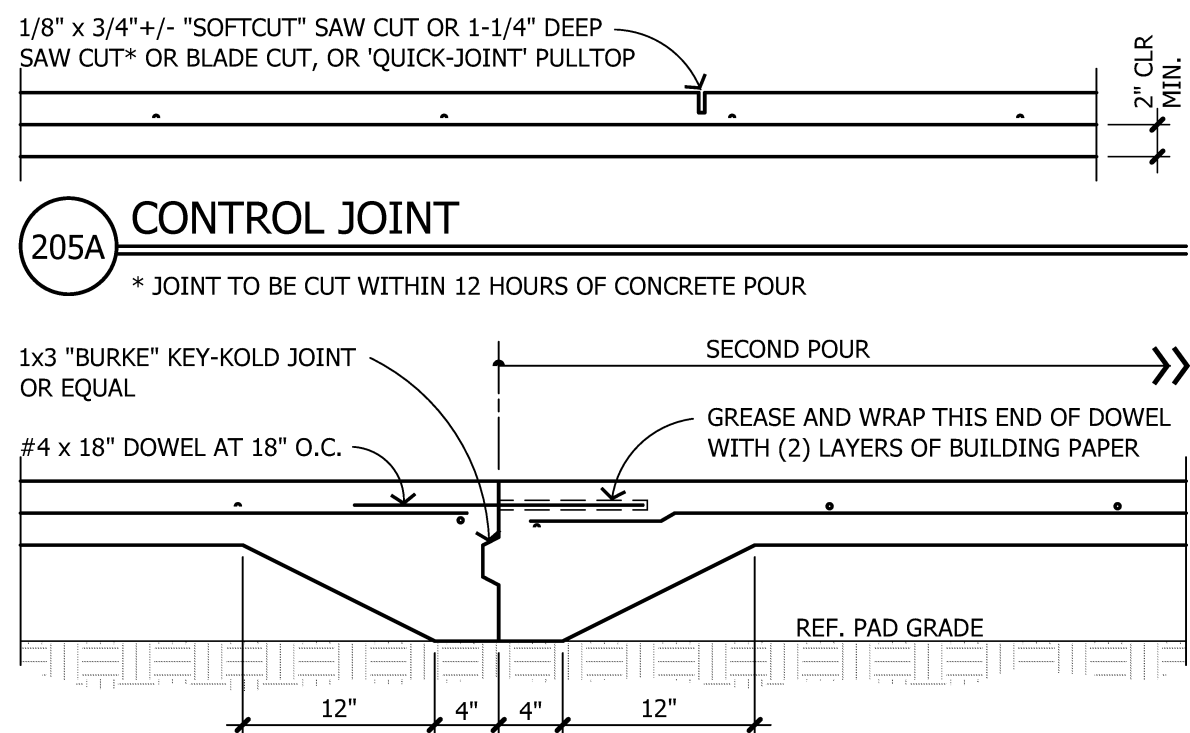
Sheet Description:
SHEARWALL SCHEDULE AND HOLDOWN SCHEDULE AND TYPICAL DETAILS

No Scale
Initial Issue Date: March 24, 2023
Drawn By: E. Bennett
Project Designer: J. Peek
Project Manager: J. Peek Ext. 23
Job No. W021420
Sheet No.

SD1

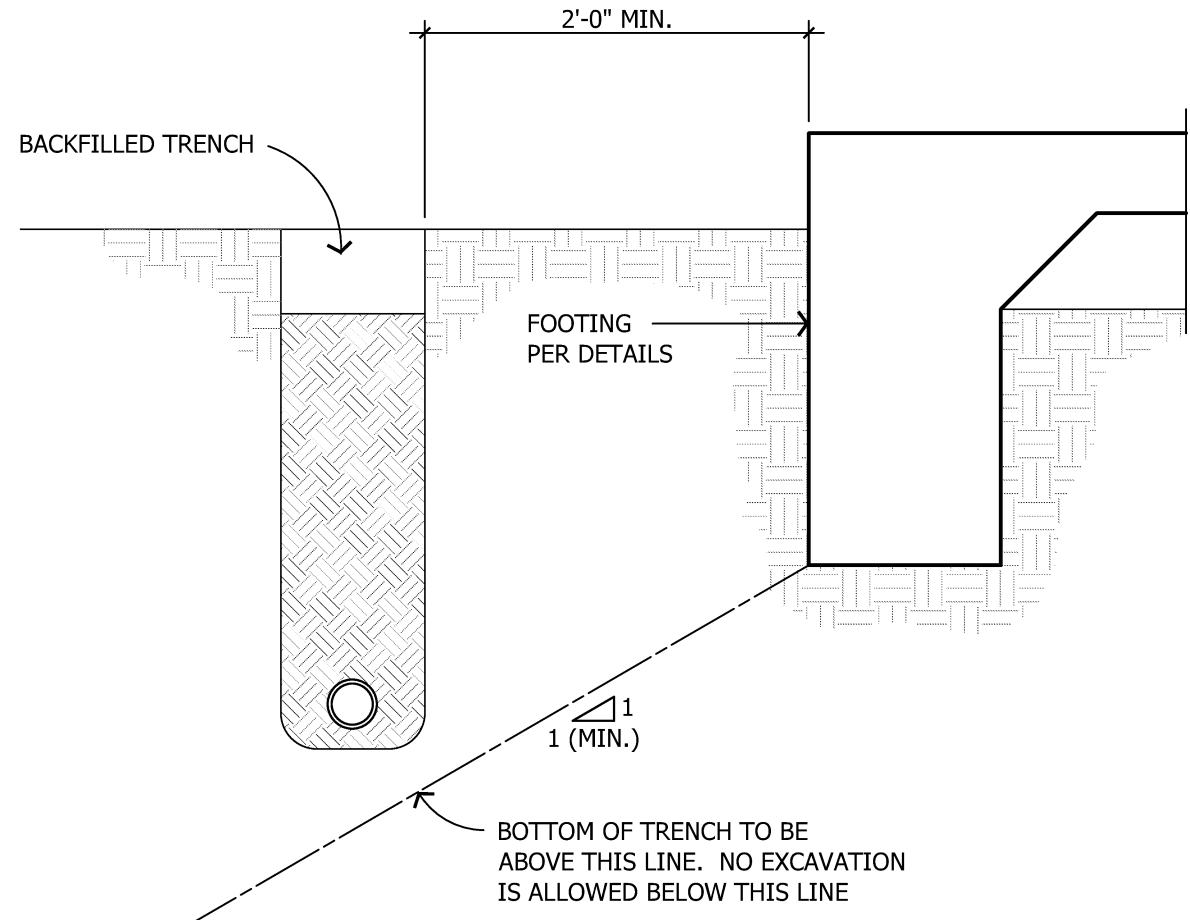


201 TYPICAL ANCHOR BOLT AND SHOT PIN PLACEMENT

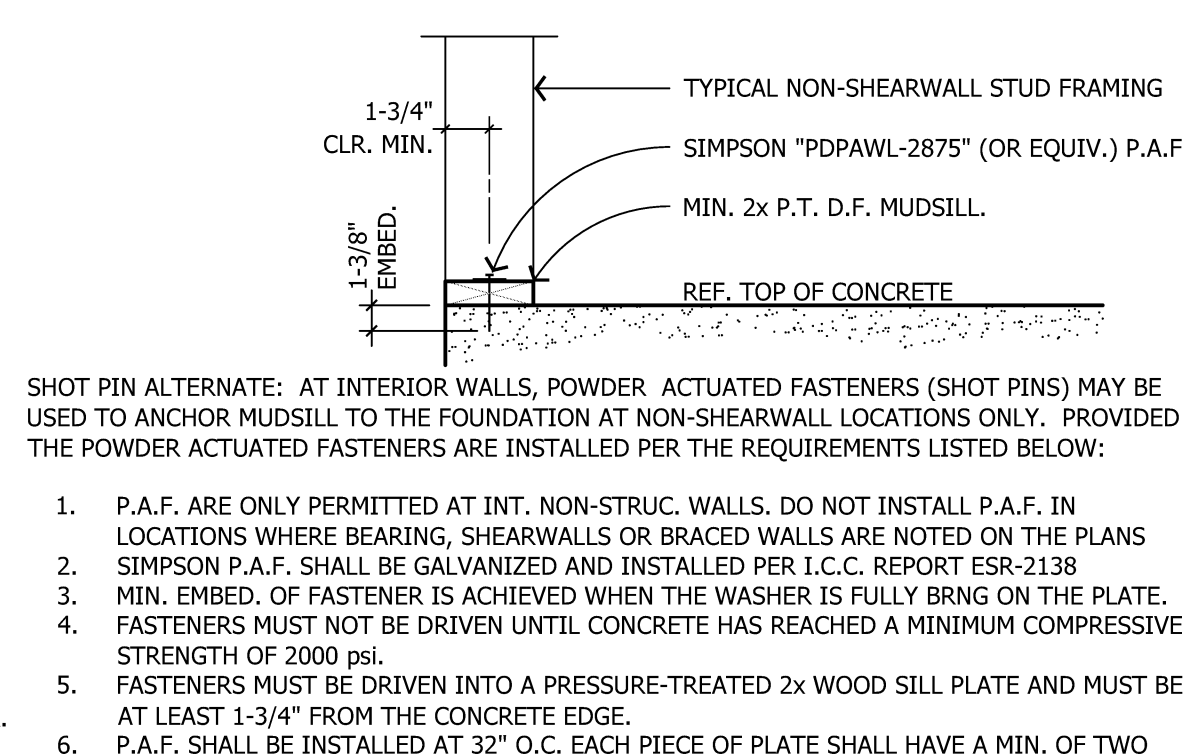


205B CONSTRUCTION JOINT

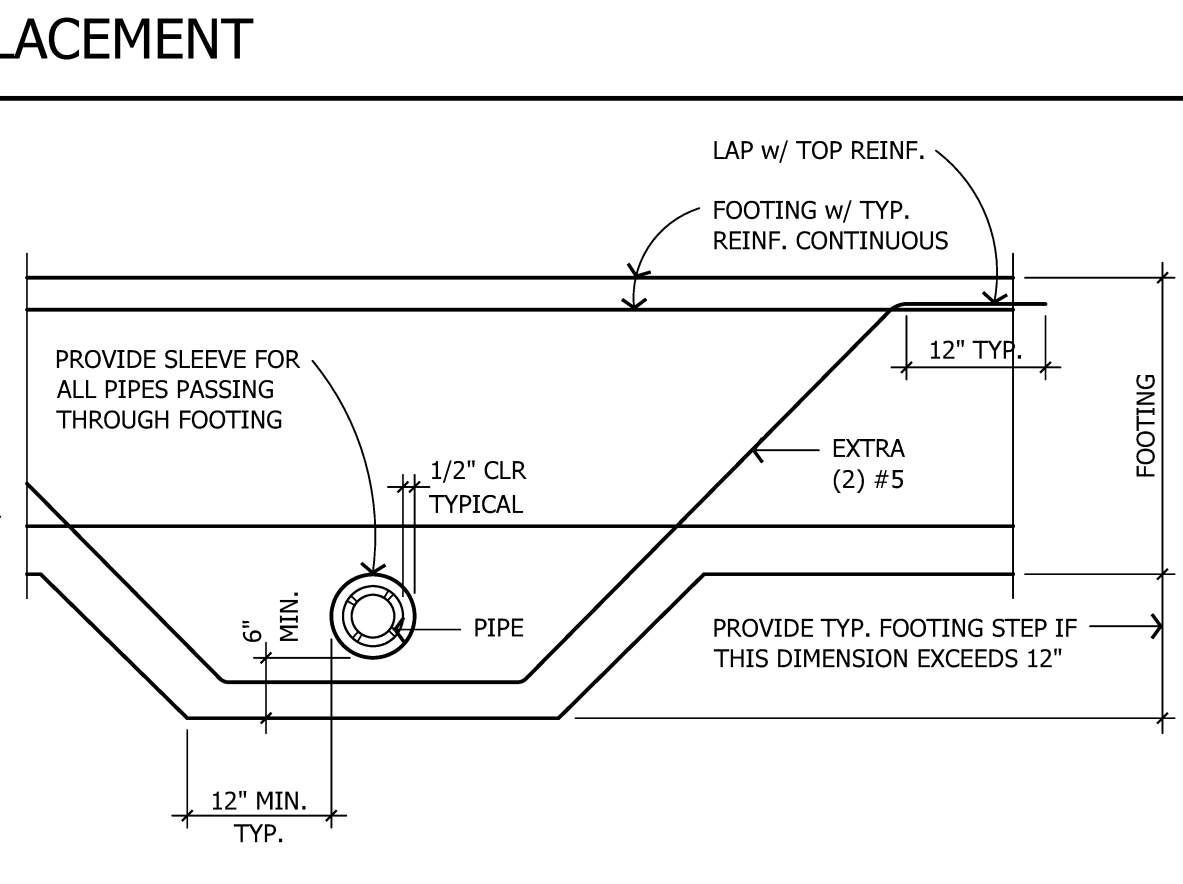
205 CRACK CONTROL JOINTS (SLAB-ON-GRADE)



207 PIPE / TRENCH PARALLEL TO FOOTING

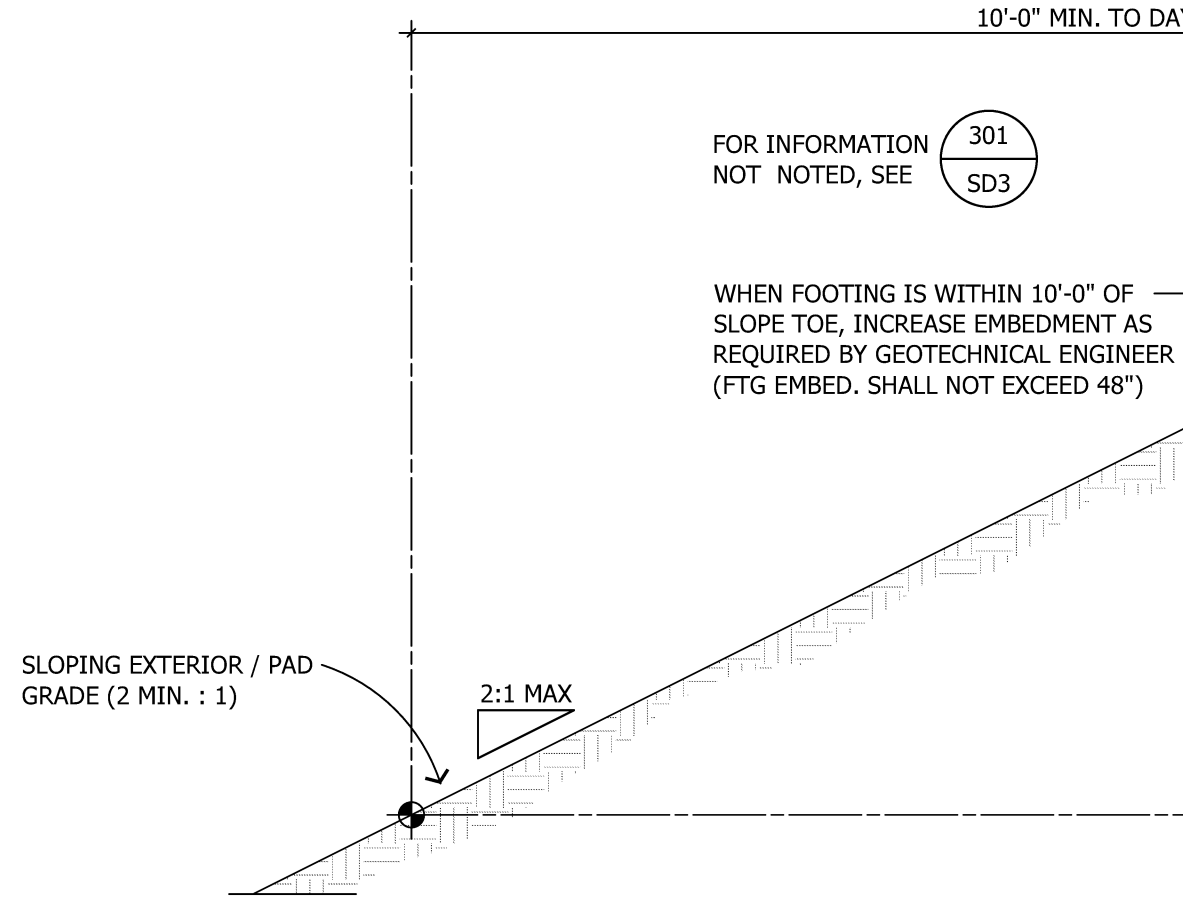


202 TYP. REINFORCING HOOKS AND LAPS

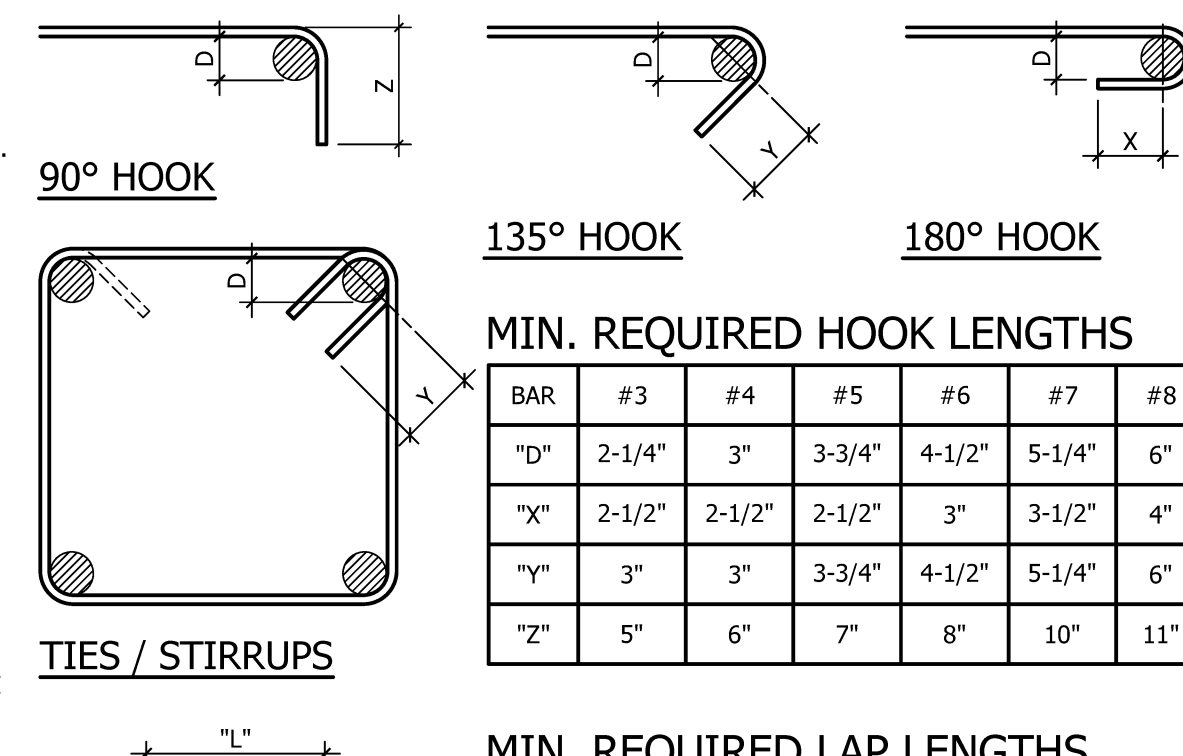


206A PIPE BELOW FOOTING

206 PIPE PERPENDICULAR TO FOOTING

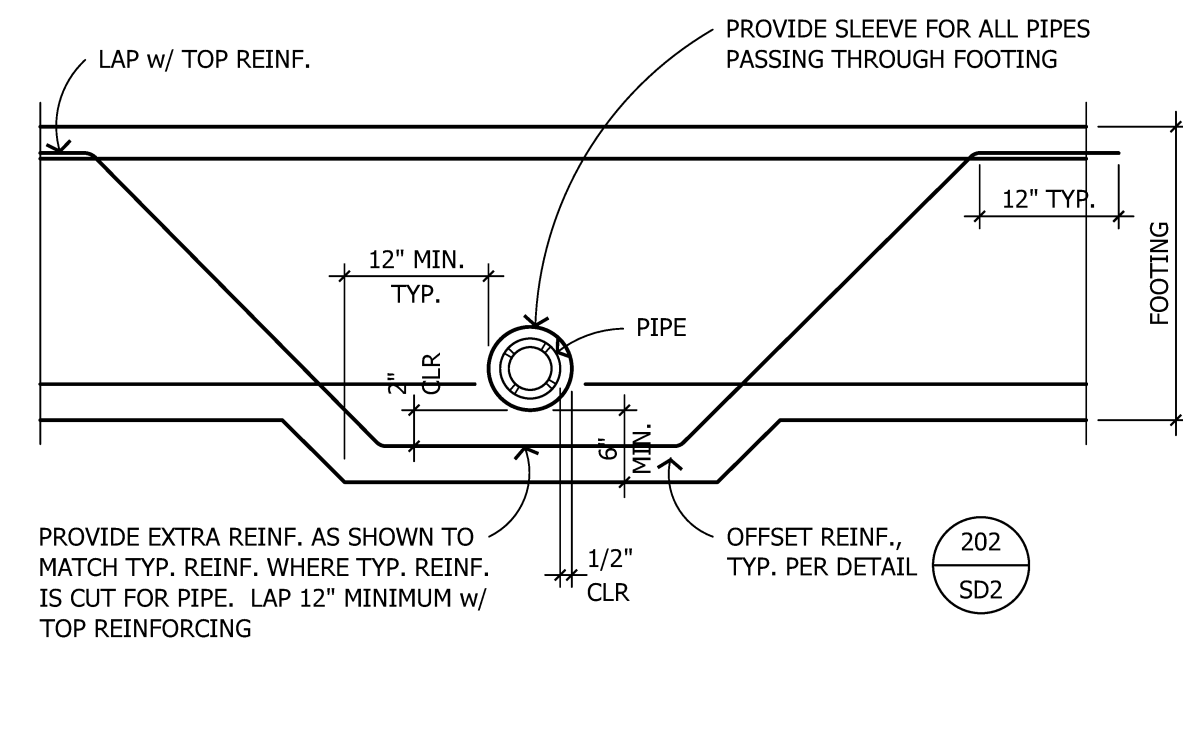


208 INCREASED FOOTING DEPTH BY SLOPE



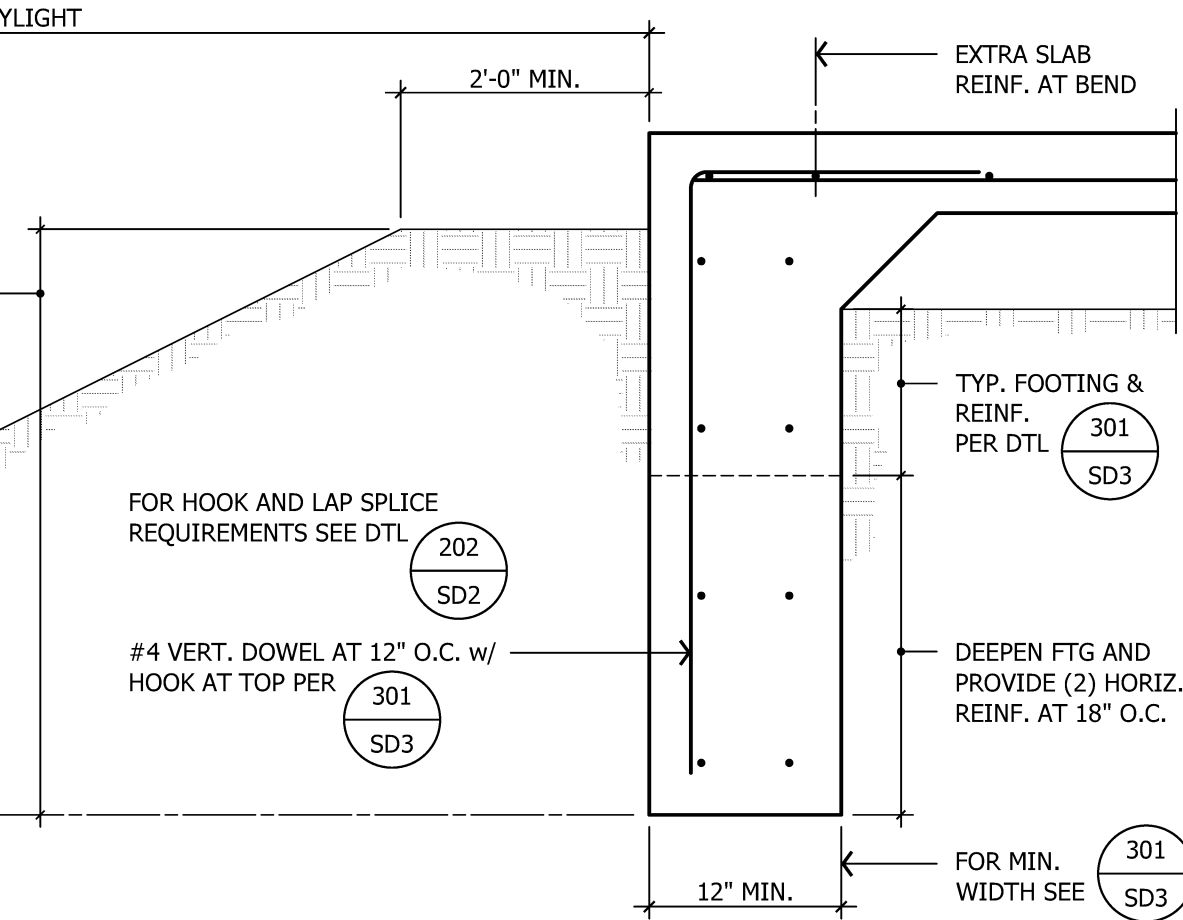
203A TEE COND. and 203B CORNER COND.

203 STD REINF. HOOKS - DOUBLE ROW

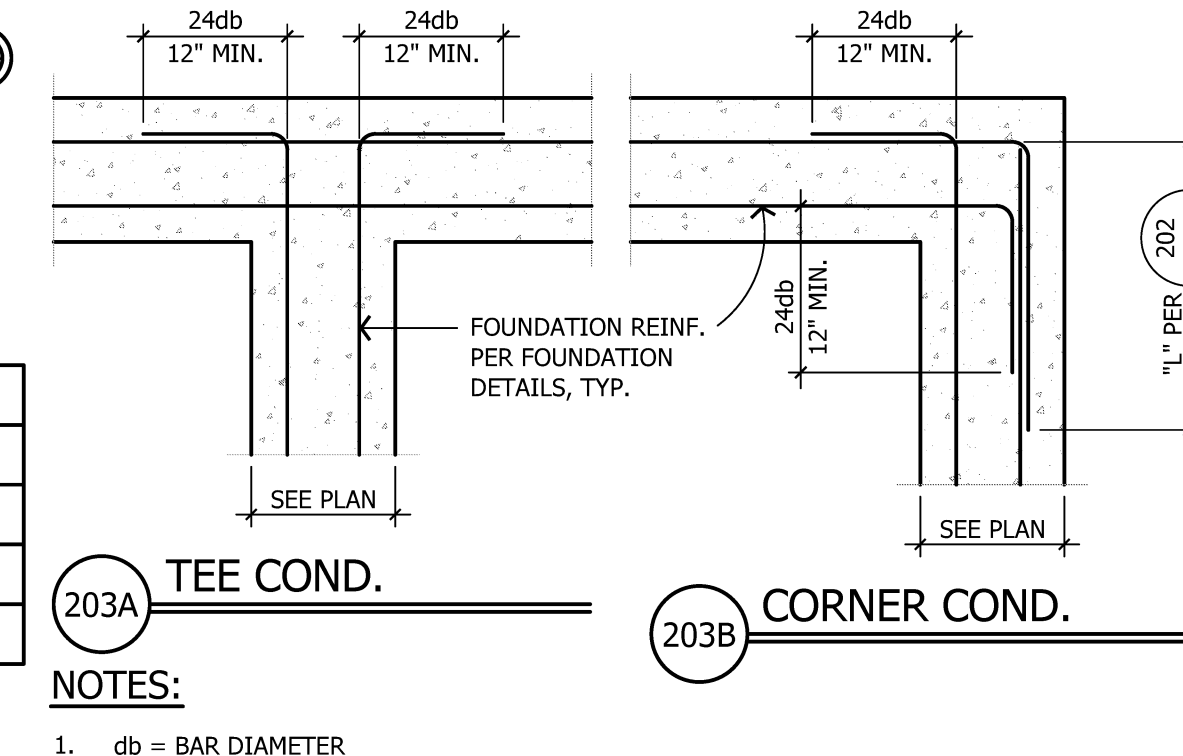


206B PIPE THROUGH FOOTING

206C PIPE BELOW FOOTING

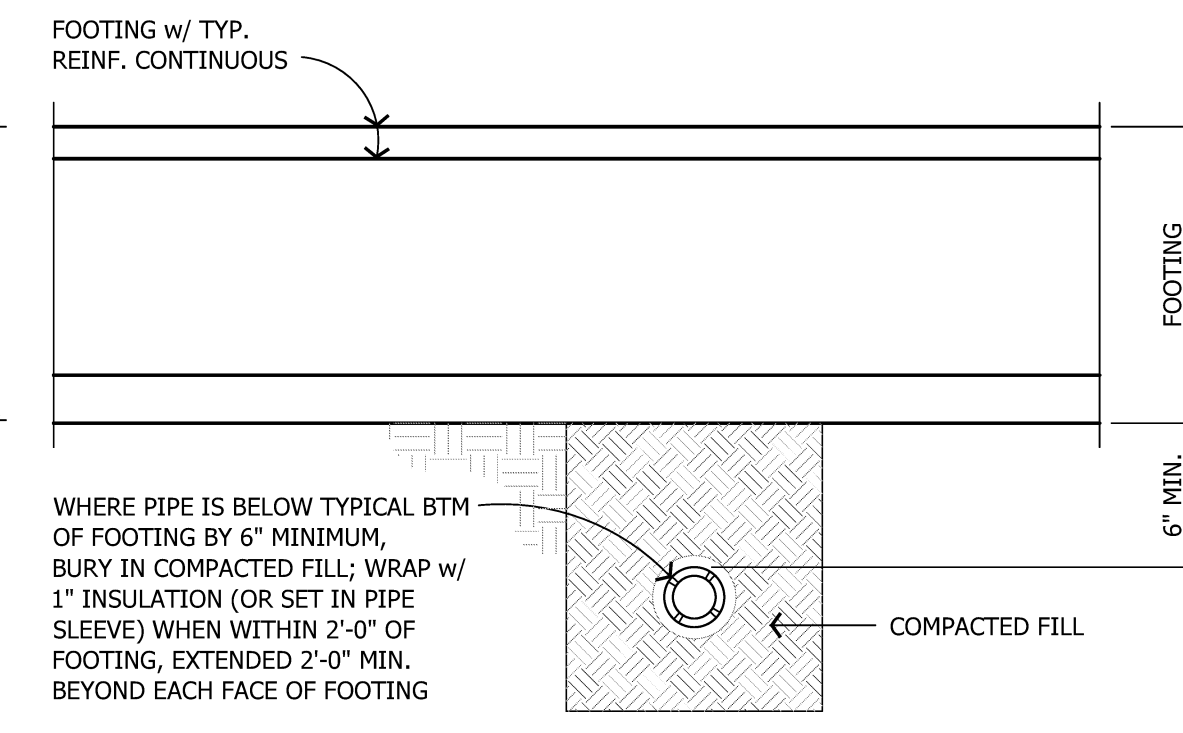


209A TYPICAL LAP SPLICE



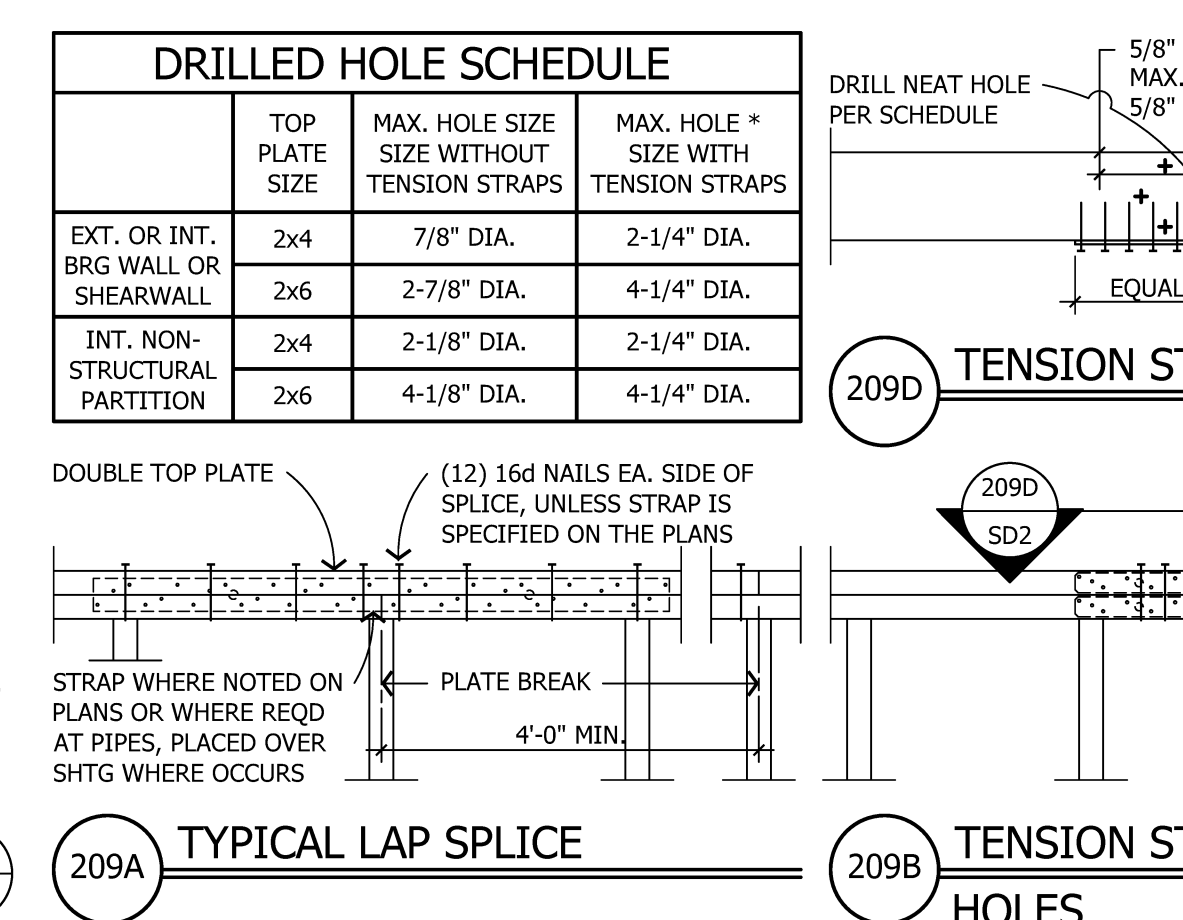
209B TENSION STRAP AT DRILLED HOLES

209C COMPRESSION STRAP AT NOTCHES

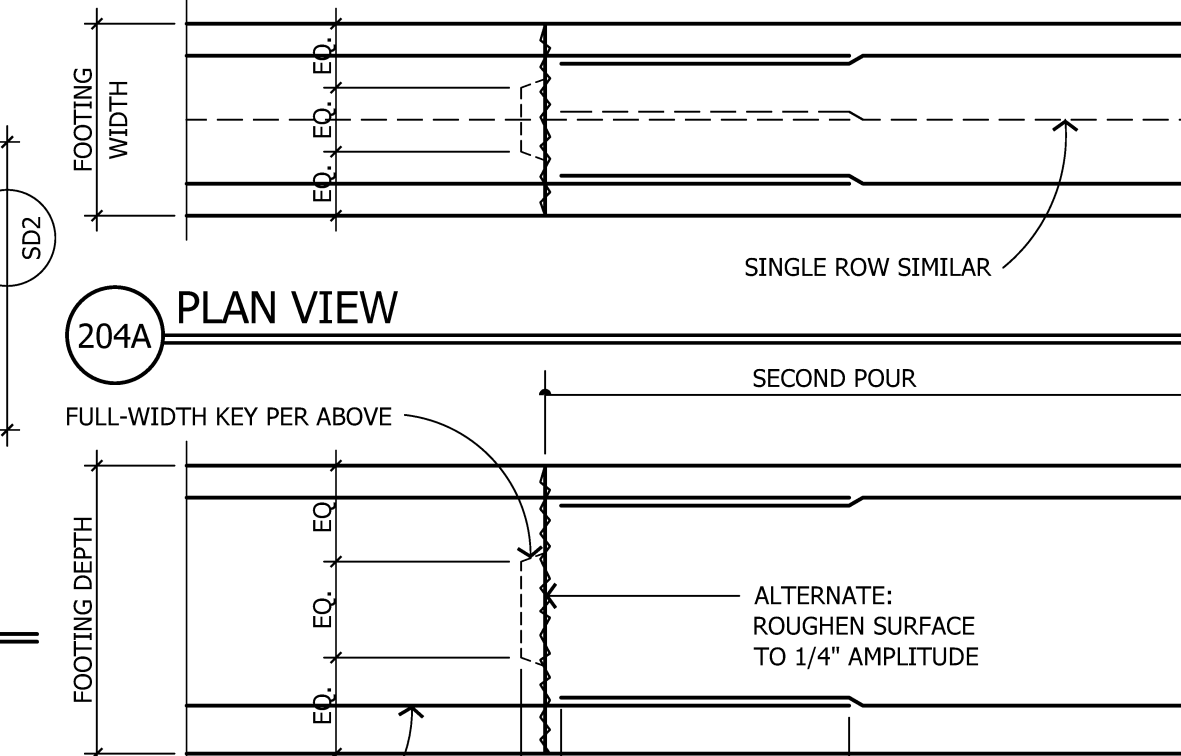


209D TENSION STRAP - PLAN VIEW

209E COMPRESSION STRAP - PLAN VIEW

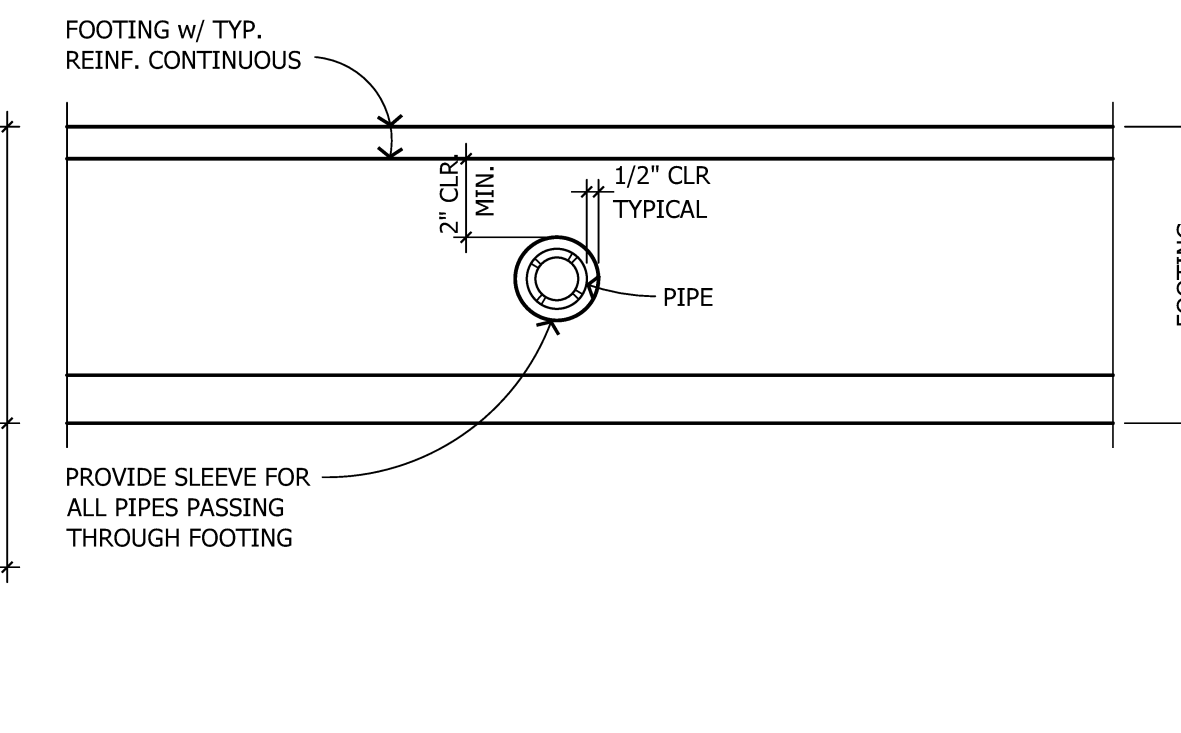


209F INSULATION AT HEADER



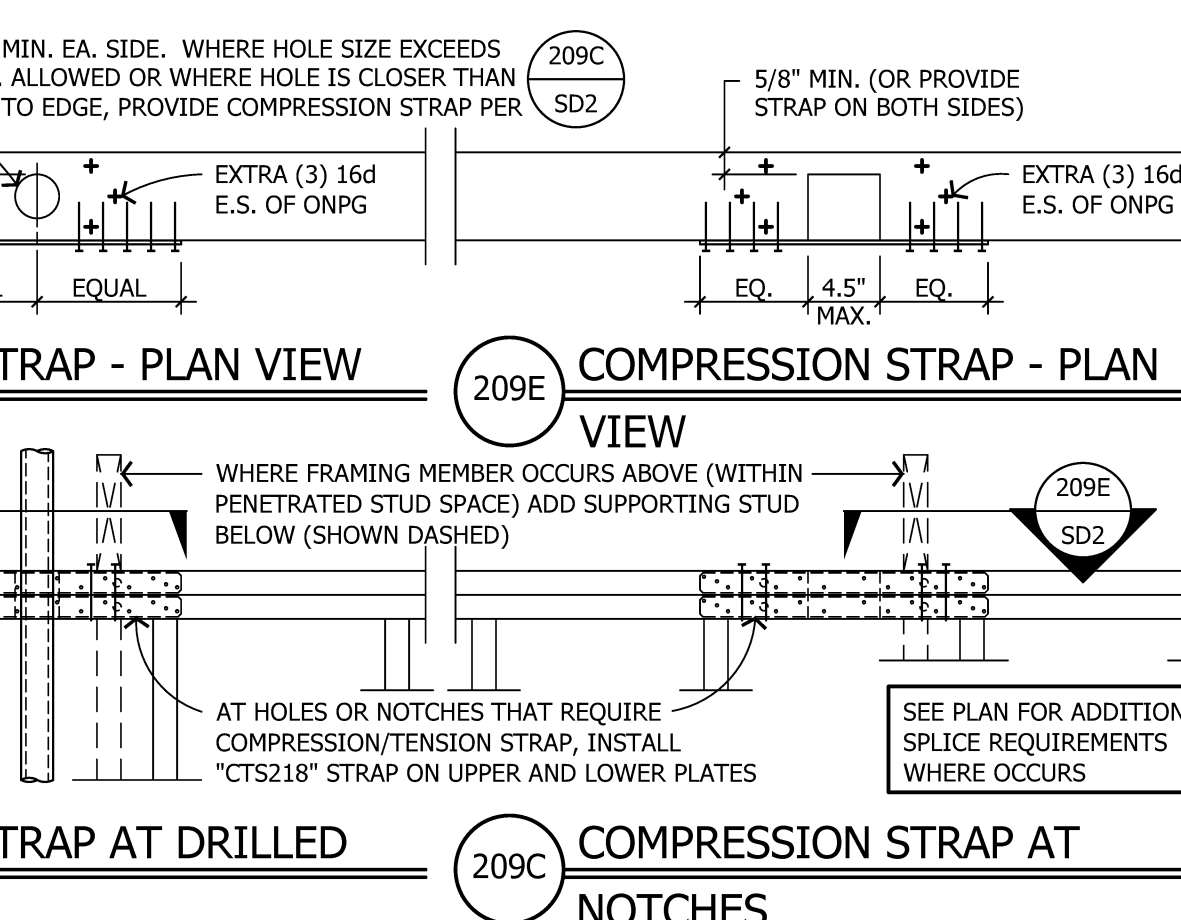
204 FOOTING CONSTRUCTION JOINT

204A PLAN VIEW

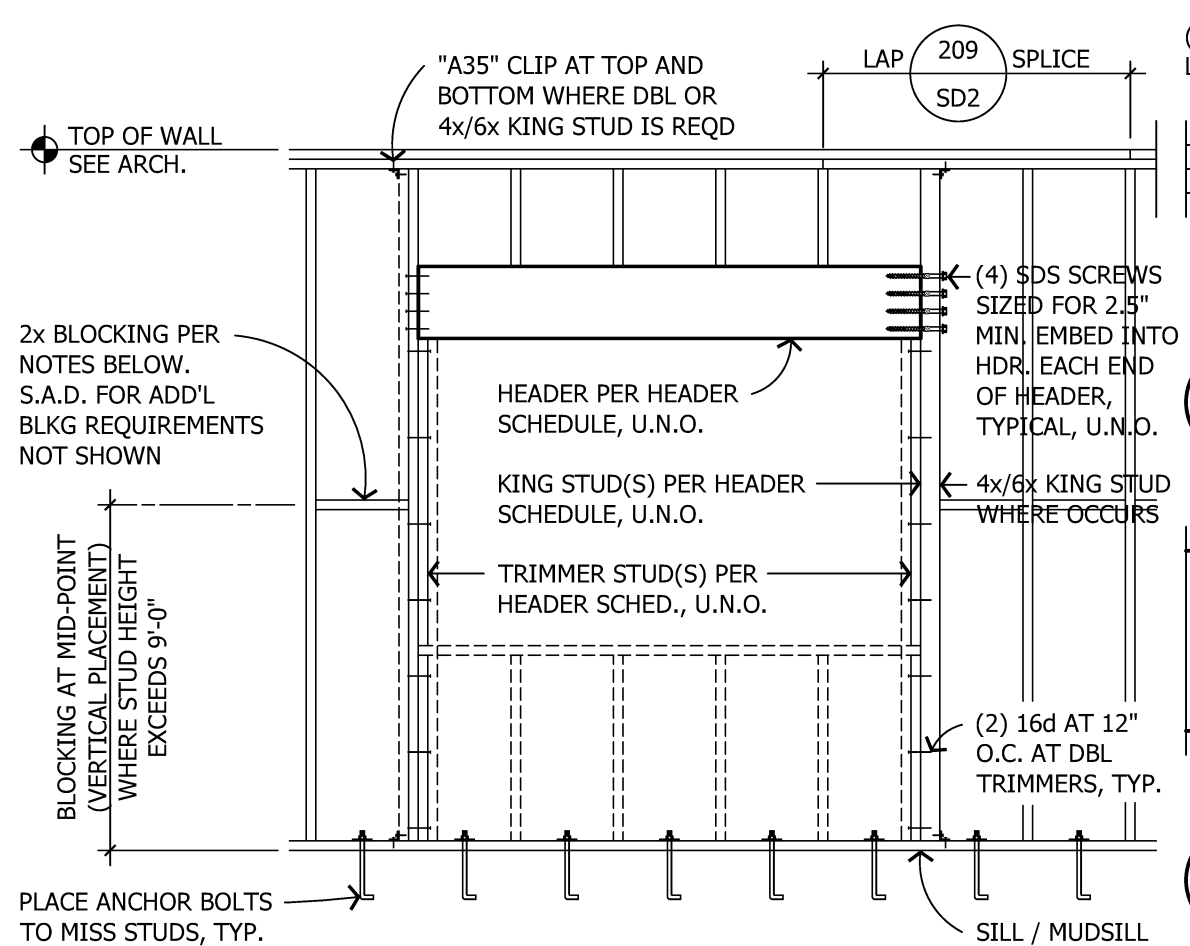


206C PIPE BELOW FOOTING

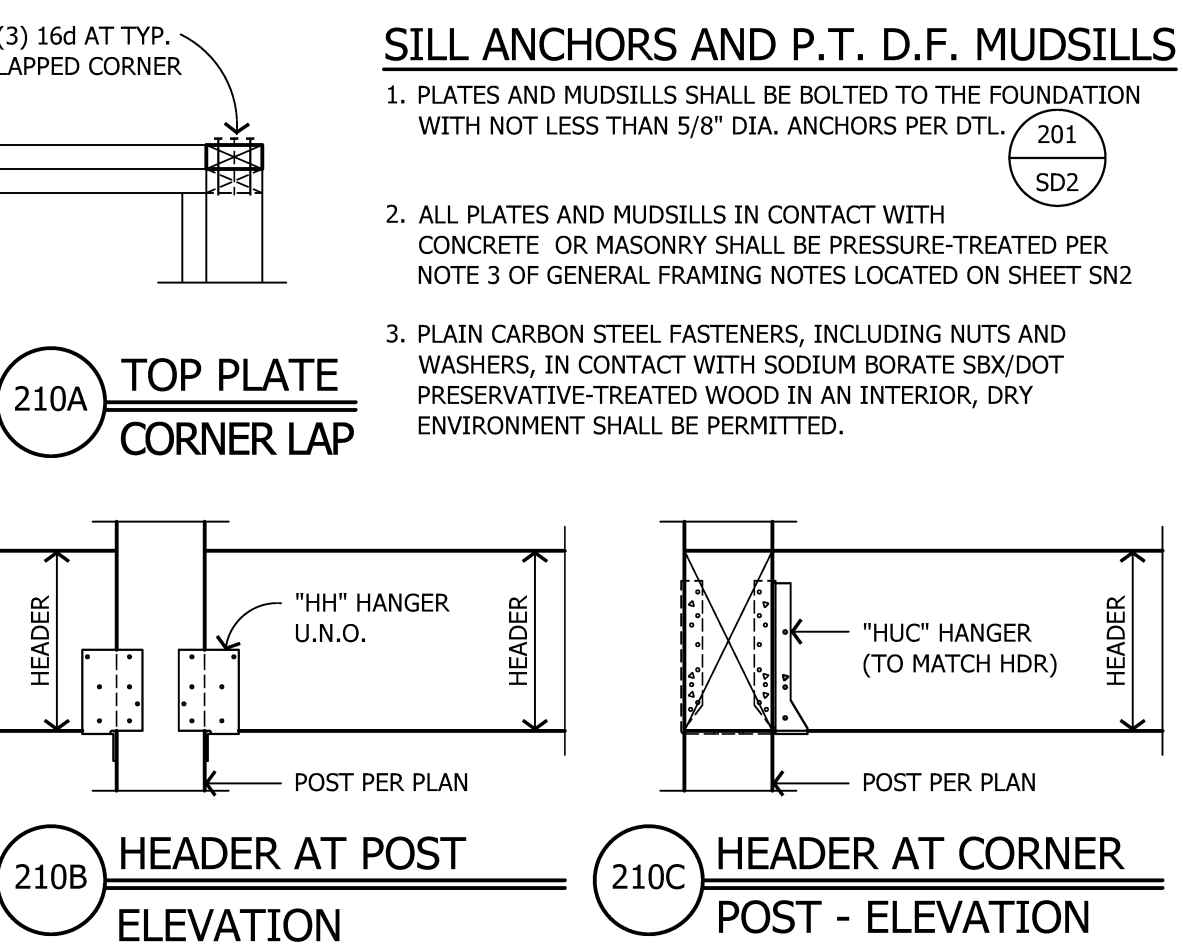
206D PIPE BETWEEN BARS



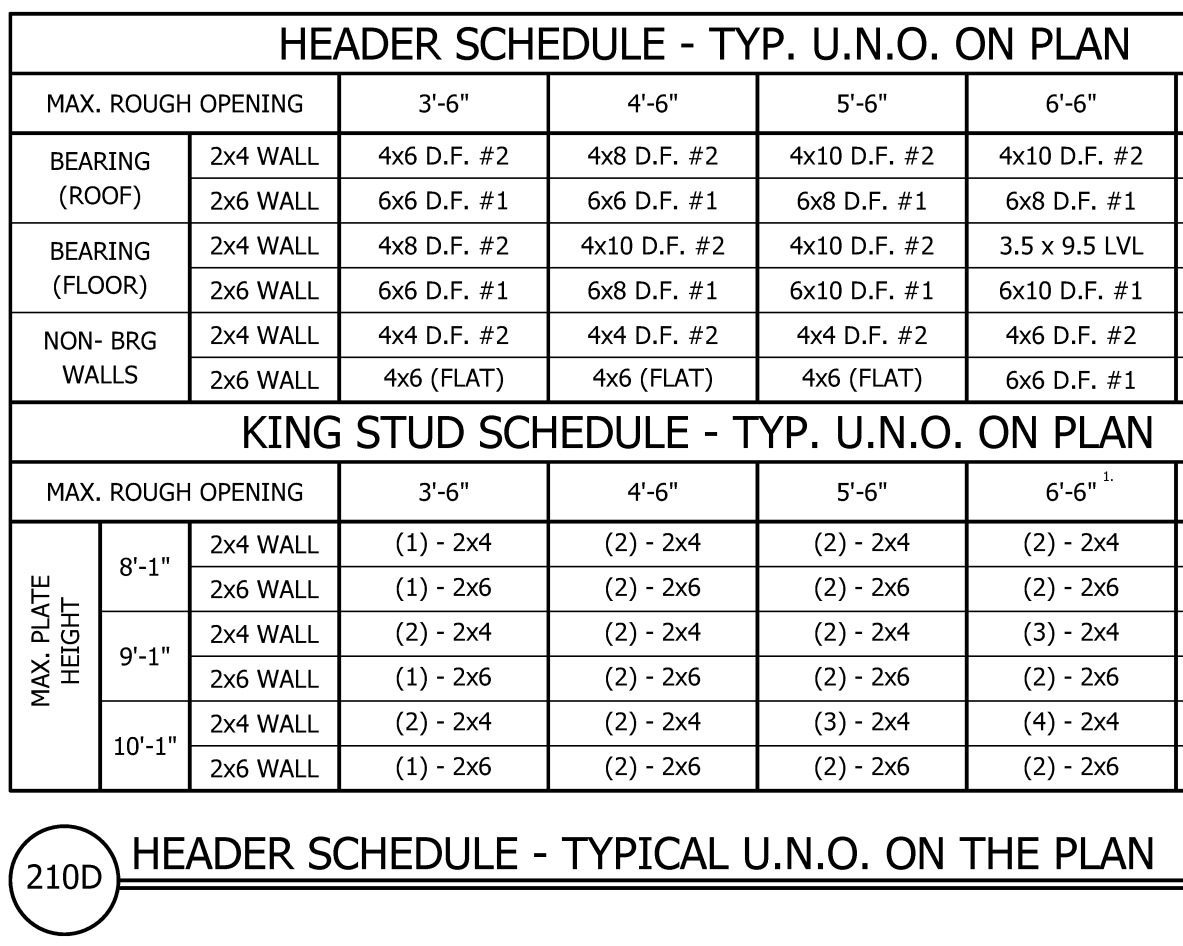
209C COMPRESSION STRAP AT NOTCHES



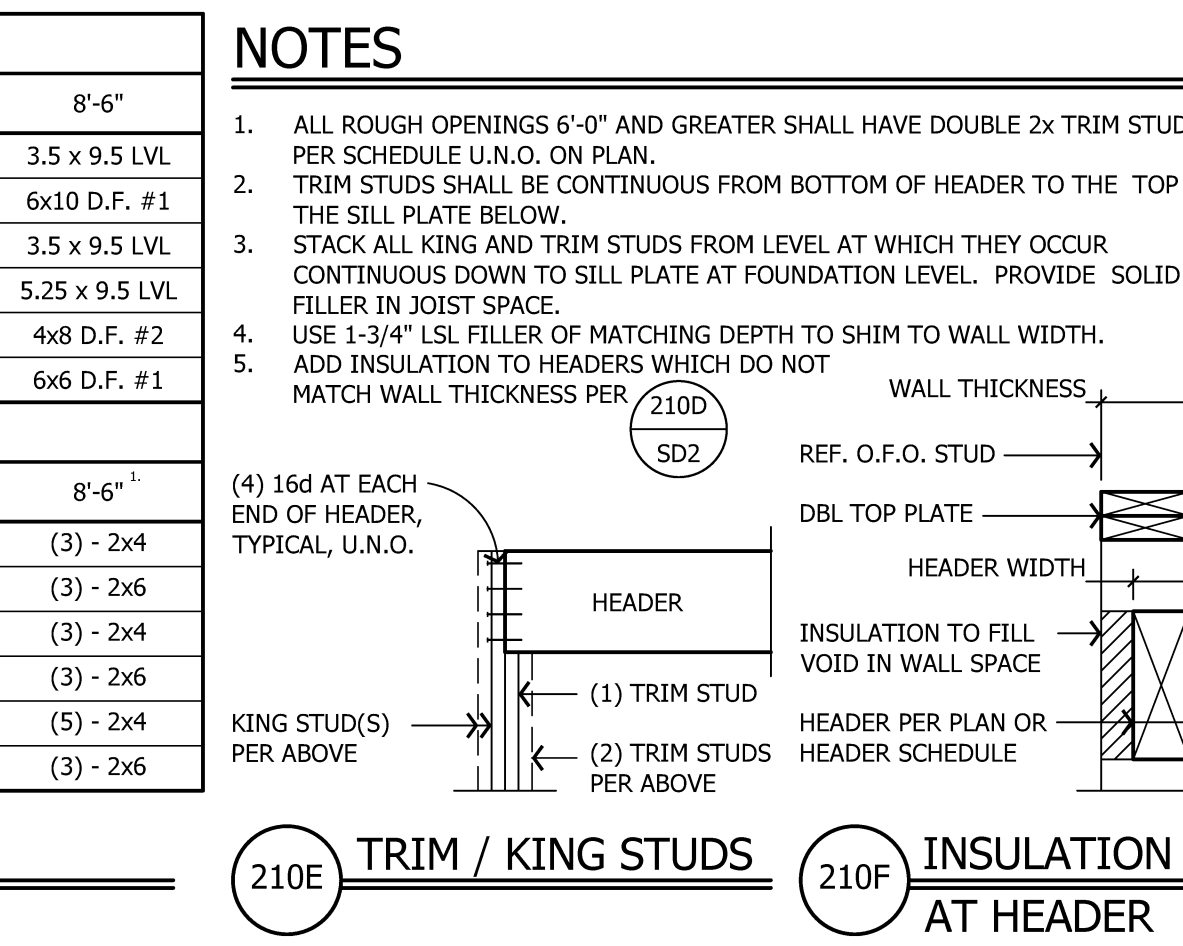
210 TYPICAL FRAMED WALL OPENING AND HEADER SCHEDULE



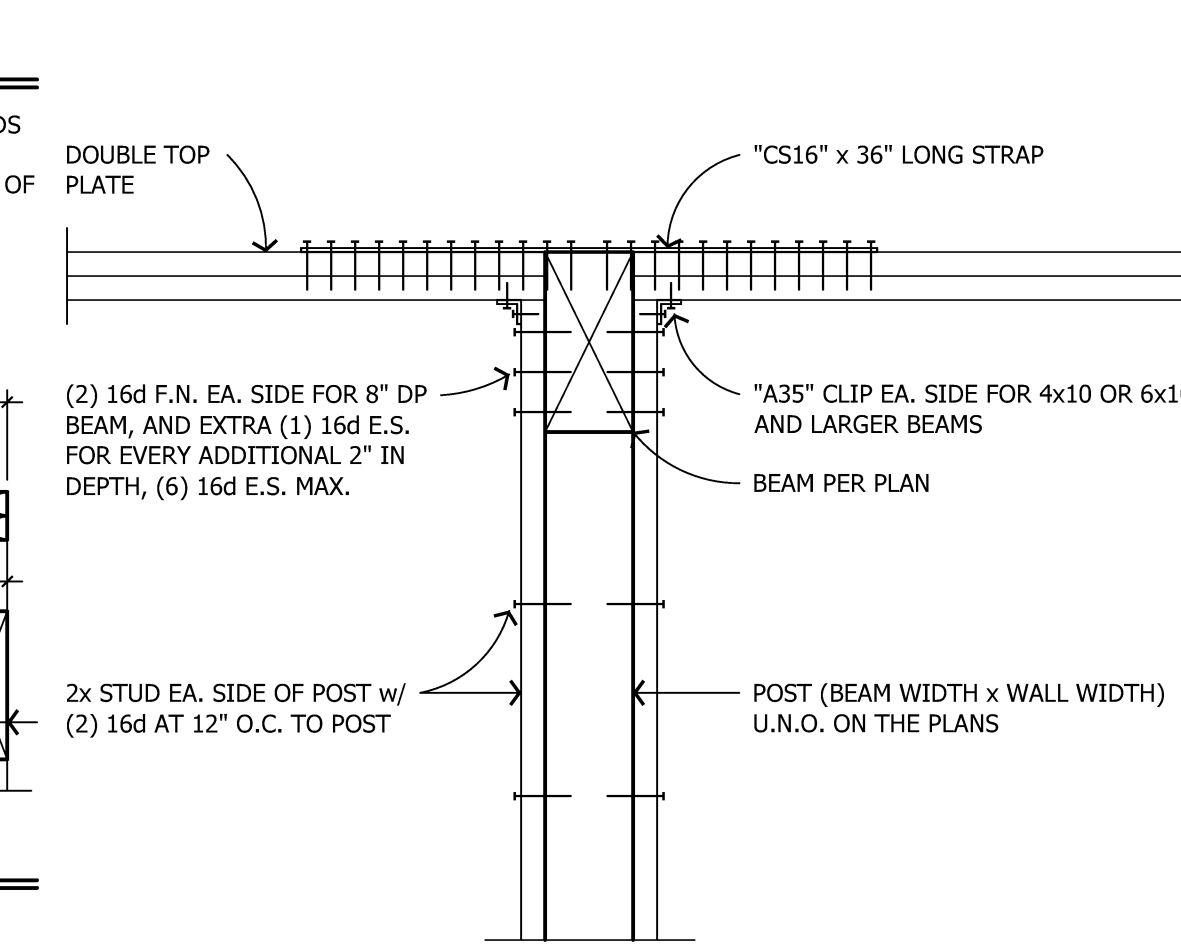
210A TOP PLATE CORNER LAP and 210B HEADER AT POST ELEVATION



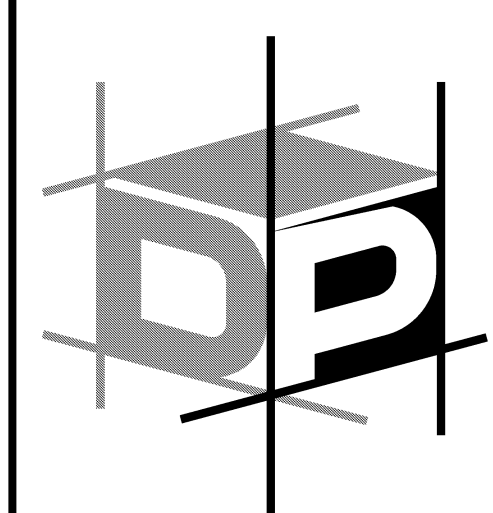
210C HEADER AT CORNER POST - ELEVATION and 210D HEADER SCHEDULE - TYPICAL U.N.O. ON THE PLAN



210E TRIM / KING STUDS and 210F INSULATION AT HEADER



211 TYPICAL BEAM SUPPORT IN WALL



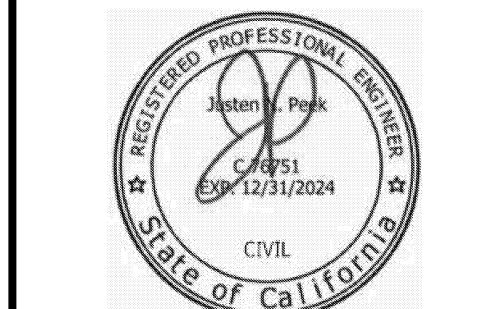
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PLEASE NOTE THE REVISION NUMBER AND DATE ARE FOR STRUCTURAL SHEETS ONLY.
Engineering Seal:



Sheet Description:
TYPICAL CONCRETE AND CARPENTRY DETAILS

No Scale

Initial Issue Date: March 24, 2023

Drawn By: E. Bennett

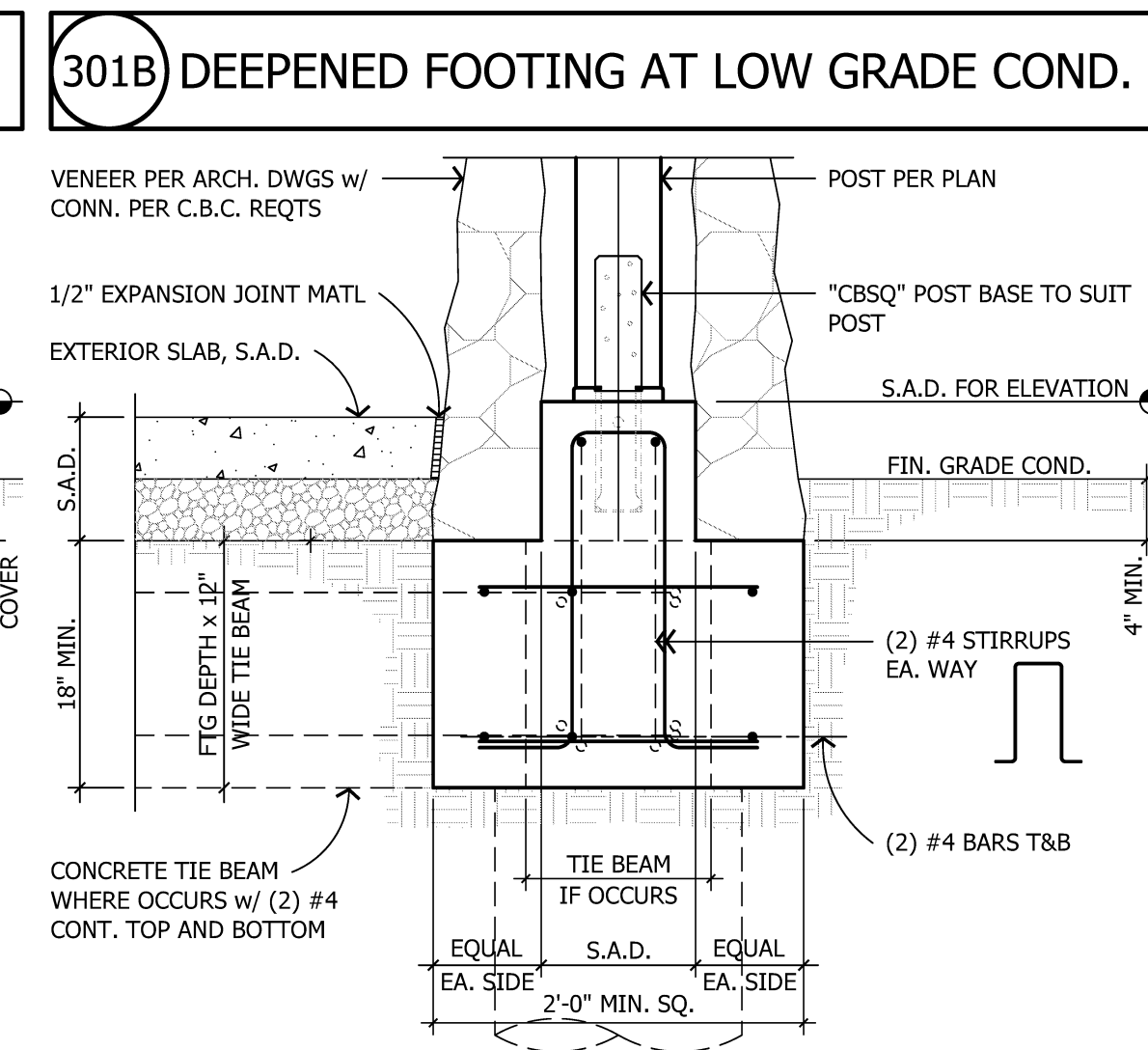
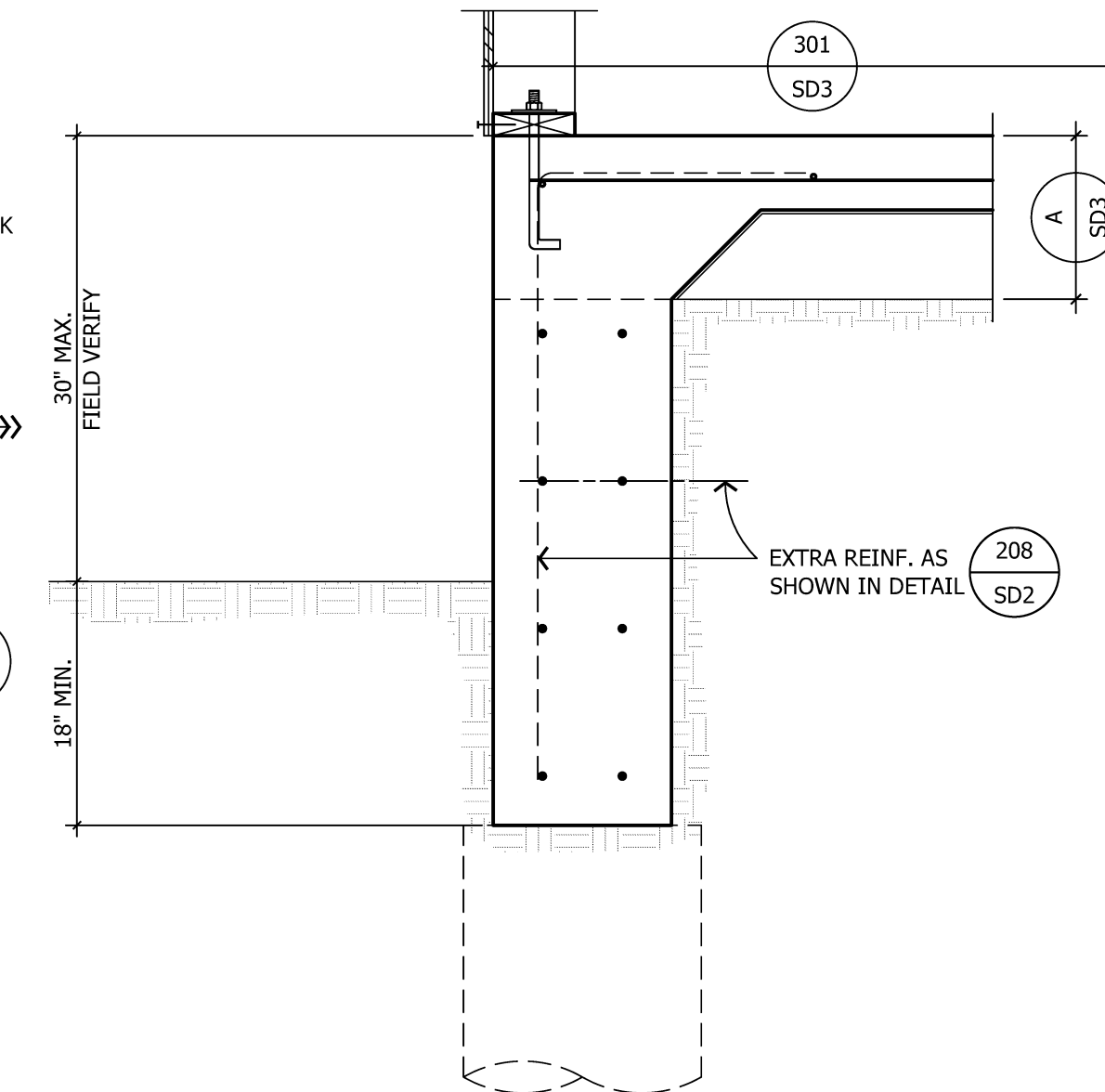
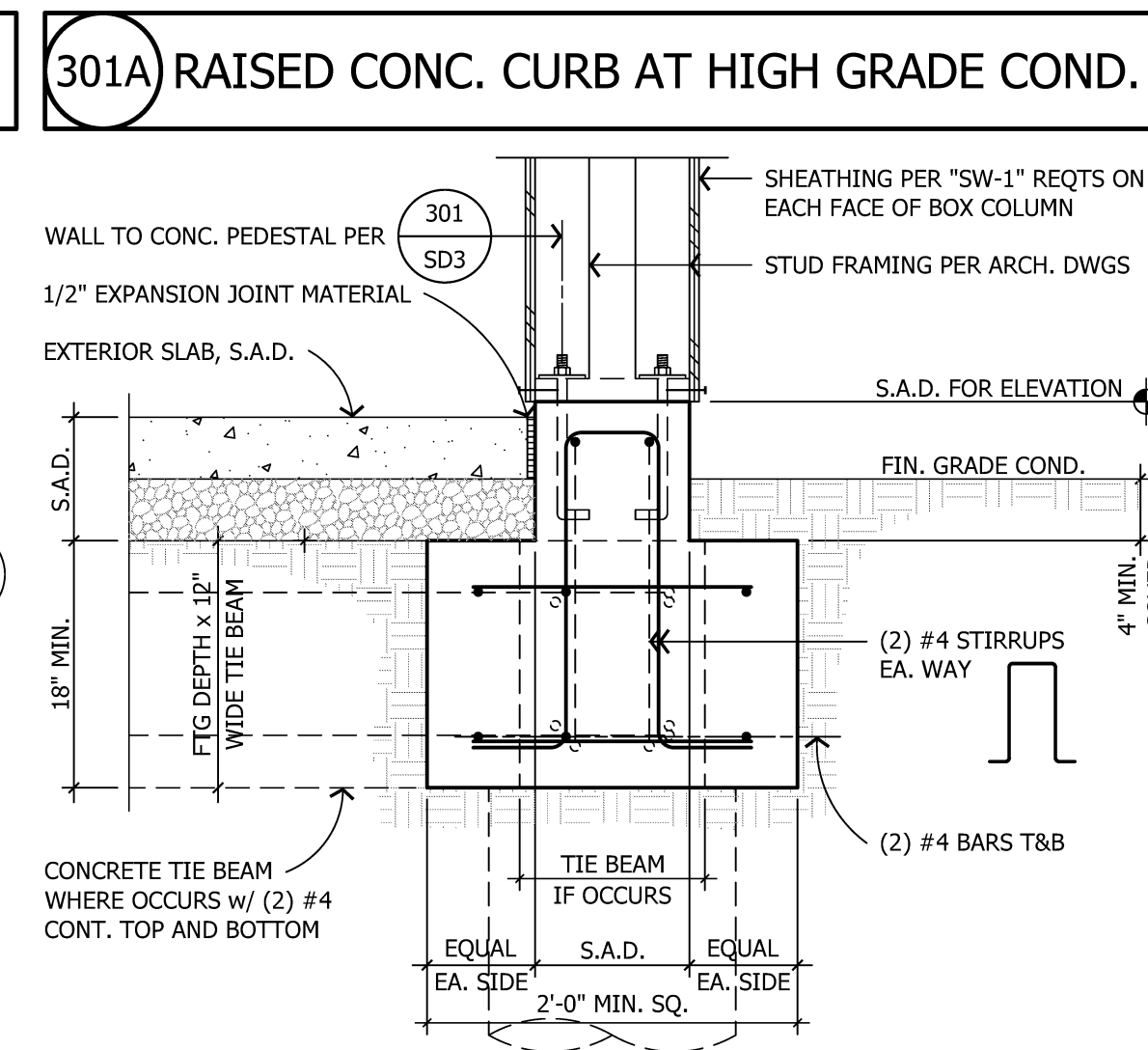
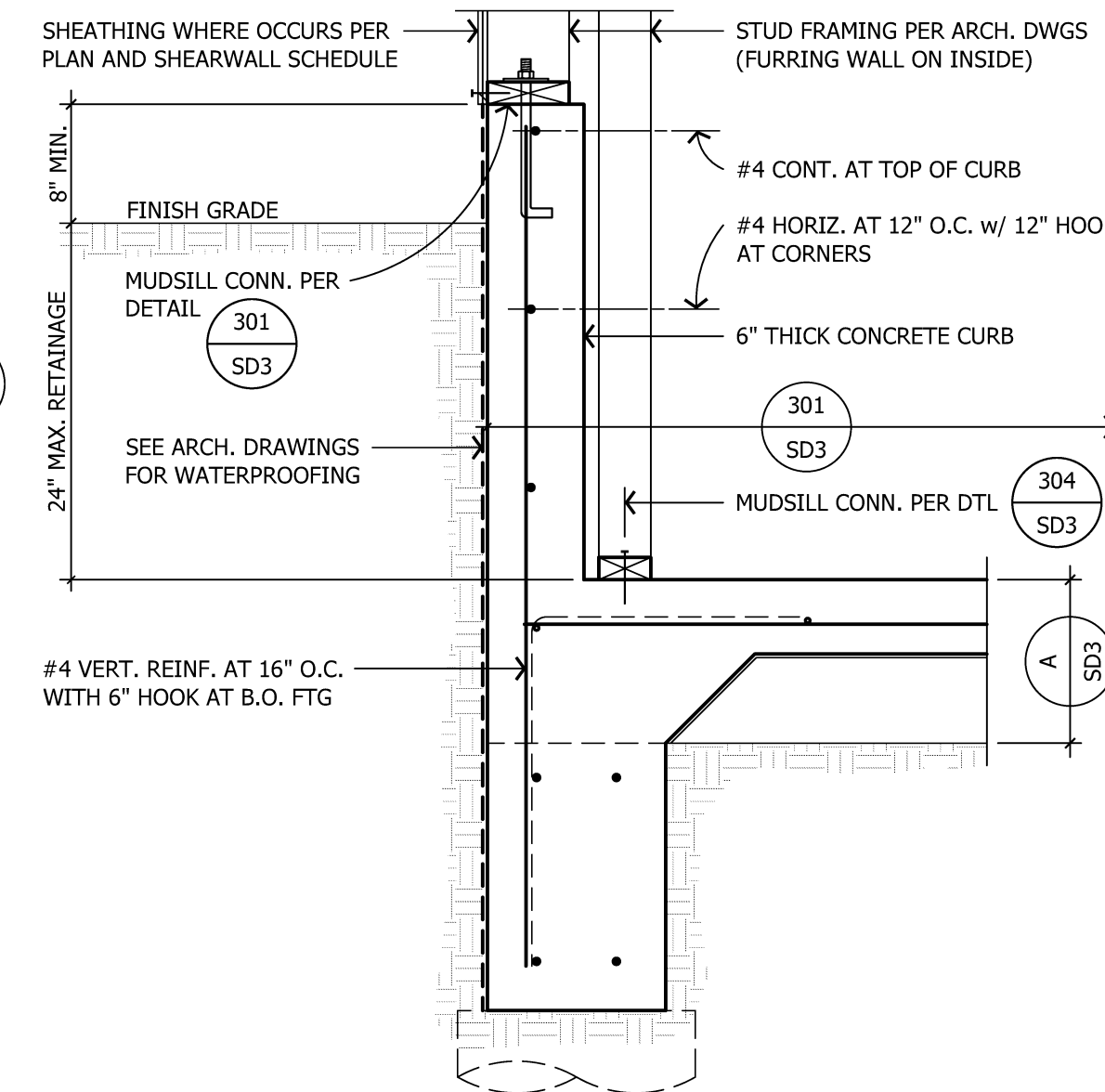
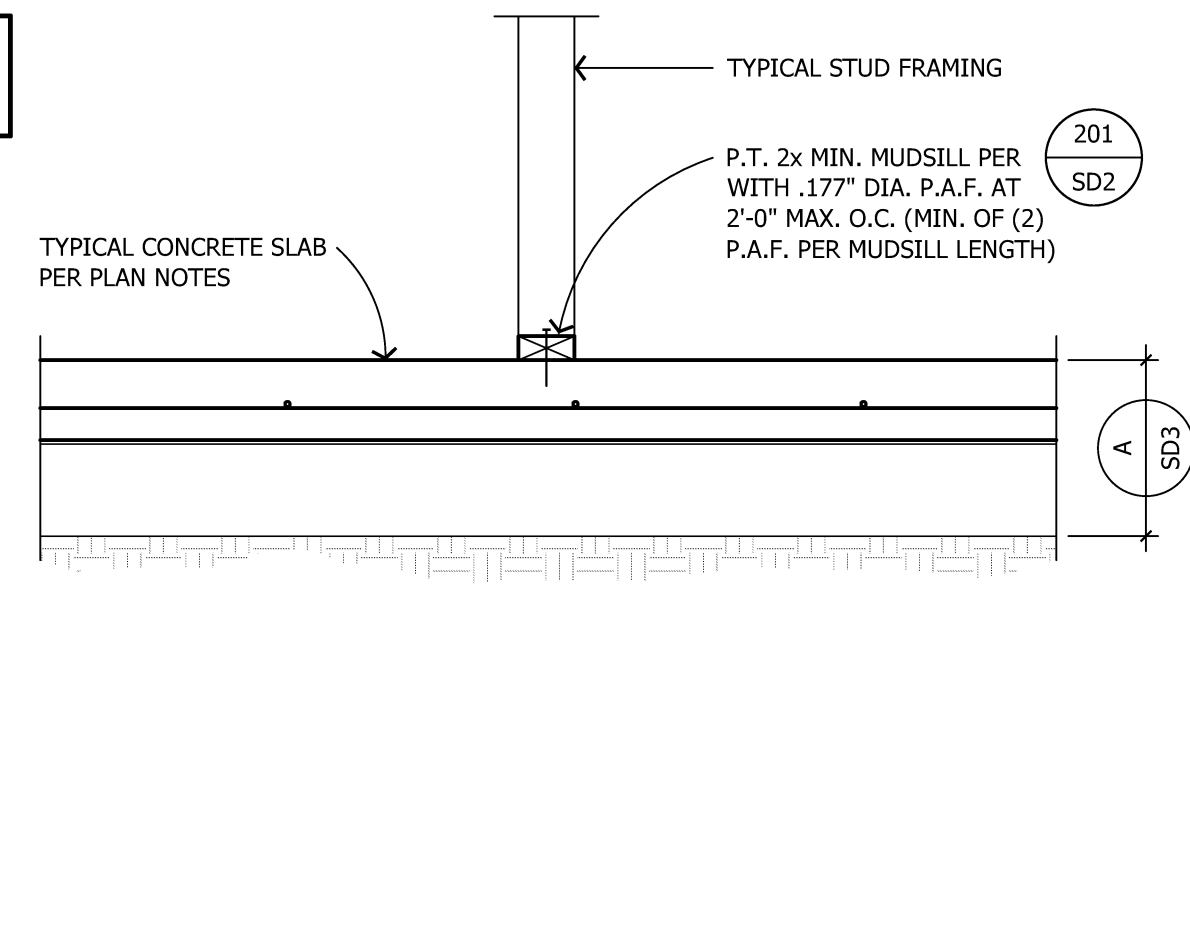
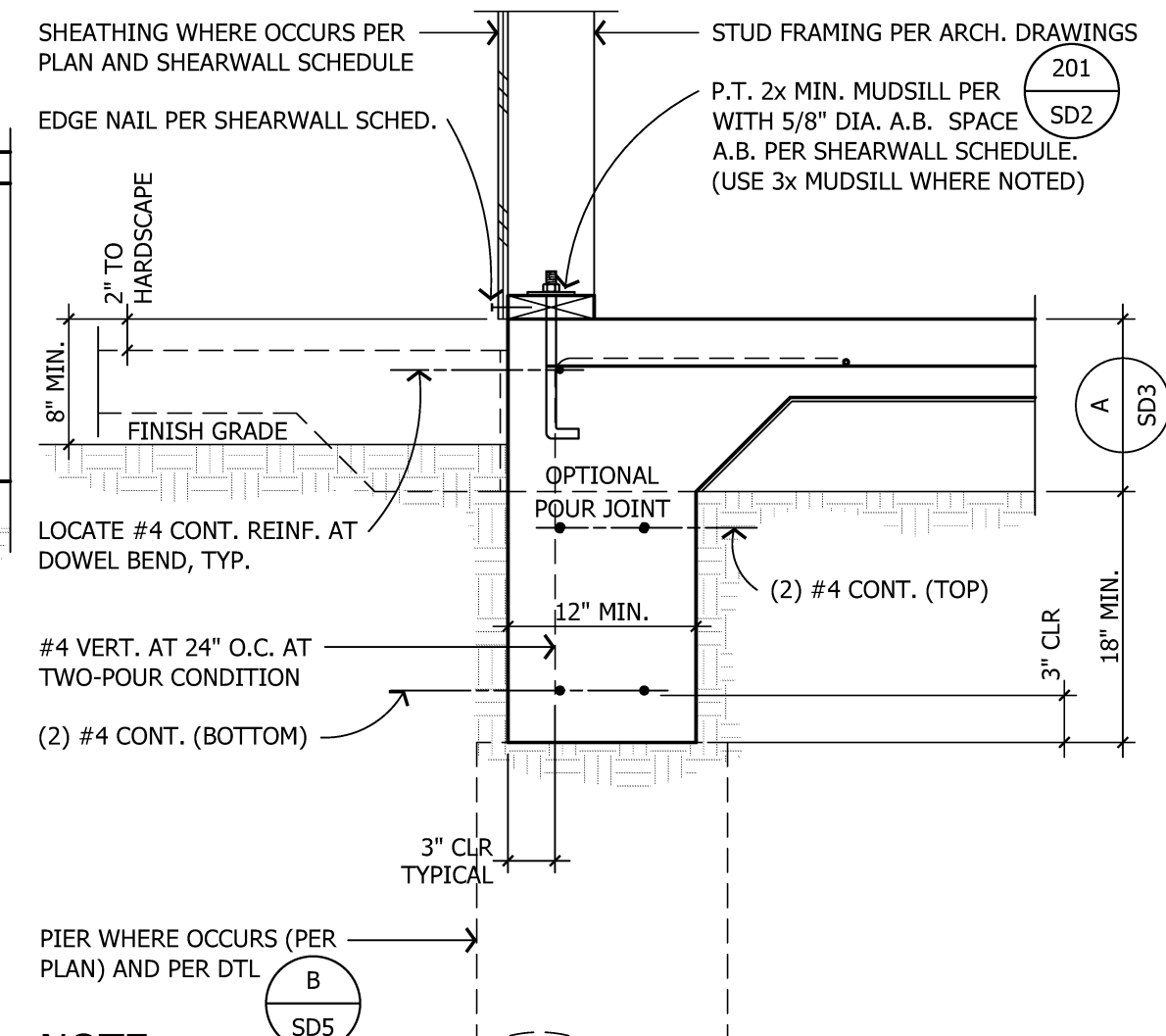
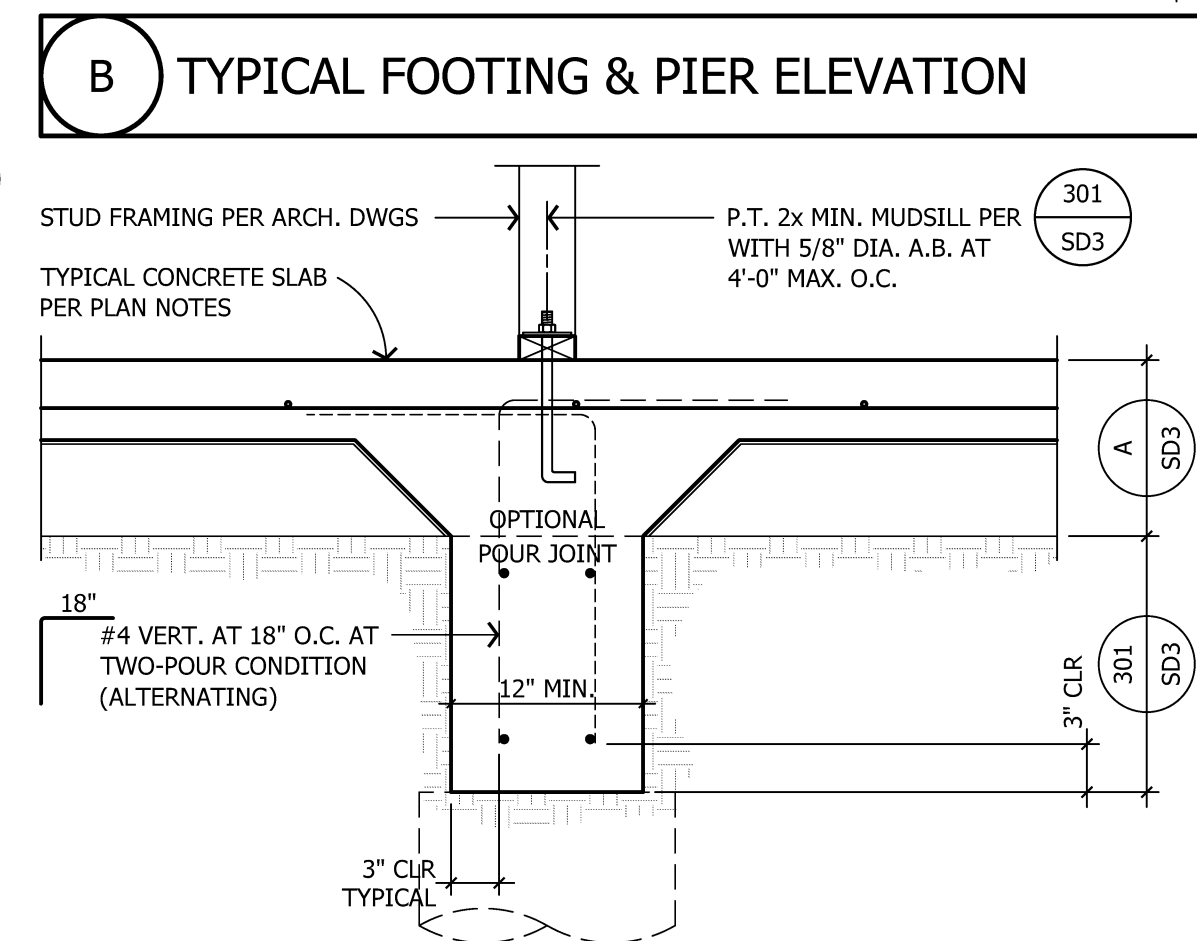
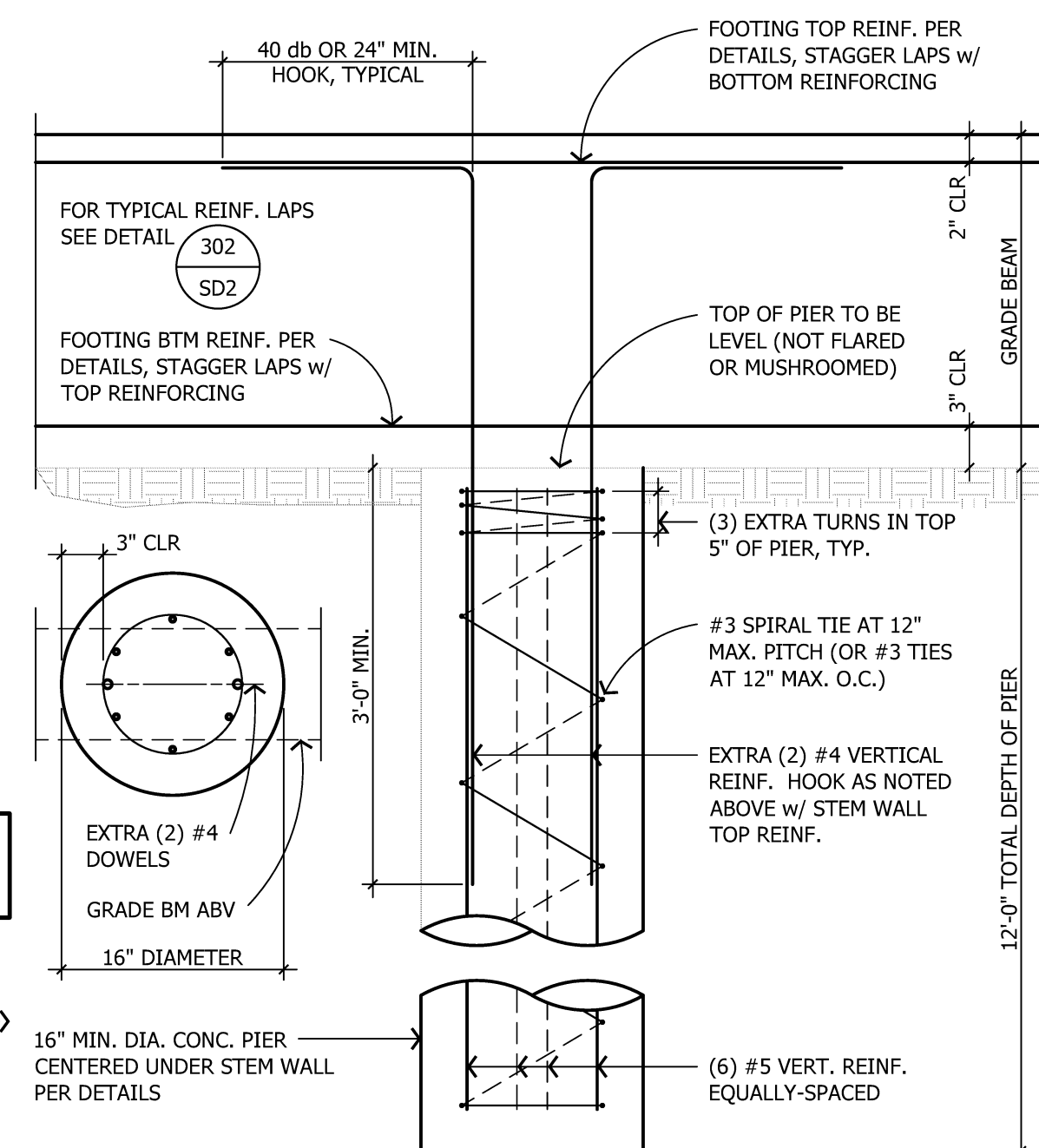
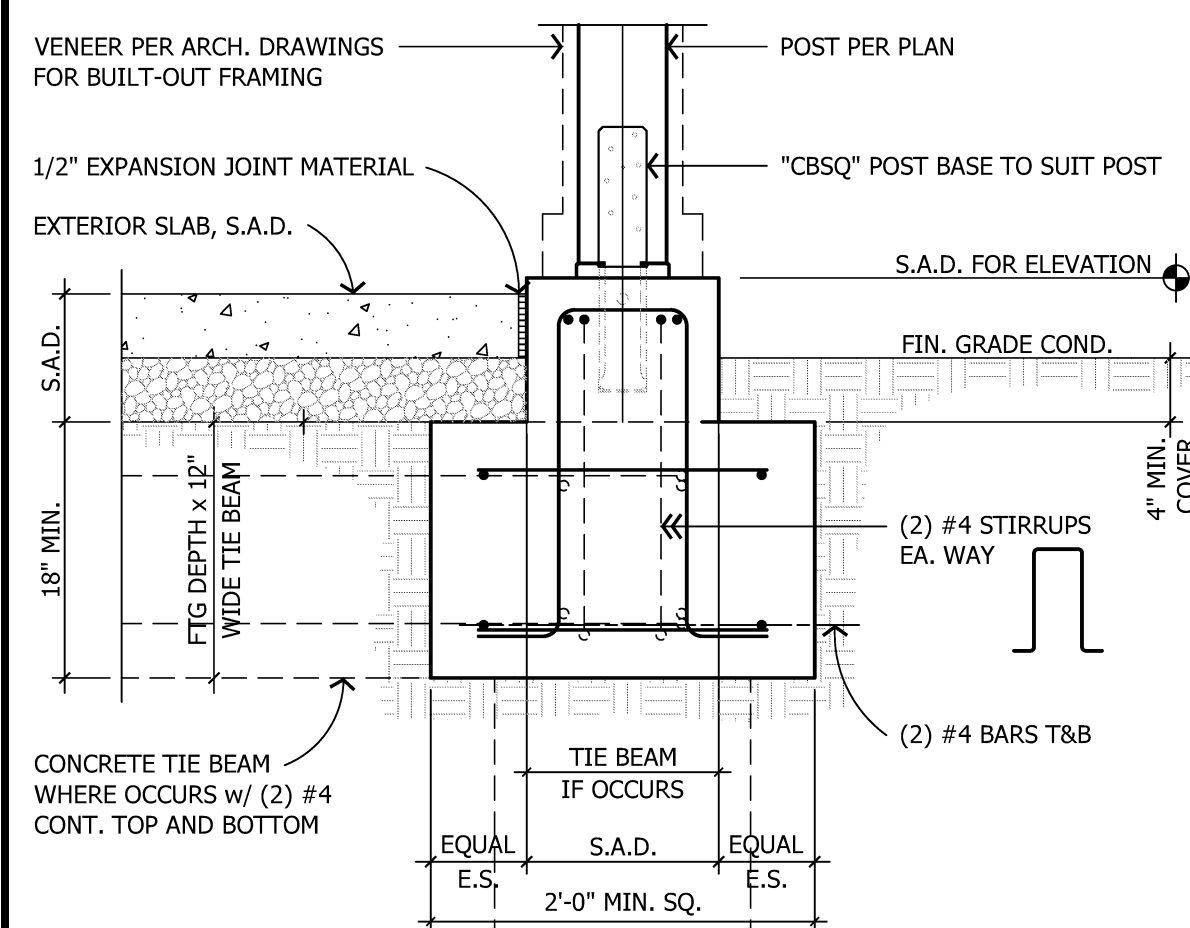
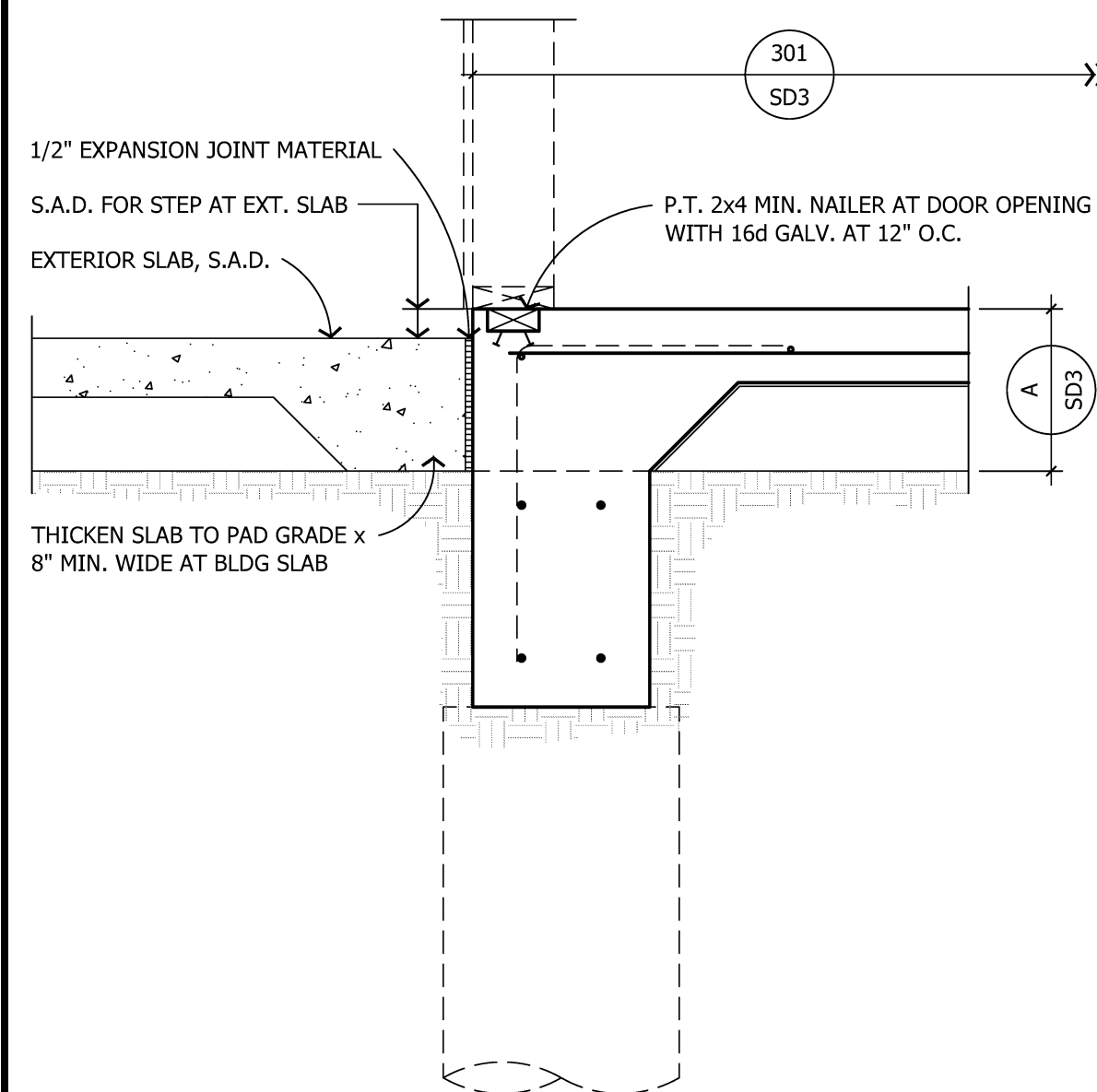
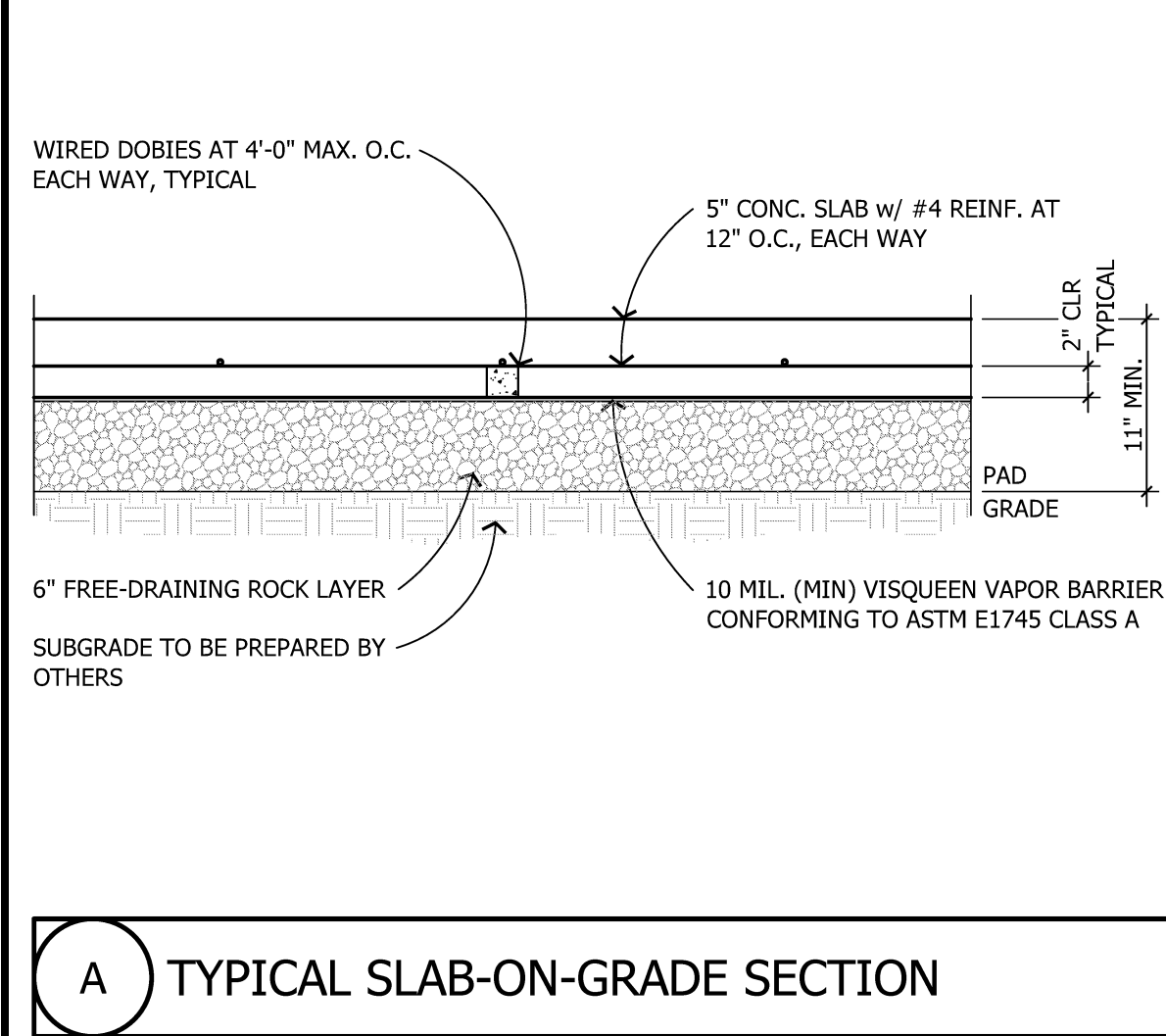
Project Designer: J. Peek

Project Manager: J. Peek Ext. 23

Job No. W021420

Sheet No.

SD2

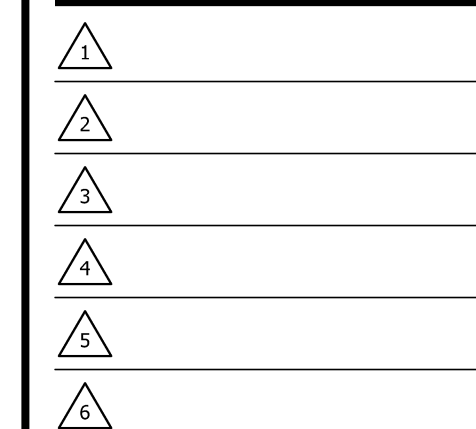


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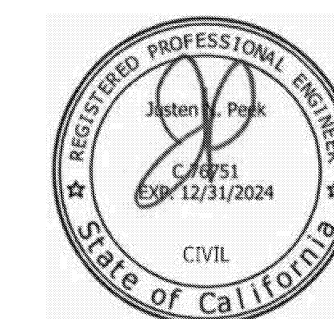
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Engineering Seal:

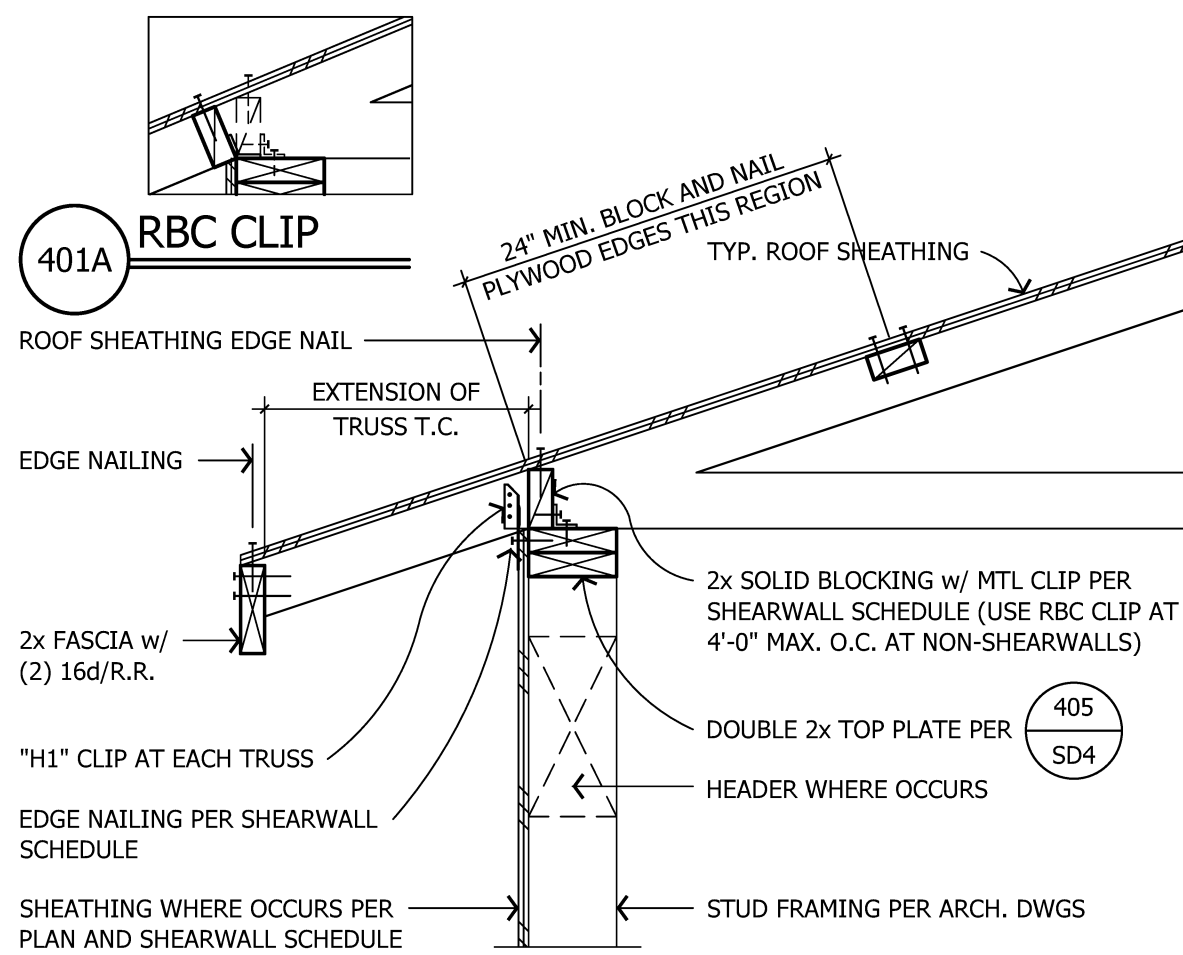


Sheet Description:
FOUNDATION DETAILS
SLAB ON GRADE w/ PIERS

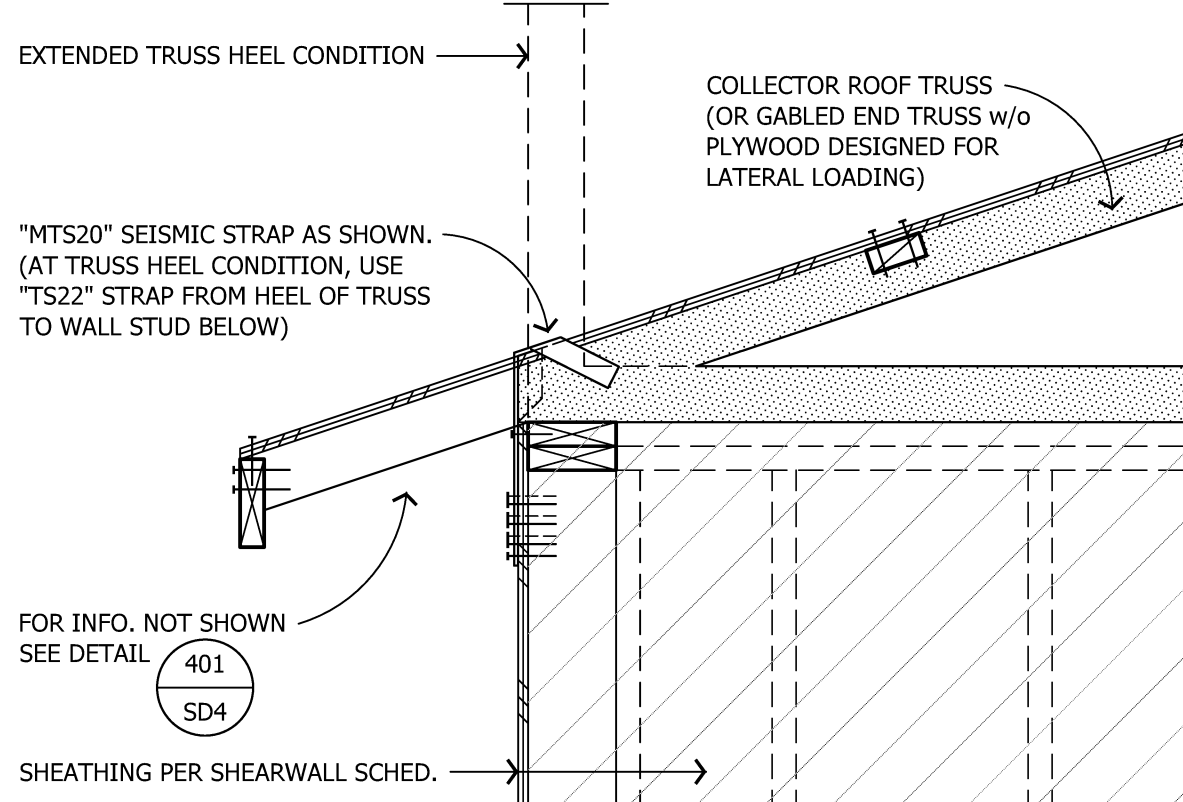
Scale: 1" = 1'-0"	
Initial Issue Date:	March 24, 2023
Drawn By:	E. Bennett
Project Designer:	J. Peek
Project Manager:	J. Peek Ext. 23
Job No.	W021420

Sheet No.

SD3

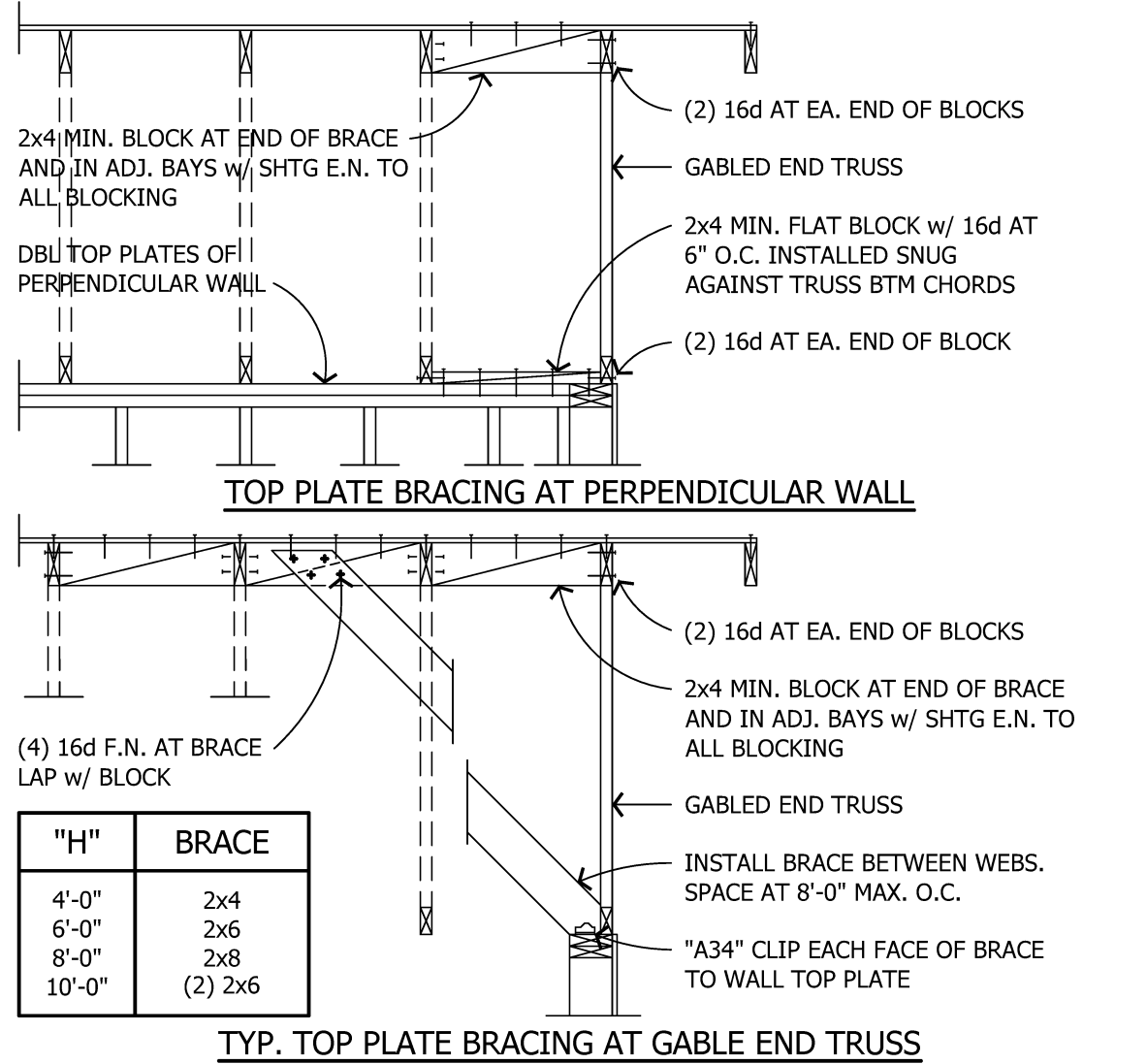


401 TRUSS PERPEND. TO EXTERIOR WALL

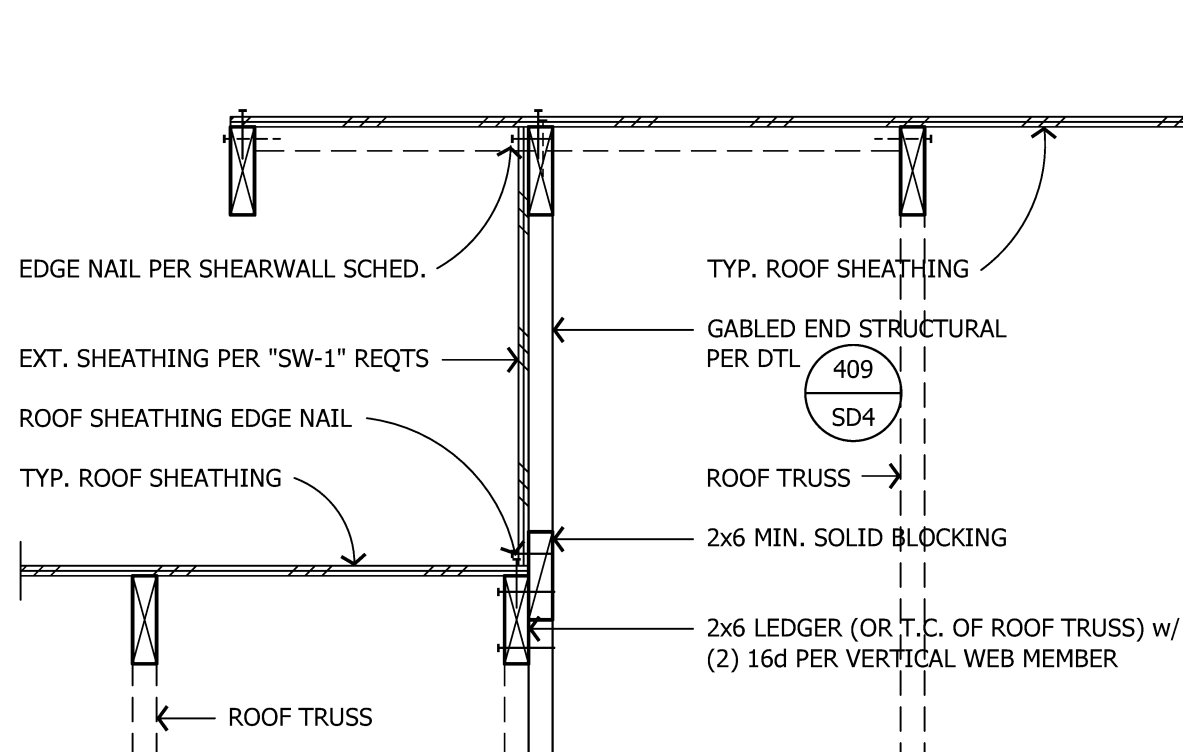


TYPICAL AT ENDS OF COLLECTOR AND GABLED END TRUSSES WHERE SHEARWALL SHEATHING DOES NOT EXTEND UP TO TOP CHORD.

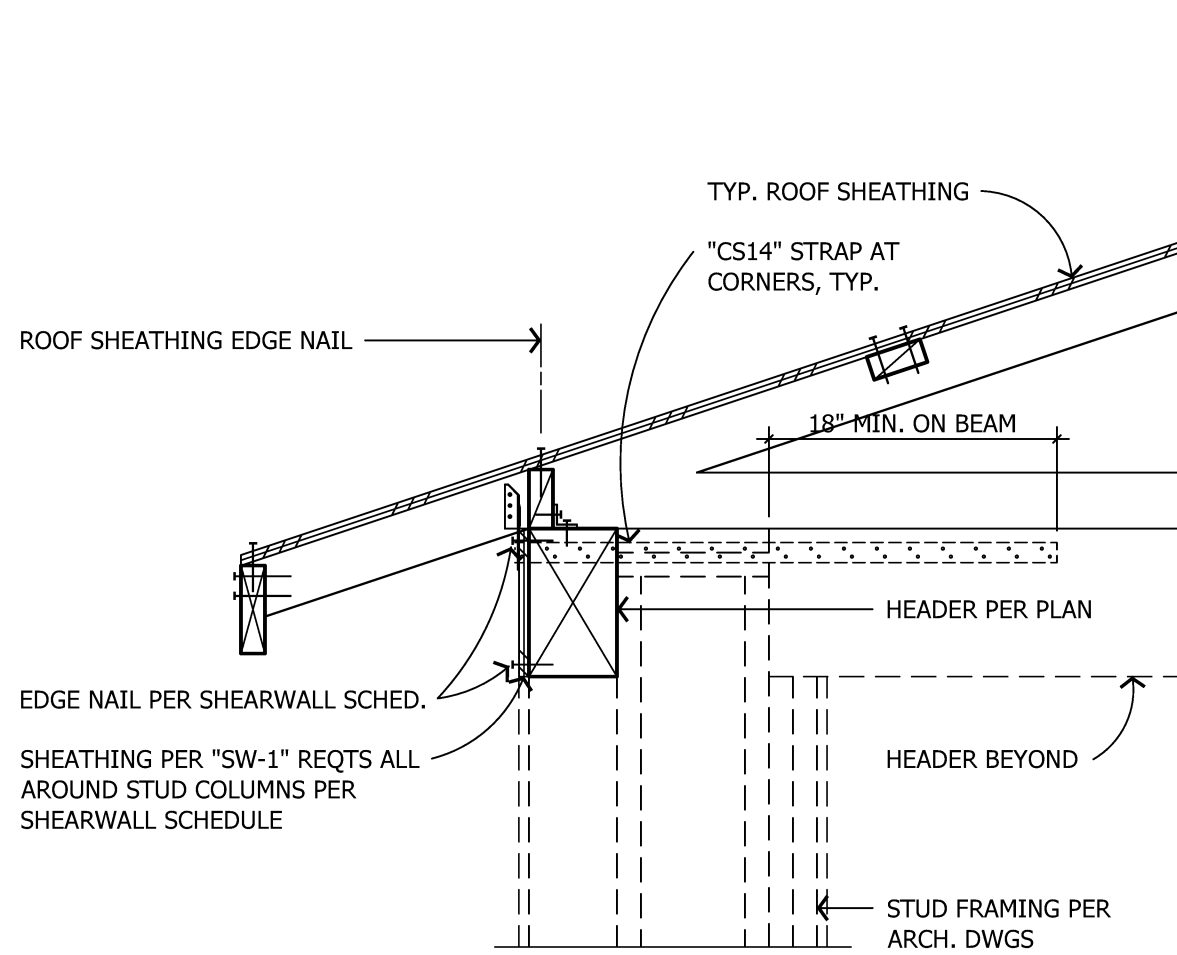
406 COLLECTOR TRUSS UPLIFT ANCHORAGE



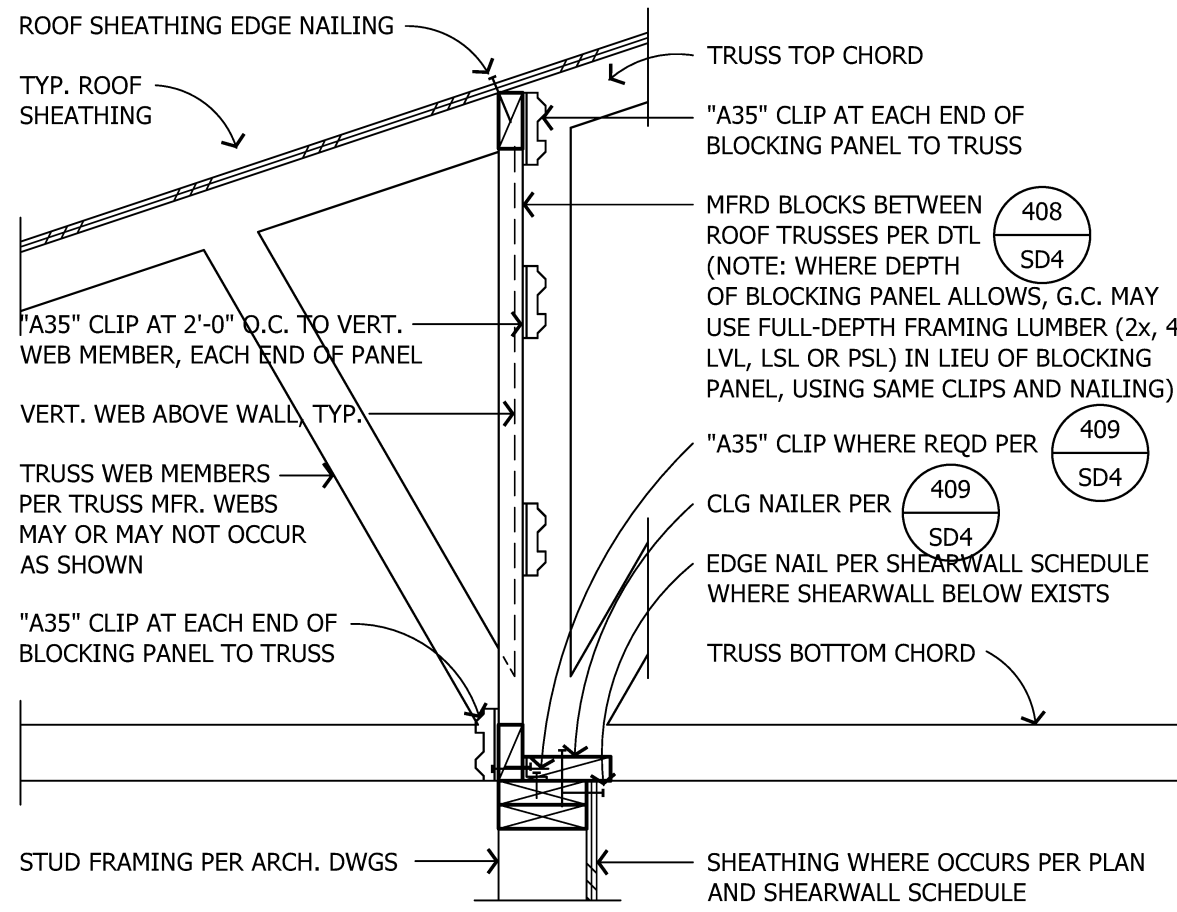
410 GABLE END TRUSS BRACING



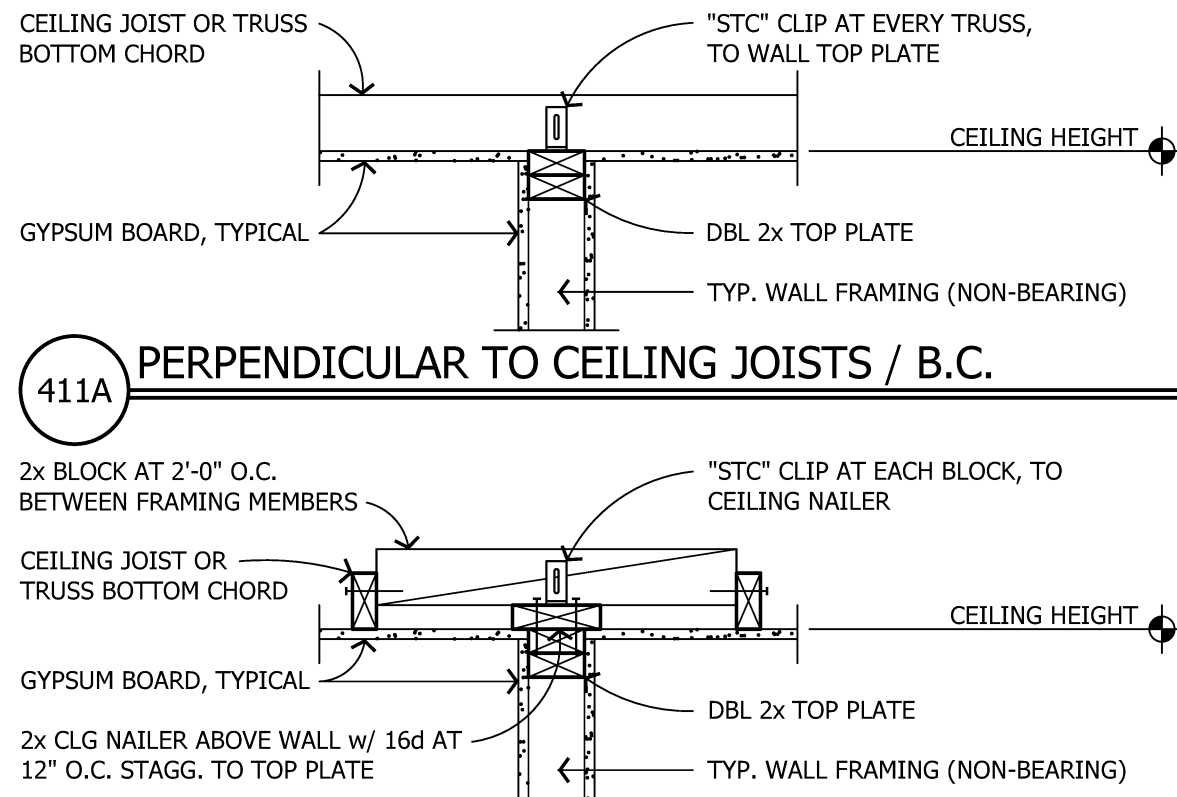
415 ROOF STEP



402 TRUSS PERPEND. TO PORCH BEAM



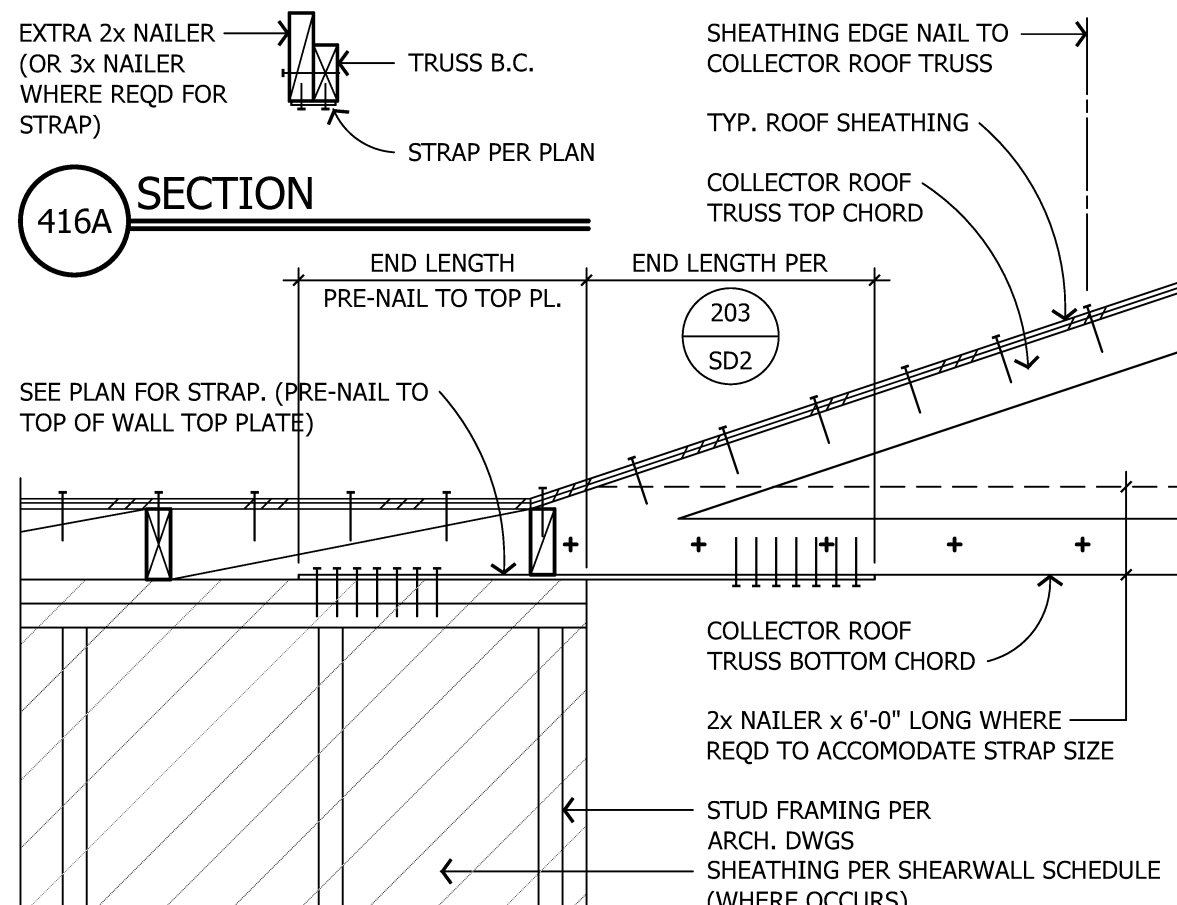
407 TRUSS TO INTERIOR SHEAR WALL



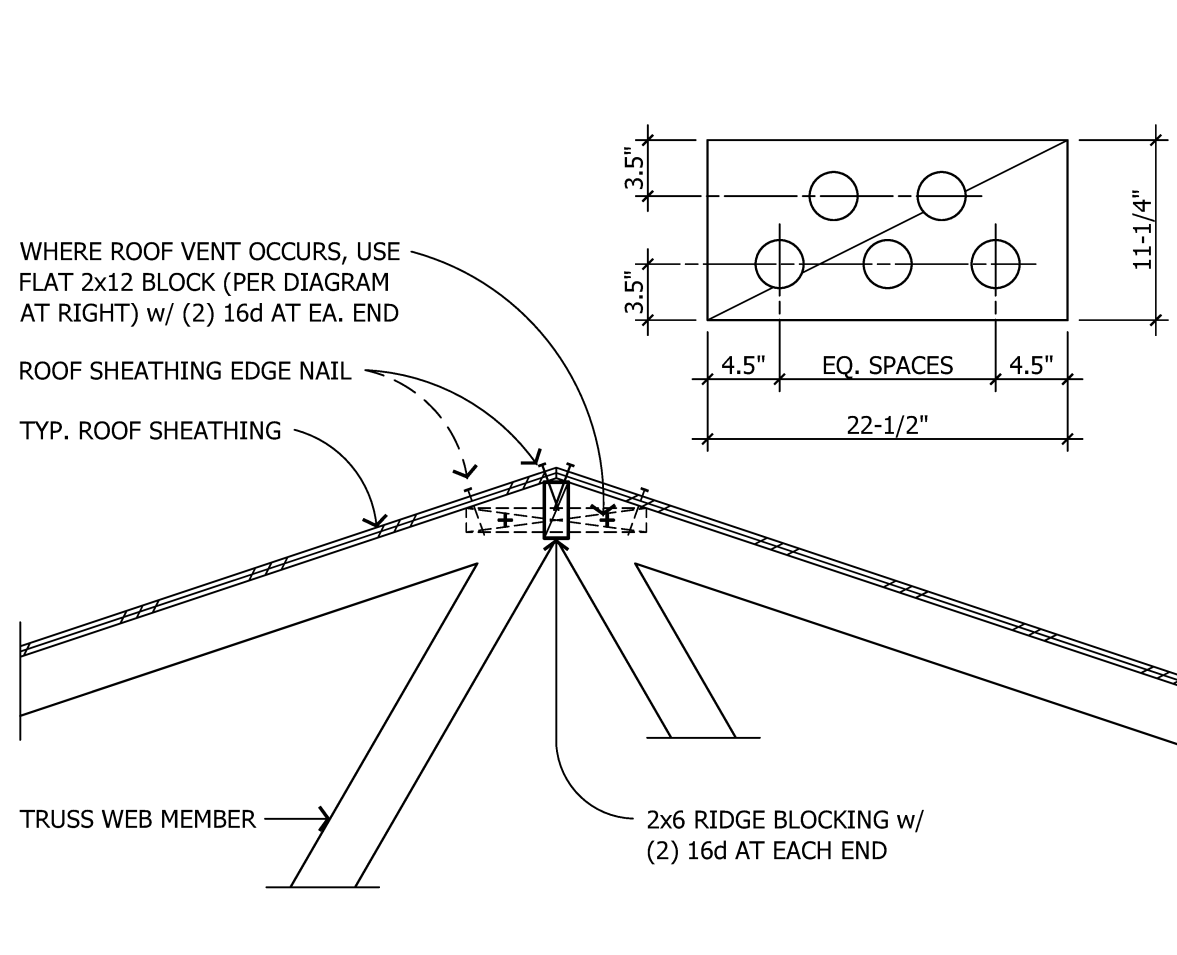
411A PERPENDICULAR TO CEILING JOISTS / B.C.

411B PARALLEL TO CEILING JOISTS / B.C.

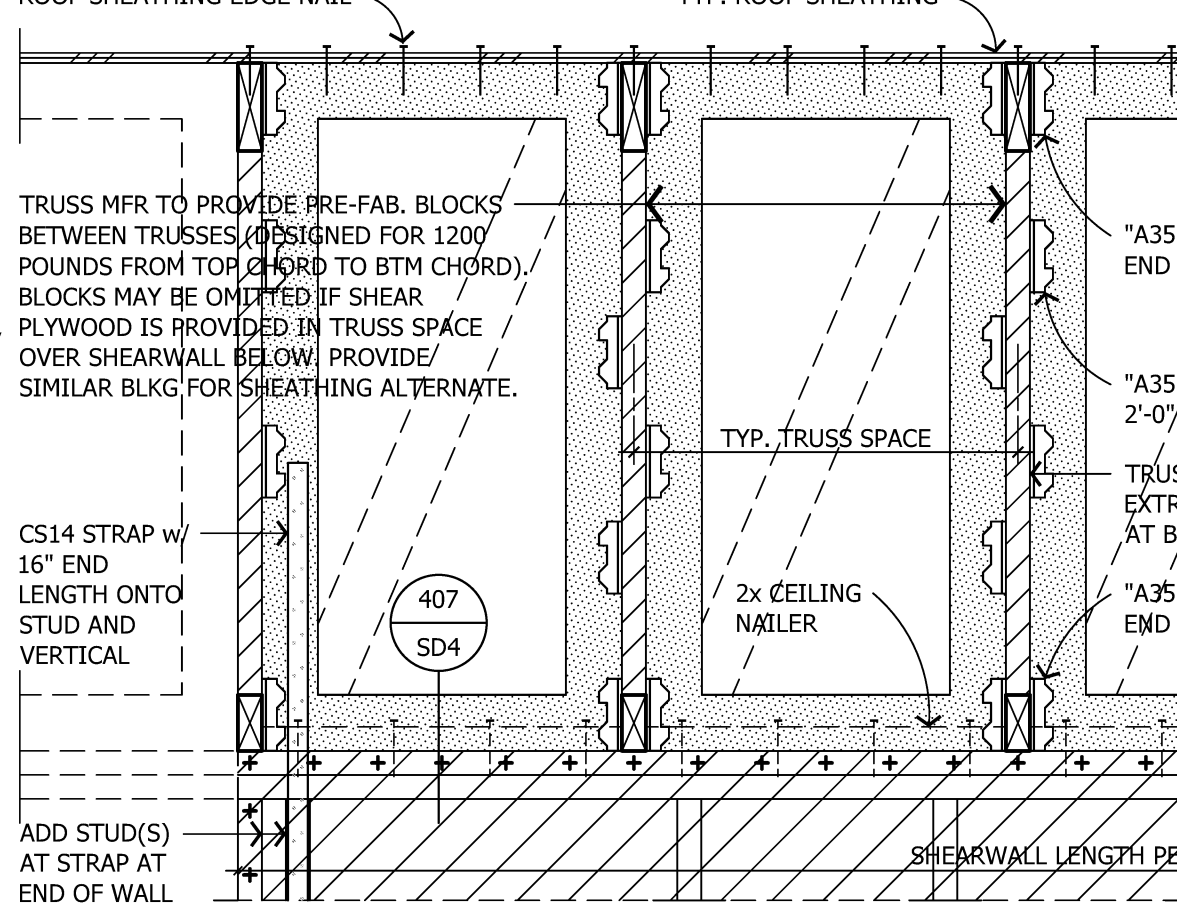
411 INTERIOR NON-BEARING WALL TO TRUSS



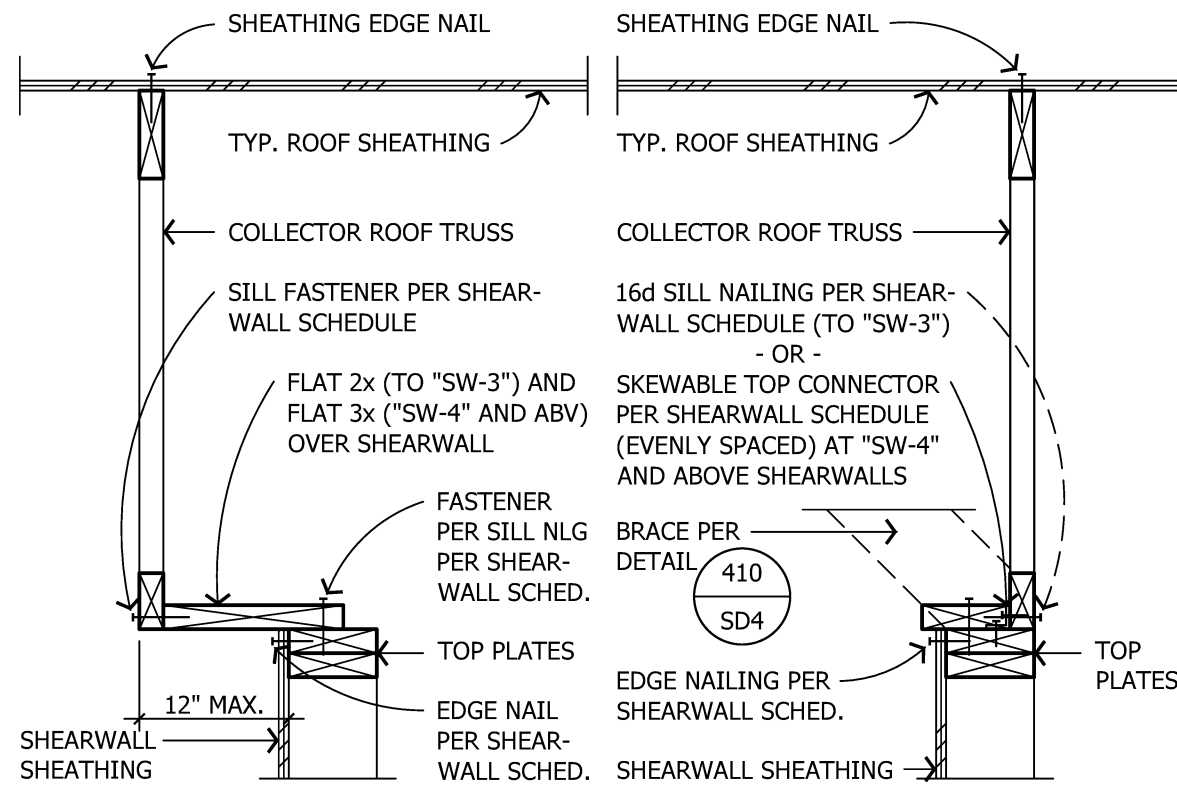
416 HORIZONTAL STRAP TO WALL TOP PLATE



403 RIDGE BLOCKING AT ROOF TRUSSES



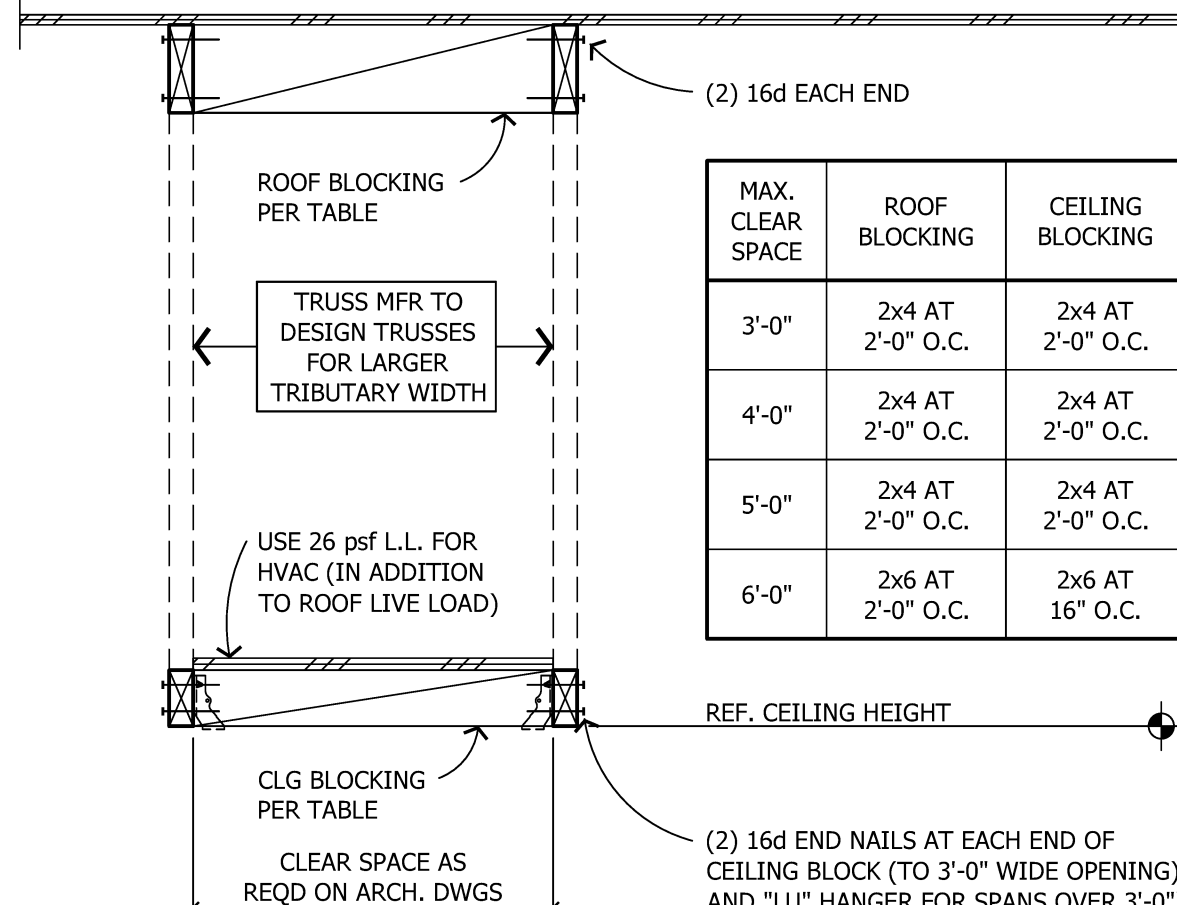
408 FABRICATED TRUSS BLOCKING PANEL ELEVATION ABOVE SHEARWALL



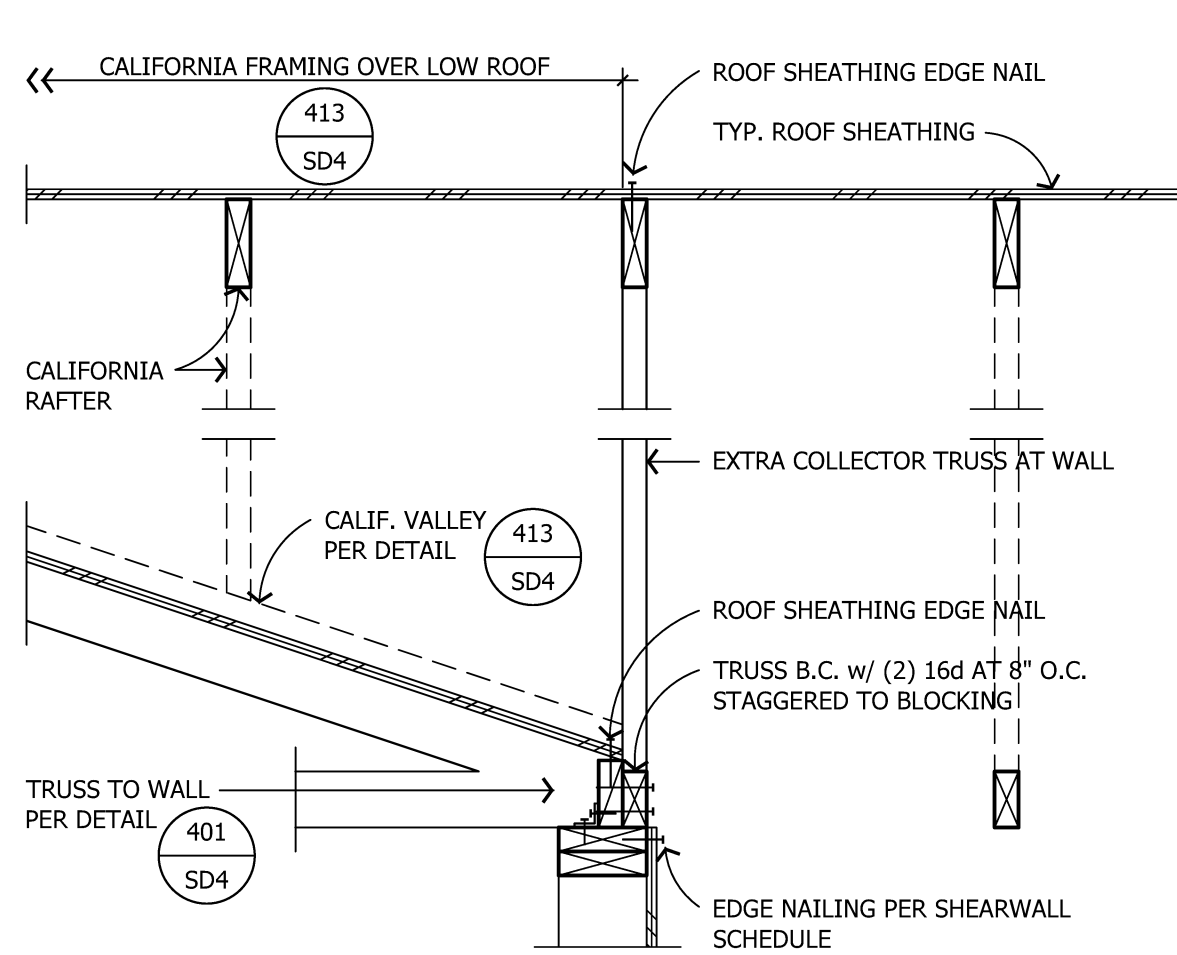
412A OFFSET SHEARWALL

412B ALIGNED SHEARWALL

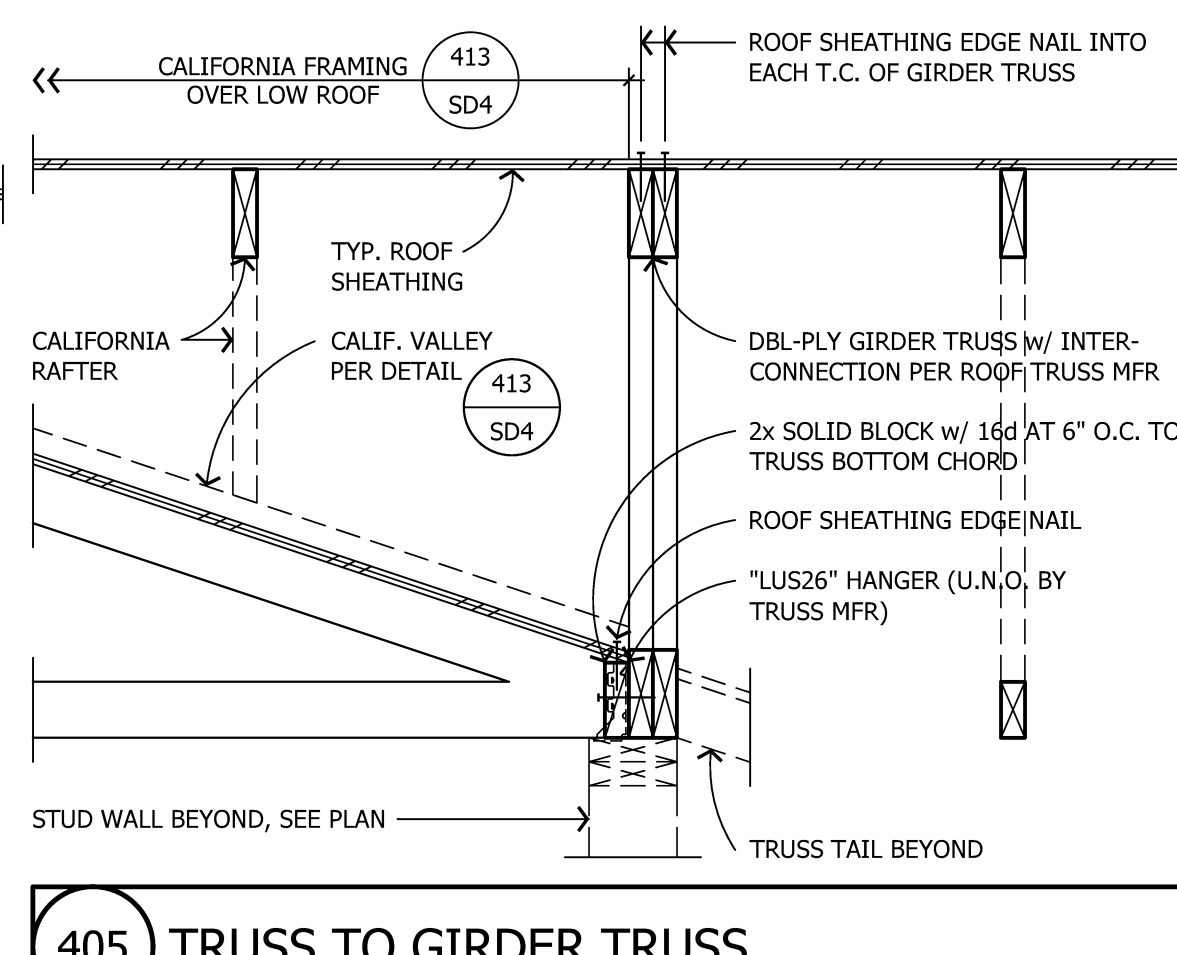
412 PARALLEL TRUSS TO INTERIOR SHEARWALL



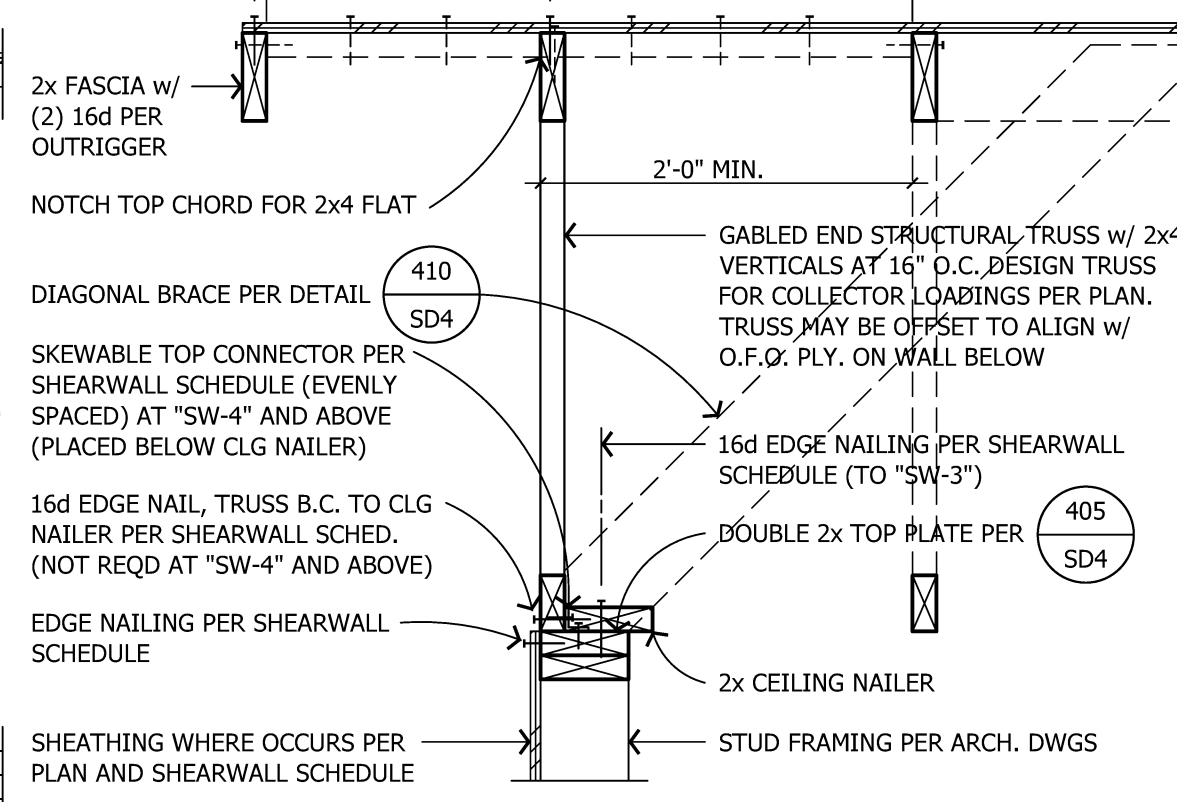
417 ROOF TRUSSES AT HVAC / ATTIC ACCESS



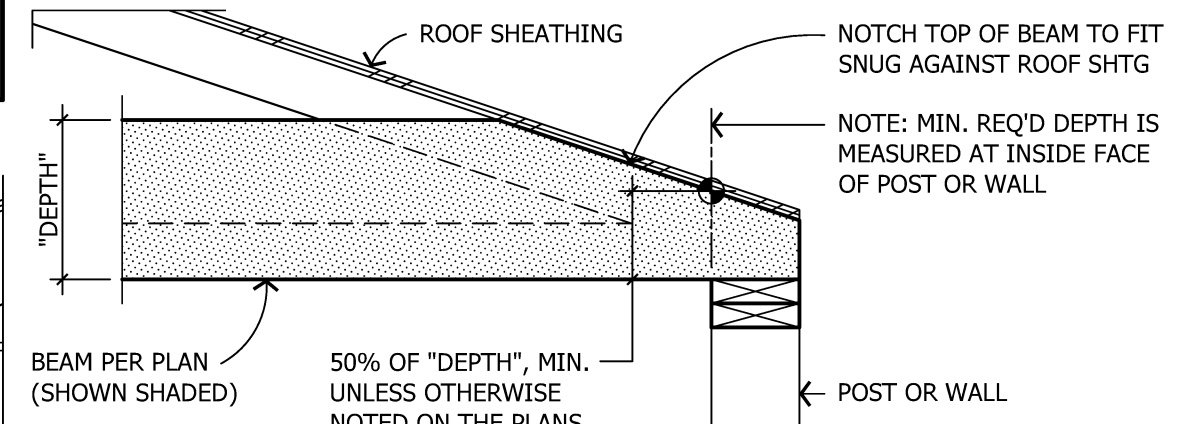
404 TRUSS TO INTERIOR BEARING WALL



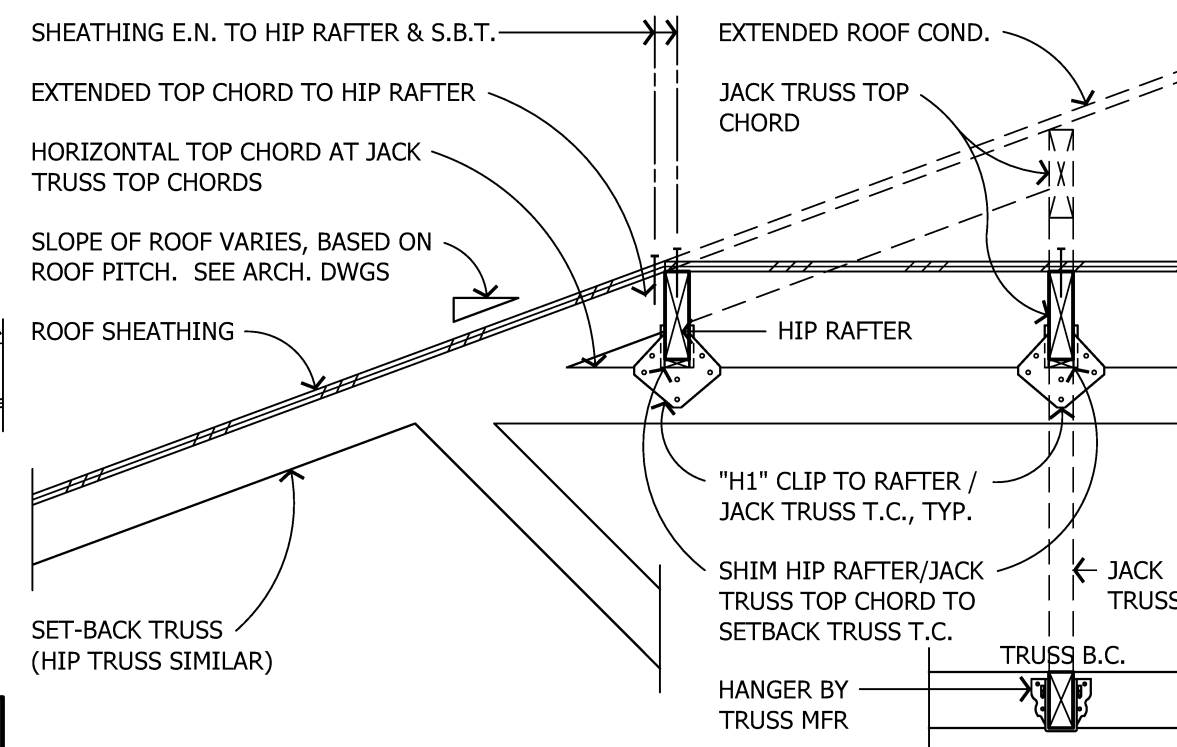
405 TRUSS TO GIRDER TRUSS



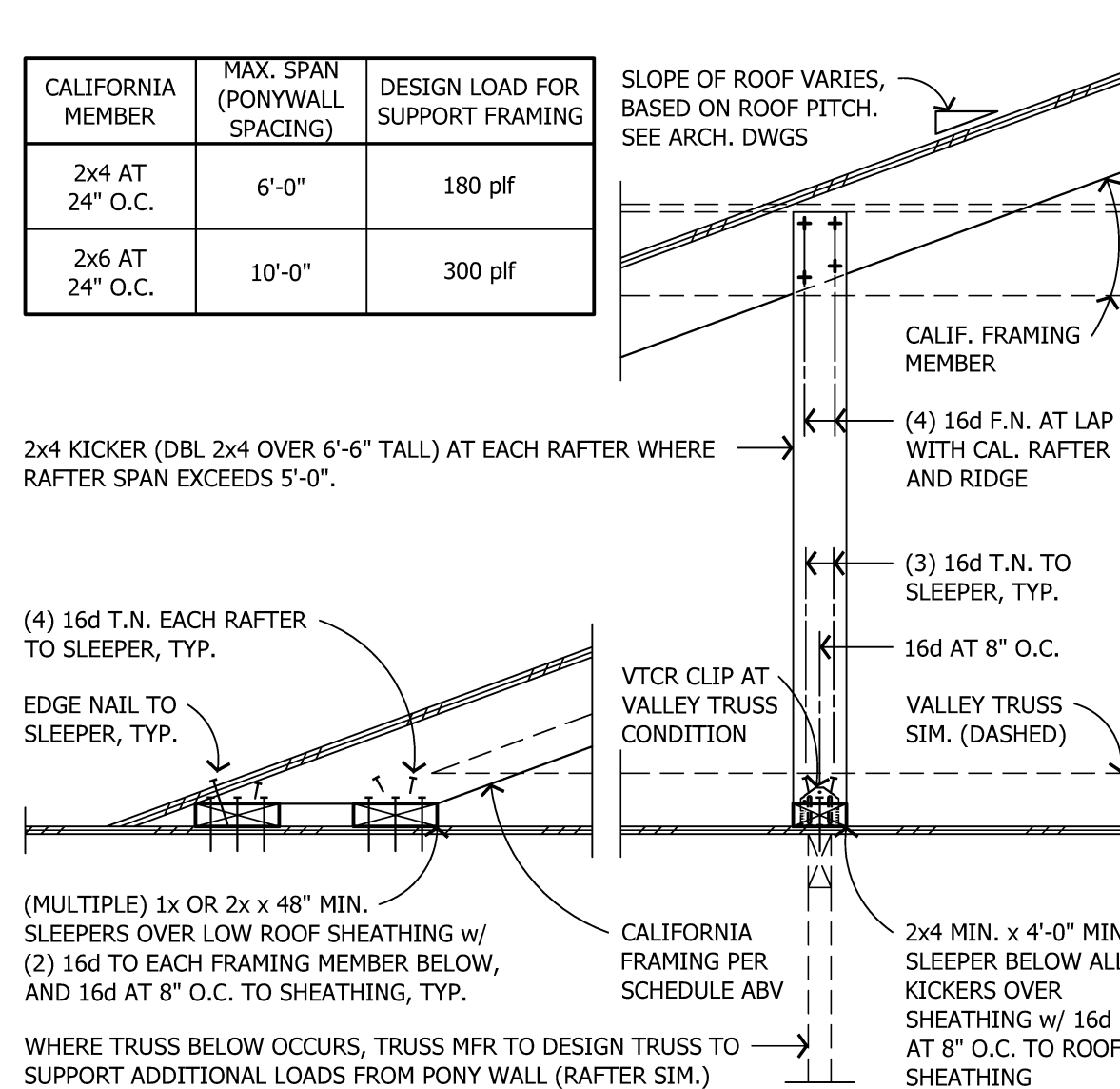
409 GABLED END TRUSS



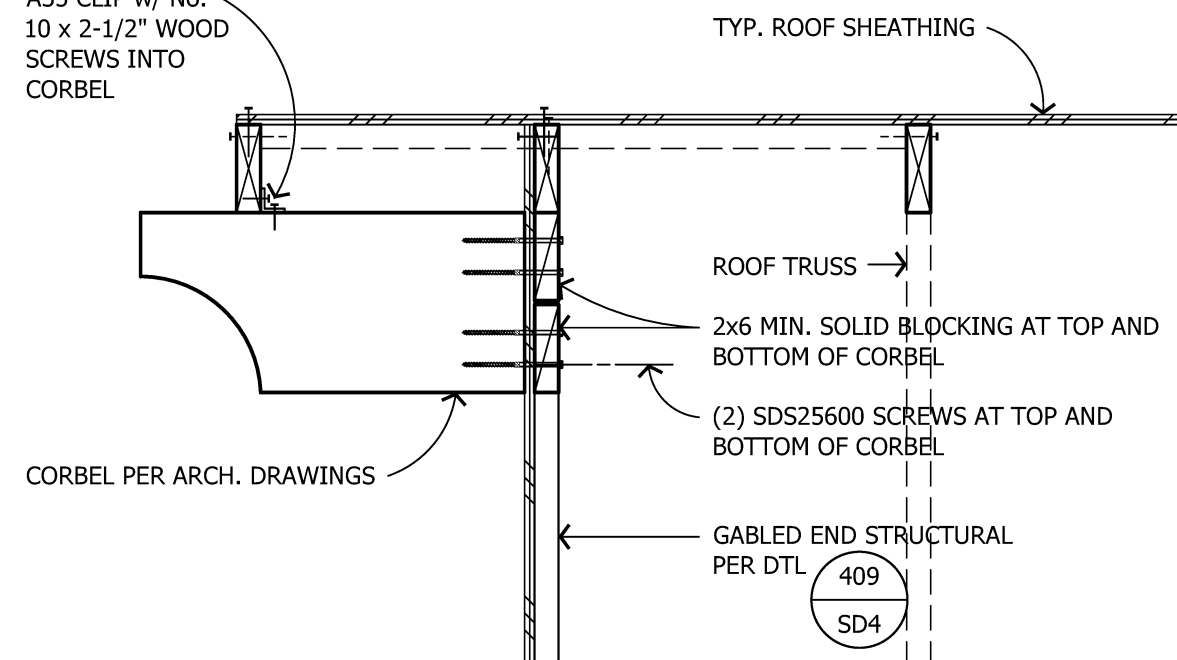
414 SHAPED TOP OF BEAM AT EAVE



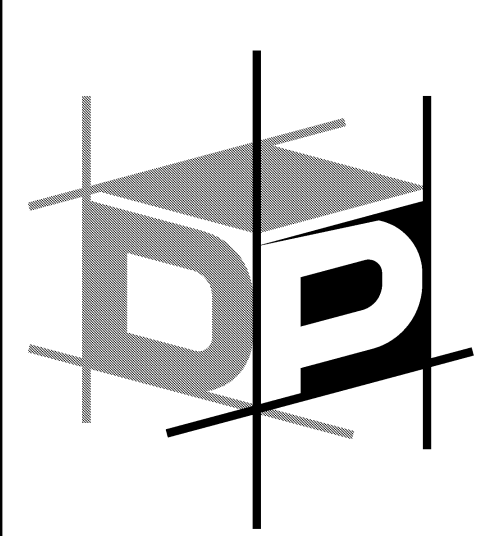
419 CONNECTION AT SET-BACK TRUSS



413 CALIFORNIA FRAMING DETAIL



418 CORBEL BEAM AT GABLED END TRUSS



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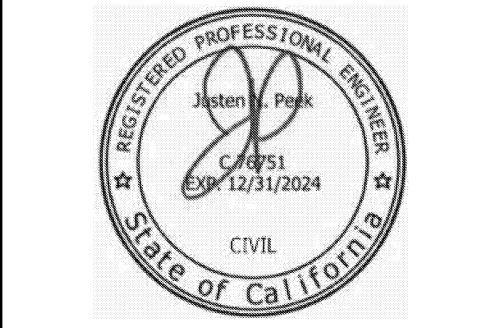
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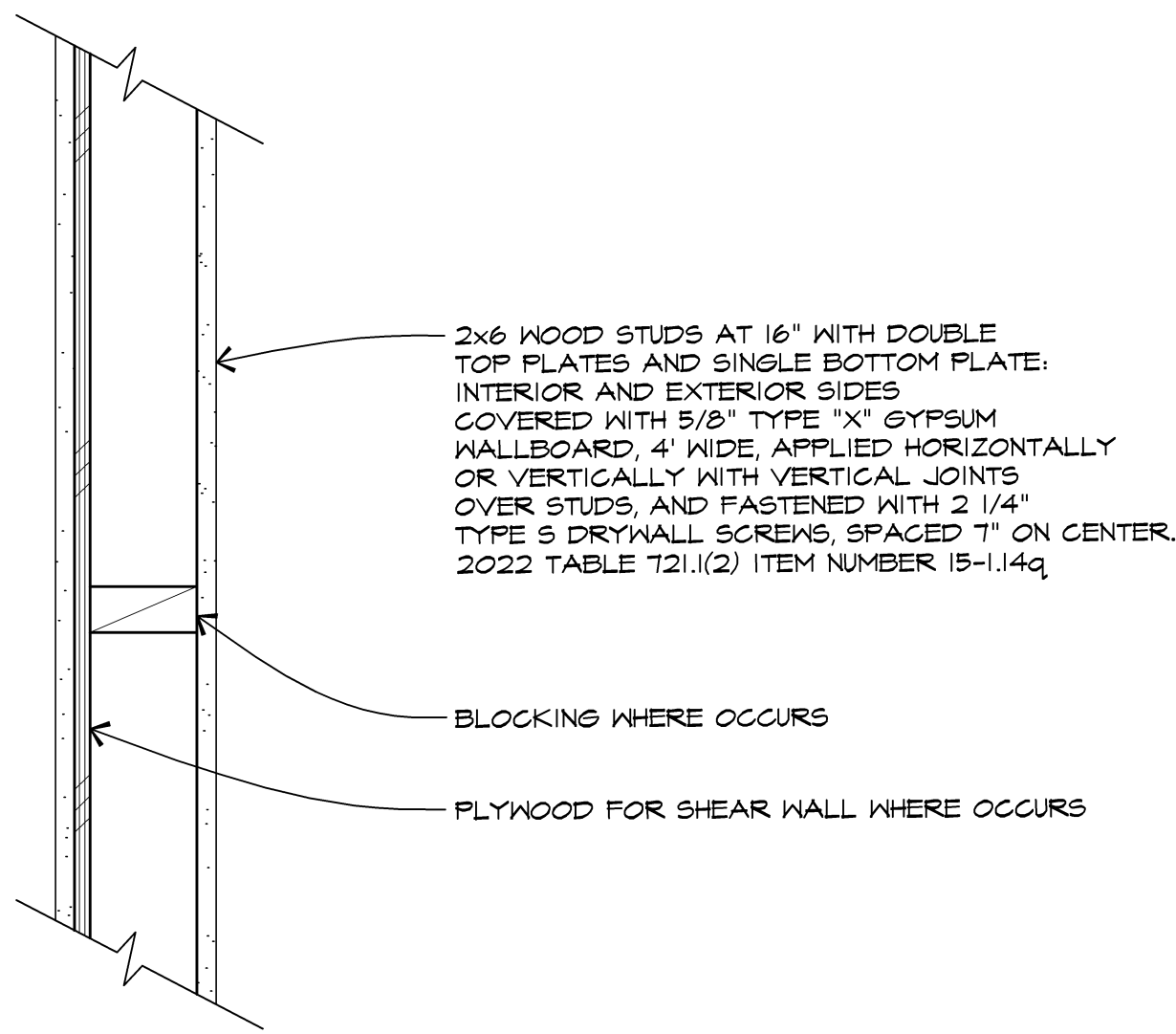
PLEASE NOTE THE REVISION NUMBER AND DATE ARE FOR STRUCTURAL SHEETS ONLY.
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Sheet Description:
ROOF FRAMING DETAILS

Scale: 1" = 1'-0"
Initial Issue Date: March 24, 2023
Drawn By: E. Bennett
Project Designer: J. Peek
Project Manager: J. Peek Ext. 23
Job No. W021420
Sheet No.

SD4



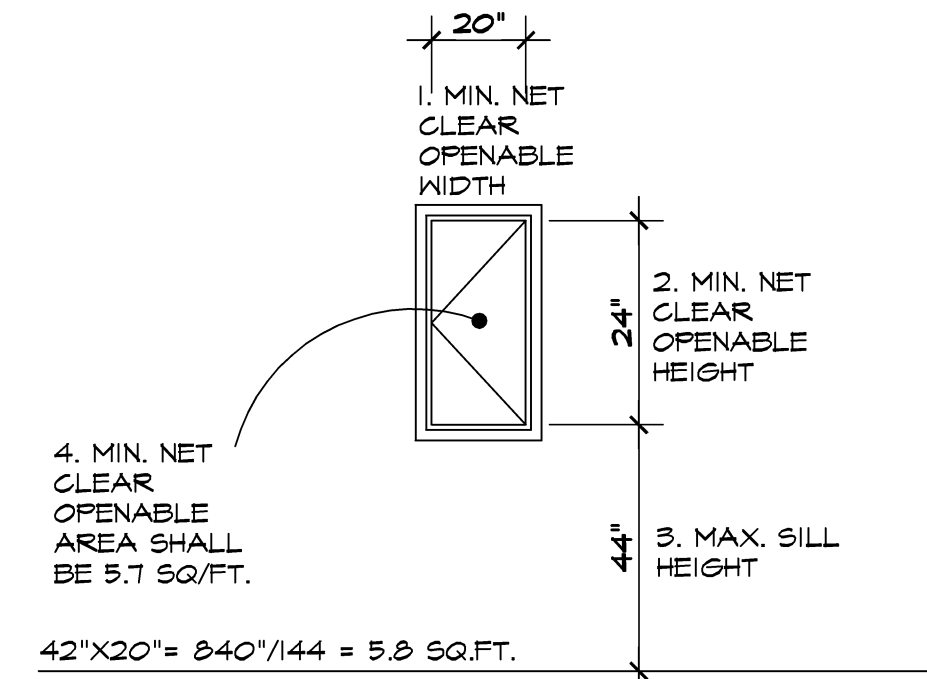
1
A1
1 HOUR FIRE RATED WALL DETAIL

FLOOR PLAN NOTES:

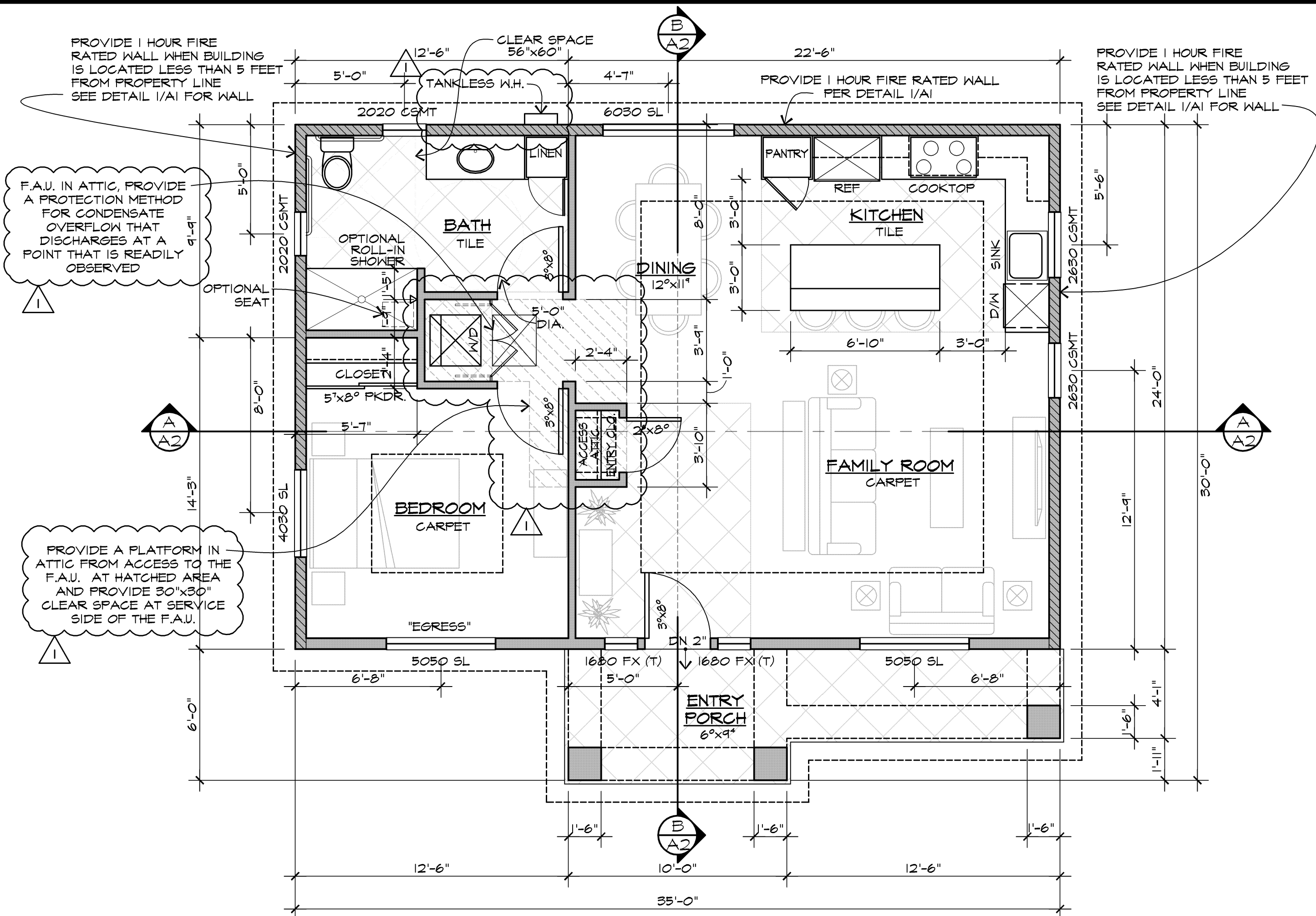
- 1) ALL NEW EXTERIOR WALLS TO BE 2x6 FRAMED STUD WALLS.
- 2) WINDOWS MARKED W/ (T) SHALL BE TEMPERED SAFETY GLASS.
- 3) ALL WINDOW HEAD HTS. SHALL BE 8'-0" TYP., U.N.O.
- 4) ALL WINDOWS MARKED "EGRESS" SHALL MEET MIN. EMERGENCY EGRESS WINDOW STANDARDS OF MIN. NET CLEAR OPENING OF 5.7 SQ. FT. & MIN. NET CLEAR HEIGHT OF 24" & MIN. NET CLEAR WIDTH OF 34" CONTRACTOR SHALL VERIFY W/ WINDOW MANUFACTURE.
- 5) THERE MUST BE AT LEAST 15" INCHES BETWEEN THE CENTER OF PLUMBING FIXTURES SUCH AS WATER CLOSETS OR SINKS AND THE NEAREST WALL OR OBJECT.
- 6) CONTRACTOR SHALL VERIFY LOCATION & NUMBER OF A/C UNITS W/ OWNER. PROVIDE 4" CONC. SLAB W/ #10 W/M. ALL NECESSARY PLUMBING & ELECTRICAL.
- 7) MINIMUM OF 36-INCH DEEP LANDING IS REQUIRED IN DIRECTION OF TRAVEL. MAXIMUM RISE OF STEP IS 7.75-INCH.
- 8) BATHTUB AND SHOWER FLOORS AND WALLS ABOVE BATHTUBS WITH INSTALLED SHOWER HEADS AND IN SHOWER COMPARTMENTS SHALL BE FINISHED WITH NONABSORBENT SURFACE. SUCH WALL SURFACES SHALL EXTEND TO A HEIGHT OF NOT LESS THAN 6 FEET ABOVE THE FLOOR. CRC R307.2.
- 9) GYPSUM BOARD SHALL NOT BE USED WHERE THERE WILL BE DIRECT EXPOSURE TO WATER, OR IN AREAS SUBJECT TO CONTINUOUS HIGH HUMIDITY. CRC T02.3.7.

* EGRESS WINDOW DETAIL

EGRESS WINDOWS SHALL COMPLY W/ ALL 4 CONDITIONS ILLUSTRATED BELOW.

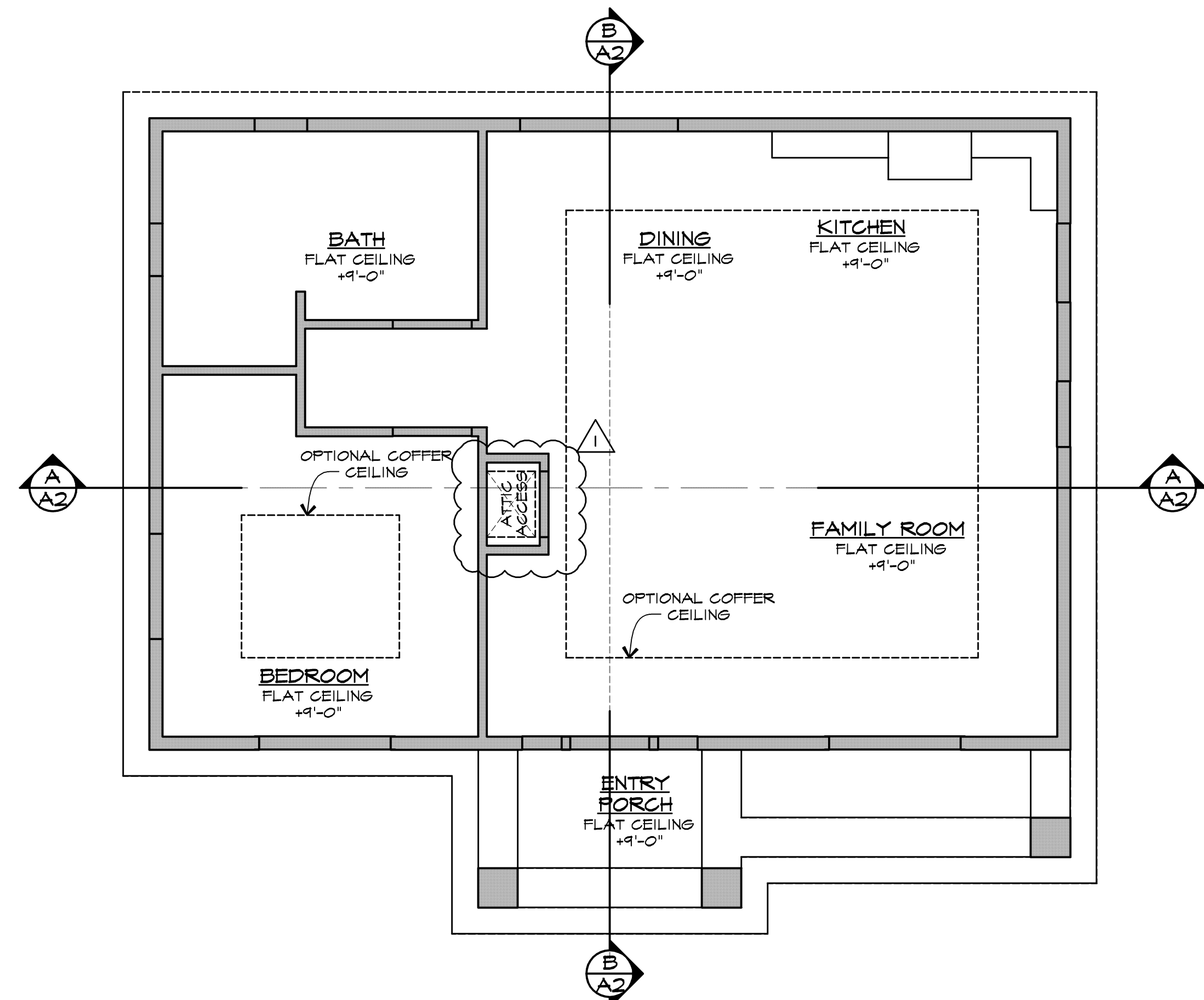


ESCAPE OR RESCUE WINDOWS SHALL HAVE A MINIMUM NET CLEAR OPENABLE AREA OF 5.7 SQ. FT. (0.53 M²). (EXCEPTION: THE MINIMUM NET CLEAR OPENING FOR EMERGENCY ESCAPE AND RESCUE GRADE-LEVEL OPENINGS SHALL BE 5 SQUARE FEET (0.46M²). THE MINIMUM NET CLEAR OPENABLE HEIGHT DIMENSION SHALL BE 24 INCHES (610 MM). THE MINIMUM NET CLEAR OPENABLE WIDTH DIMENSION SHALL BE 20 INCHES (508 MM). THE NET CLEAR OPENING DIMENSIONS SHALL BE THE RESULT OF NORMAL OPERATION OF THE OPENING WHEN WINDOWS ARE PROVIDED AS A MEANS OF ESCAPE OR RESCUE, THEY SHALL HAVE A FINISHED SILL HEIGHT NOT MORE THAN 44 INCHES (1118 MM) ABOVE THE FLOOR. CRC SECTION R310.1, R310.1.1, R310.1.2, R310.1.3, AND R310.1.4. CONTRACTOR NOTE: CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLIANCE WITH ALL EGRESS REQUIREMENTS.



FLOOR PLAN (CONTEMPORARY STYLE)

SCALE: 1/4" = 1'-0"

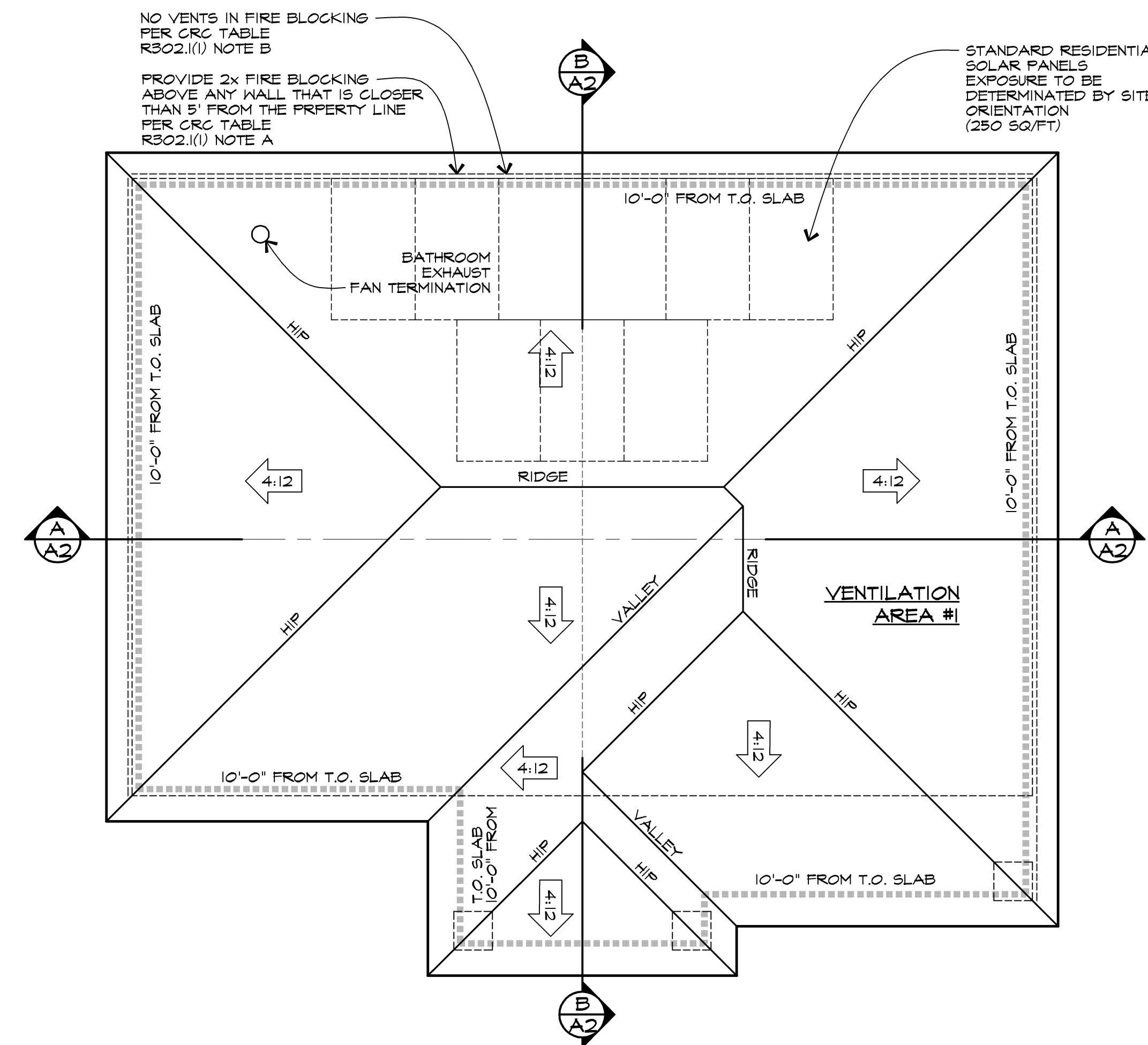


REFLECTED CEILING PLAN

SCALE: 1/4" = 1'-0"

VENTILATION CALCULATIONS

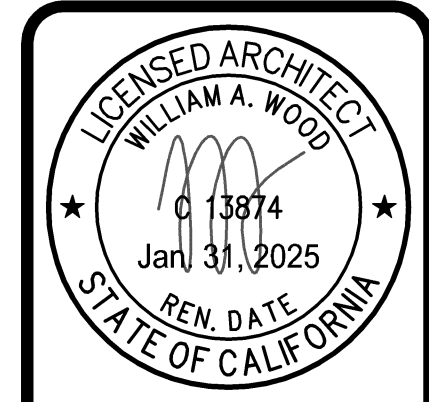
- AREA I
- * ATTIC AREA: 948 SQ. FT.
 - * REQUIRED AREA: 455 SQ. IN. (CONVERTED TO INCHES @ 1/300)
 - * UPPER AREA: 227.5 SQ. IN. (50% REQUIRED IN UPPER 1/3)
 - * PROVIDE 227.5 SQ. IN. IN UPPER 1/3 OFF ROOF WITH OHAGIN'S "FLAT" VENT (98.75 SQ. IN. FREE AREA) * 2 VENTS REQUIRED
 - * LOWER AREA: 227.5 SQ. IN. (50% REQUIRED IN LOWER 1/3)
 - * TO BE PROVIDED BY 32.5 VENTED FRIEZE BLOCKS MIN. (7 SQ. IN. FREE VENTING PER BLOCK VIA 3-2" DIA. HOLES)



ROOF PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	DATE
PLAN CHECK COMMENTS	4-25-23



DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

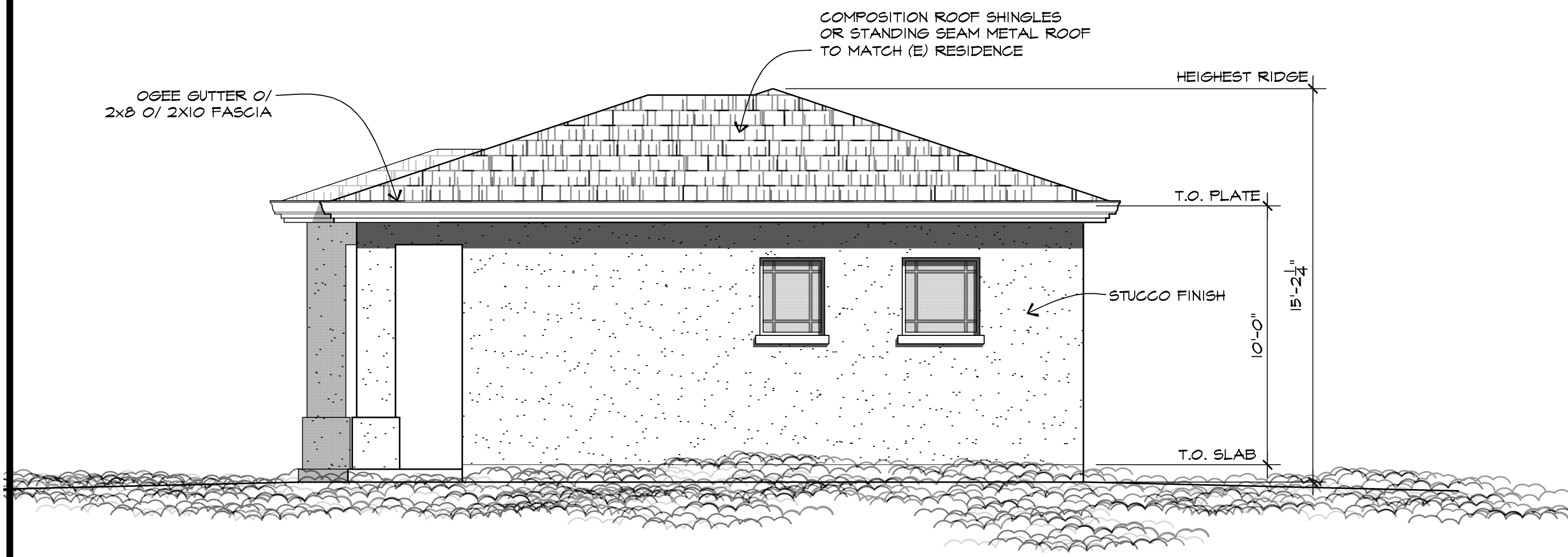
WILLIAM WOOD
ARCHITECTS

301 HARTZ AVENUE, SUITE 203
DANVILLE, CALIFORNIA 94526
(925) 820-8233

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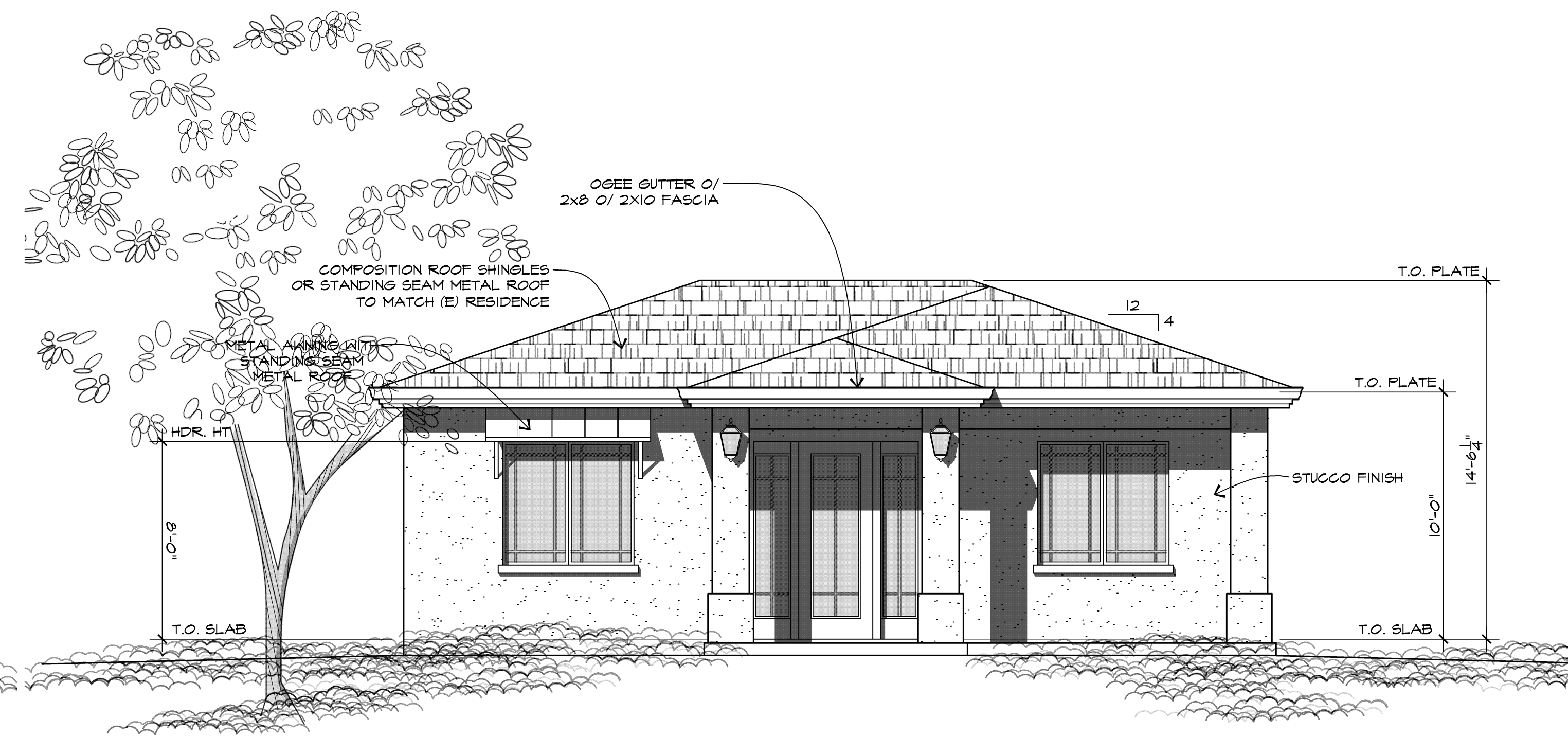
DRAWN JJ
CHECKED WW
DATE 03-30-23
SCALE AS SHOWN
JOB NO. 20.921R
SHEET A2
OF SHEETS

Apr 26, 2023 - 10:58am A2 One Bedroom Contemporary Style.dwg



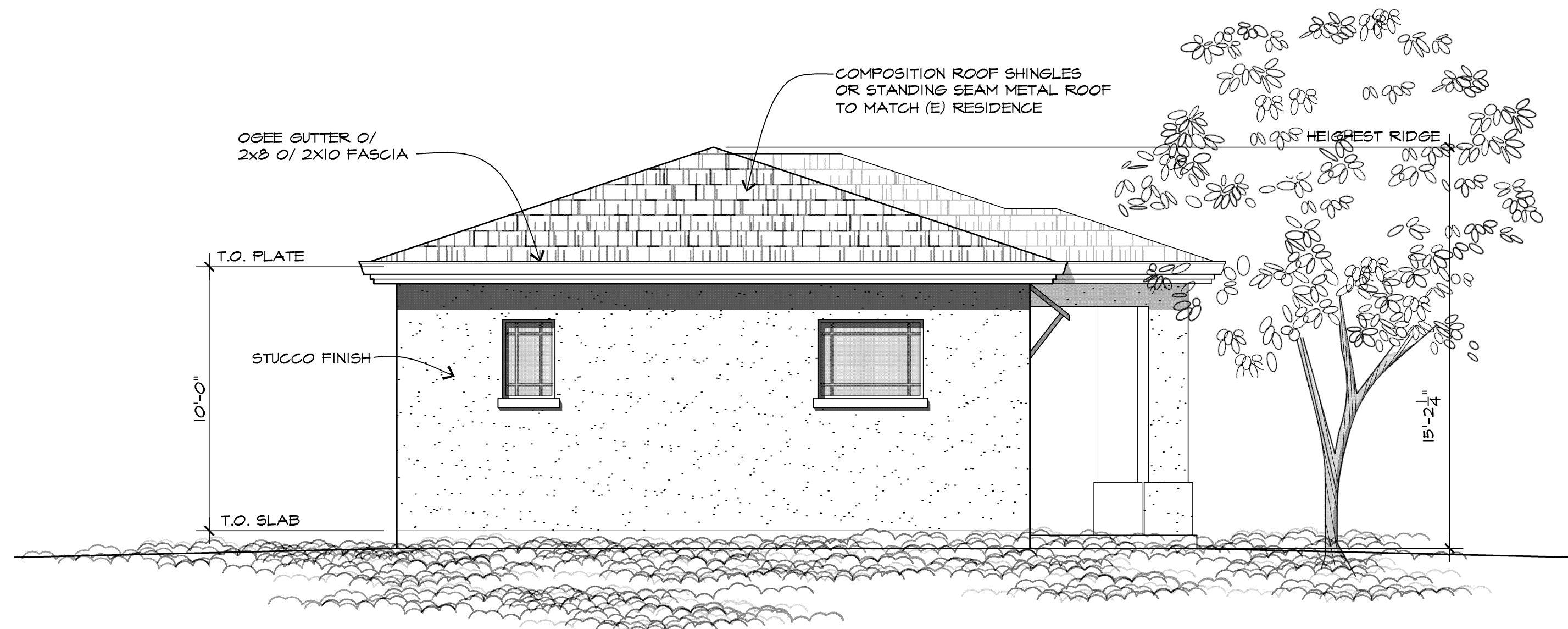
RIGHT SIDE ELEVATION - (CONTEMPORARY STYLE)

SCALE: 1/4" = 1'-0"



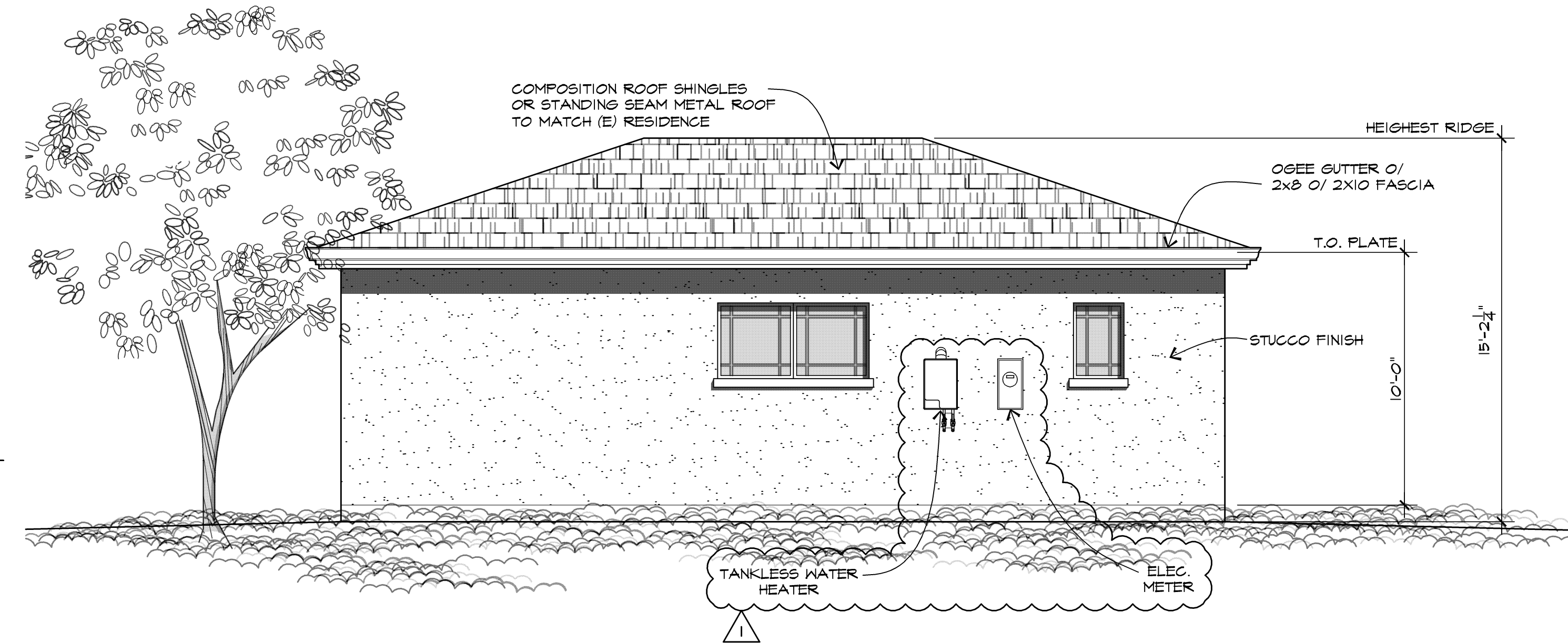
FRONT ELEVATION - (CONTEMPORARY STYLE)

SCALE: 1/4" = 1'-0"



LEFT SIDE ELEVATION - (CONTEMPORARY STYLE)

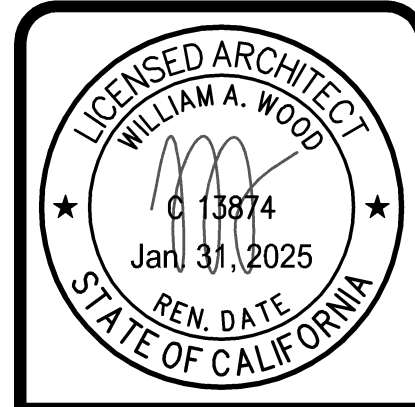
SCALE: 1/4" = 1'-0"



REAR ELEVATION - (CONTEMPORARY STYLE)

SCALE: 1/4" = 1'-0"

REVISIONS	DATE
PLAN CHECK COMMENTS	4-25-23



DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

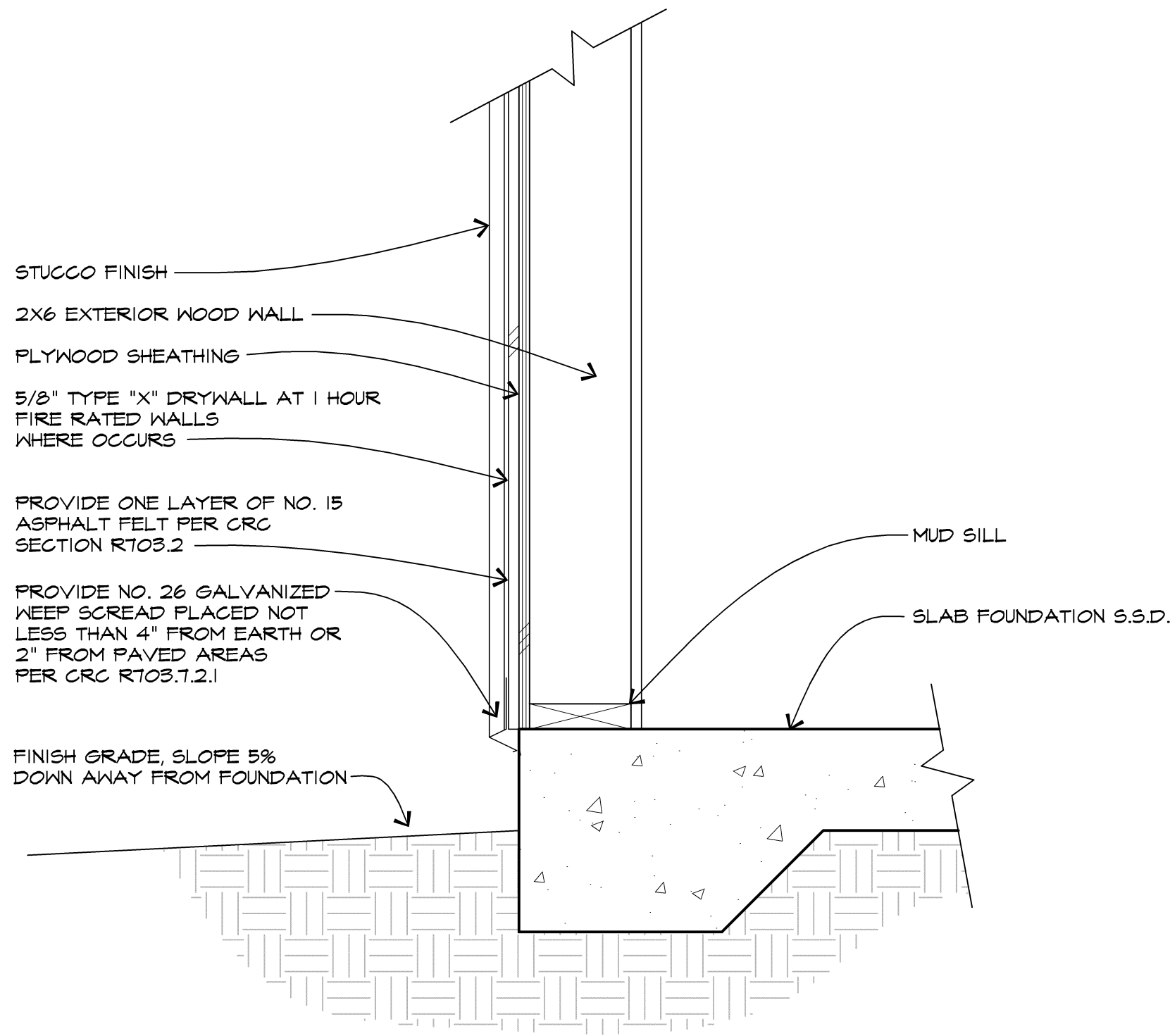
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DRAWN JJ
CHECKED WW
DATE 03-30-23
SCALE AS SHOWN
JOB NO. 20.921R
SHEET A3
OF SHEETS

Apr 26, 2025 - 5:55pm A3 One Bedroom Contemporary Style.dwg

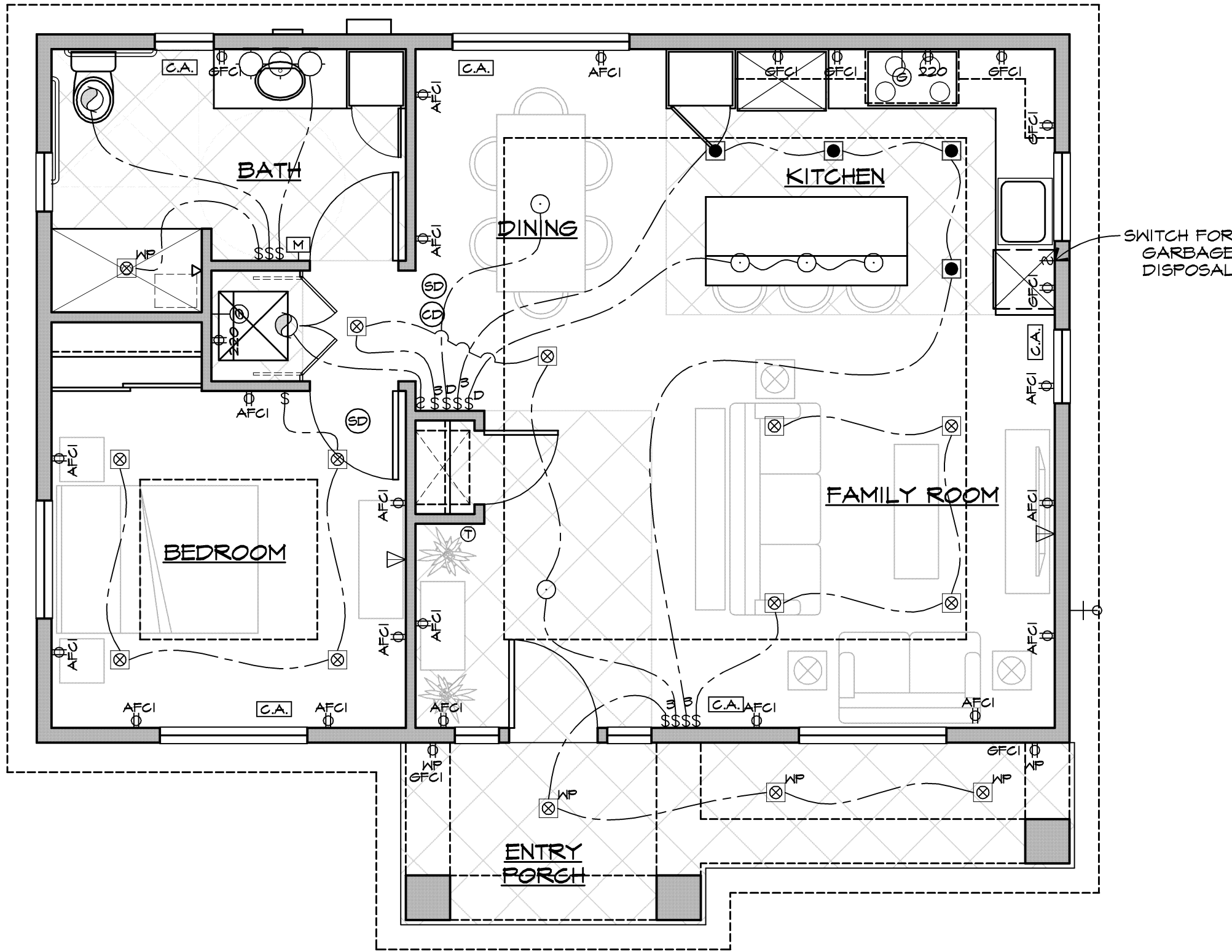


TYPICAL WALL DETAIL

SCALE: 1 1/2" = 1'-0"

- ELECTRICAL/MECHANICAL KEY**
- SWITCHED RECEPTACLE OUTLET @ 12" U.N.O.
 - DUPLEX RECEPTACLE OUTLET @ 12" U.N.O. W/ ARC-FAULT CIRCUIT INTERRUPTER
 - DUPLEX RECEPTACLE OUTLET W/ GROUND FAULT CIRCUIT INTERRUPTER
 - WEATHER PROOF DUPLEX RECEPTACLE OUTLET (GFCI PROTECTED PER NEC210-8(a)(3))
 - DUPLEX RECEPTACLE OUTLET - 220 VOLT
 - SINGLE POLE SWITCH @ 48" U.N.O.
 - THREE-WAY SWITCH
 - SWITCH W/ DIMMER
 - SWITCH W/ MOTION LIGHT
 - MOTION SENSOR
 - CEILING MOUNTED LIGHT FIXTURE
 - WALL MOUNTED LIGHT FIXTURE
 - WALL MOUNTED LIGHT FIXTURE - SHALL BE SUITABLE FOR DAMP LOCATIONS
 - HANGING LIGHT
 - RECESSED LED CEILING LIGHT FIXTURE
 - RECESSED LED LOW VOLTAGE CEILING LIGHT
 - RECESSED LIGHT FIXTURE - SHALL BE LABELED SUITABLE FOR DAMP LOCATIONS
 - EXHAUST FAN
 - SMOKE DETECTOR - (110V W/ BATTERY BACK-UP) HARD WIRED & INTERCONNECTED
 - CARBON MONOXIDE DETECTOR - (110V W/ BATTERY BACK-UP) HARD WIRED & INTERCONNECTED
 - TELEVISION OUTLET
 - HOSE BIBB (1/2" CW) W/ NON- REMOVABLE ANTI-SIPHON DEVICE (FOR OUTSIDE BIBB)
 - GAS OUTLET
 - THERMOSTAT
 - CEILING SUPPLY AIR REGISTER
 - FLOOR AIR SUPPLY REGISTER
 - COLD AIR RETURN @ CEILING

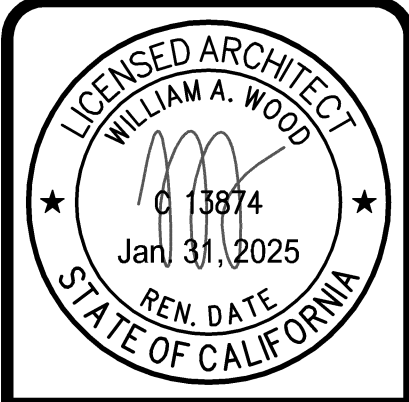
- ELECTRICAL NOTES**
- THERMOSTAT LOCATION TO BE DETERMINED BY HVAC DESIGNER.
 - CEILING AIR REGISTERS TO BE DETERMINED BY HVAC DESIGNER.
 - KITCHEN & BATHROOM LIGHTING SHALL MEET EFFICIENCY REQUIREMENT OF 2022 TITLE 24.
 - CONTRACTOR TO PROVIDE A COLD WATER CONNECTION @ THE REFRIGERATOR.
 - CONTRACTOR TO PROVIDE THE OPTION OF GAS OR 220V @ THE COOK TOP (VERIFY W/ OWNER).
 - WATER CLOSETS TO BE MAX 1.28 GALLONS PER FLUSH PER CAL GREEN SECTION 4.303.1.1.
 - INSTALLATION INSTRUCTIONS FOR ALL EQUIP. SHALL BE PROVIDED TO THE FIELD INSPECTOR @ THE TIME OF INSPECTION.
 - PROVIDE TWO SMALL APPLIANCE BRANCH CIRCUITS FOR THE KITCHEN, AND ARE LIMITED TO SUPPLYING WALL AND COUNTER SPACE OUTLETS FOR THE KITCHEN, PANTRY, BREAKFAST ROOM, DINING ROOM, OR SIMILAR AREAS. NOTE: THESE CIRCUITS CANNOT SERVE OUTSIDE PLUGS, RANGE HOOD, DISPOSALS, DISHWASHERS OR MICROWAVES-ONLY THE REQUIRED COUNTERTOP/WALL OUTLETS INCLUDING THE REFRIGERATOR.
 - PROVIDE A DEDICATED 20-AMP BRANCH CIRCUIT TO SUPPLY THE LAUNDRY RECEPTACLE OUTLET.
 - PROVIDE AN ARC-FAULT CIRCUIT INTERRUPTER TO PROTECT ALL RECEPTACLES IN THE BEDROOMS.
 - PROVIDE RECEPTACLE OUTLETS IN FOLLOWING LOCATIONS:
 - A. AT EACH KITCHEN AND DINING AREA COUNTER SPACE WIDER THAN 12" LOCATED SO THAT NO POINT ALONG THE COUNTER WALL IS OVER 24" FROM A RECEPTACLE.
 - B. AN EXTERIOR RECEPTACLES AT THE FRONT AND REAR OF THE HOME. THEY MUST BE WITHIN 6 FEET, 6 INCHES OF GRADE AND WATERPROOF.
 - C. ONE RECEPTACLE FOR THE LAUNDRY, (I.E. WASHER).
 - D. IN ANY HALLWAY 10 FT OR MORE IN LENGTH.
 - KITCHEN COUNTERTOP RECEPTACLES SHALL BE LOCATED ABOVE THE COUNTERTOP NOT MORE THAN 18", AND MAY NOT BE INSTALLED FACE-UP IN THE COUNTER.
 - ALL LIGHT FIXTURE IN TUB OR SHOWER ENCLOSURES SHALL BE LABEL "SUITABLE FOR DAMP LOCATIONS" AND IF CEILING HEIGHT IS > 8 FT. BE PROTECTED BY GFCI.
 - AN APPROVED CARBON MONOXIDE ALARM SHALL BE INSTALLED IN DWELLING UNITS AND IN SLEEPING UNITS WITHIN WHICH FUEL-BURNING APPLIANCES ARE INSTALLED. CARBON MONOXIDE ALARMS REQUIRED BY SECTIONS R315.1 AND R315.2 SHALL BE INSTALLED IN THE FOLLOWING LOCATIONS: OUTSIDE OF EACH SEPARATE DWELLING UNIT SLEEPING ARE IN THE IMMEDIATE VICINITY OF THE BEDROOM(S), ON EVERY LEVEL OF A DWELLING UNIT INCLUDING BASEMENTS. CARBON MONOXIDE ALARMS COMBINED WITH SMOKE ALARMS SHALL COMPLY WITH SECTION R315, ALL APPLICABLE STANDARDS, AND REQUIREMENTS FOR LISTING AND APPROVAL BY THE OFFICE OF THE STATE FIRE MARSHALL, FOR SMOKE ALARMS.
 - EXHAUST FANS TO BE 50 CFM INTERMITTENT OR 20 CFM CONTINUOUS. IN WATER CLOSETS, 50 CFM CONTINUOUS.
 - ALL RECEPTACLES IN THE BATHROOMS, UTILITY ROOMS, KITCHEN COUNTERS, KITCHEN ISLAND, GARAGE (INCLUDING CEILING MOUNTED) AND ALL EXTERIOR LOCATIONS ARE TO BE GFCI PROTECTED CEC 210.8(g).
 - CONDUIT IN CONCRETE SLAB IS TO BE RATED FOR WET LOCATIONS. CEC 430.
 - PROVIDE DEDICATED CIRCUITS FOR FOR BATHROOM CIRCUITS SERVING ONLY THE RECEPTACLES PER CEC 201.11(g)(3).
 - PROVIDE TAMPER RESISTANT RECEPTACLES IN ALL NEW CIRCUITS PER CEC SECTION 406.12. & 406.4(D)(5).
 - OUTDOOR LIGHTS TO BE "HIGH EFFICACY" OR "LOW EFFICACY" AND ON PHOTO/MOTION SENSOR PER CA ENERGY SECTION 150.0(k)(4).
 - HOSE BIBS SHALL HAVE NON-REMOVABLE BACKFLOW PREVENTER OR BIB-TYPE VACUUM BREAKER.
 - WATER CLOSETS TO BE 1.28 GALLONS PER FLUSH MAXIMUM OR DUAL FLUSH PER CPC 411.2.
 - KITCHEN FAUCET TO BE 1.8 GALLONS PER MINUTE, MAXIMUM PER CPC 4.303.1.4.4.
 - LAVATORY FAUCET TO BE 1.2 GALLONS PER MINUTE, MAXIMUM, PER CPC 407.2.2.
 - SHOWER HEADS TO BE 2.0 GALLONS PER MINUTE, MAXIMUM, PER CPC 408.2.



ELECTRICAL/ MECHANICAL FLOOR PLAN

SCALE: 1/4" = 1'-0"

REVISIONS	DATE



DANVILLE ACCESSORY
DWELLING UNITS - ONE BEDROOM

DANVILLE CA 94526

WILLIAM WOOD
ARCHITECTS
301 HARTZ AVENUE, SUITE 203
DANVILLE, CALIFORNIA 94526
(925) 820-8233

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Apr 26, 2025 - 10:54am A4 One Bedroom Contemporary Style.dwg

DRAWN	JJ
CHECKED	WW
DATE	03-30-23
SCALE	AS SHOWN
JOB NO.	20.921R
SHEET	
A4	
OF	SHEETS

BUILDING ENERGY ANALYSIS REPORT

PROJECT:

Town of Danville ADU
Danville, CA 94526

Project Designer:

William Wood Architects
301 Hartz Ave #203
Danville, CA 94526
(925) 820-8233

Report Prepared by:

Jason Bennett
DP Advanced Engineering Inc.
3381 Walnut Blvd. Suite 220
Brentwood, CA 94513
(925) 516-3502

Job Number:

W021420

Date:

March 31, 2023

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Contemporary 1Bedroom

Calculation Date/Time: 2023-03-30T18:50:48-07:00

(Page 1 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Contemporary_1Bedroom.ribd22

GENERAL INFORMATION					
01	Project Name	Danville ADU Contemporary 1Bedroom			
02	Run Title	Title 24 Analysis			
03	Project Location	Varies			
04	City	Danville, CA	05	Standards Version	2022
06	Zip code	94526	07	Software Version	CBECC-Res 2022.2.1
08	Climate Zone	12	09	Front Orientation (deg/ Cardinal)	All orientations
10	Building Type	Single family	11	Number of Dwelling Units	1
12	Project Scope	Newly Constructed	13	Number of Bedrooms	1
14	Addition Cond. Floor Area (ft ²)	0	15	Number of Stories	1
16	Existing Cond. Floor Area (ft ²)	n/a	17	Fenestration Average U-factor	0.3
18	Total Cond. Floor Area (ft ²)	840	19	Glazing Percentage (%)	18.00%
20	ADU Bedroom Count	n/a			

COMPLIANCE RESULTS	
01	Building Complies with Computer Performance
02	This building incorporates features that require field testing and/or verification by a certified HERS rater under the supervision of a CEC-approved HERS provider.
03	This building incorporates one or more Special Features shown below

Registration Number:

223-P010038747A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:51:36

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Contemporary 1Bedroom

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ENERGY DESIGN RATINGS						
	Energy Design Ratings			Compliance Margins		
	Source Energy (EDR1)	Efficiency ¹ EDR (EDR2efficiency)	Total ² EDR (EDR2total)	Source Energy (EDR1)	Efficiency ¹ EDR (EDR2efficiency)	Total ² EDR (EDR2total)
Standard Design	40.9	44.3	35.3			
Proposed Design						
North Facing	31.4	39.2	31.9	9.5	5.1	3.4
East Facing	31.5	39.6	32.2	9.4	4.7	3.1
South Facing	31.1	38.1	31.4	9.8	6.2	3.9
West Facing	31.7	41.4	33.3	9.2	2.9	2
RESULT ³ : PASS						
¹ Efficiency EDR includes improvements like a better building envelope and more efficient equipment						
² Total EDR includes efficiency and demand response measures such as photovoltaic (PV) system and batteries						
³ Building complies when source energy, efficiency and total compliance margins are greater than or equal to zero and unmet load hour limits are not exceeded						
• Standard Design PV Capacity: 1.87 kWdc						

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ENERGY USE SUMMARY						
Energy Use	Standard Design Source Energy (EDR1) (kBtu/ft ² -yr)	Standard Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Proposed Design Source Energy (EDR1) (kBtu/ft ² -yr)	Proposed Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Compliance Margin (EDR1)	Compliance Margin (EDR2)
Space Heating	7.56	33.23	3.22	24.68	4.34	8.55
Space Cooling	0.81	26.24	0.76	28.2	0.05	-1.96
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
North Facing Efficiency Compliance Total	11.93	96.94	7.07	85.64	4.86	11.3
Space Heating	7.56	33.23	3.19	24.17	4.37	9.06
Space Cooling	0.81	26.24	0.85	29.74	-0.04	-3.5
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
East Facing Efficiency Compliance Total	11.93	96.94	7.13	86.67	4.8	10.27

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Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Contemporary_1Bedroom.ribd22

ENERGY USE SUMMARY						
Energy Use	Standard Design Source Energy (EDR1) (kBtu/ft ² -yr)	Standard Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Proposed Design Source Energy (EDR1) (kBtu/ft ² -yr)	Proposed Design TDV Energy (EDR2) (kTDV/ft ² -yr)	Compliance Margin (EDR1)	Compliance Margin (EDR2)
Space Heating	7.56	33.23	3.08	23.23	4.48	10
Space Cooling	0.81	26.24	0.75	27.42	0.06	-1.18
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
South Facing Efficiency Compliance Total	11.93	96.94	6.92	83.41	5.01	13.53
Space Heating	7.56	33.23	3.13	23.93	4.43	9.3
Space Cooling	0.81	26.24	0.97	33.75	-0.16	-7.51
IAQ Ventilation	0.76	8.12	1.05	11.27	-0.29	-3.15
Water Heating	2.8	29.35	2.04	21.49	0.76	7.86
Self Utilization/Flexibility Credit				0		0
West Facing Efficiency Compliance Total	11.93	96.94	7.19	90.44	4.74	6.5

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CF1R-PRF-01E
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ENERGY USE INTENSITY				
	Standard Design (kBtu/ft ² - yr)	Proposed Design (kBtu/ft ² - yr)	Compliance Margin (kBtu/ft ² - yr)	Margin Percentage
North Facing				
Gross EUI¹	28.05	21.81	6.24	22.25
Net EUI²	16.19	9.78	6.41	39.59
East Facing				
Gross EUI¹	28.05	21.92	6.13	21.85
Net EUI²	16.19	9.89	6.3	38.91
South Facing				
Gross EUI¹	28.05	21.61	6.44	22.96
Net EUI²	16.19	9.58	6.61	40.83
West Facing				
Gross EUI¹	28.05	22	6.05	21.57
Net EUI²	16.19	9.97	6.22	38.42
Notes 1. Gross EUI is Energy Use Total (not including PV) / Total Building Area. 2. Net EUI is Energy Use Total (including PV) / Total Building Area.				

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD**CF1R-PRF-01E****Project Name:** Danville ADU Contemporary 1Bedroom**Calculation Date/Time:** 2023-03-30T18:50:48-07:00**(Page 6 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Contemporary_1Bedroom.ribd22

REQUIRED PV SYSTEMS											
01	02	03	04	05	06	07	08	09	10	11	12
DC System Size (kWdc)	Exception	Module Type	Array Type	Power Electronics	CFI	Azimuth (deg)	Tilt Input	Array Angle (deg)	Tilt: (x in 12)	Inverter Eff. (%)	Annual Solar Access (%)
2	NA	Standard (14-17%)	Fixed	none	true	105-300	n/a	n/a	<=7:12	96	100

REQUIRED SPECIAL FEATURES											
The following are features that must be installed as condition for meeting the modeled energy performance for this computer analysis.											
- PV System: 2 kWdc - Indoor air quality, balanced fan - IAQ Ventilation System Heat Recovery: minimum 67 SRE and 73 ASRE - IAQ Ventilation System: supply outside air inlet, filter, and H/ERV cores accessible per RACM Reference Manual - IAQ Ventilation System: fault indicator display - Insulation below roof deck - Window overhangs and/or fins - Northwest Energy Efficiency Alliance (NEEA) rated heat pump water heater; specific brand/model, or equivalent, must be installed											

HERS FEATURE SUMMARY											
The following is a summary of the features that must be field-verified by a certified HERS Rater as a condition for meeting the modeled energy performance for this computer analysis. Additional detail is provided in the building tables below. Registered CF2Rs and CF3Rs are required to be completed in the HERS Registry											
- Indoor air quality ventilation - Kitchen range hood - Minimum Airflow - Verified SEER/SEER2 - Verified Refrigerant Charge - Fan Efficacy Watts/CFM - Verified HSPF - Verified heat pump rated heating capacity - Duct leakage testing											

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BUILDING - FEATURES INFORMATION						
01	02	03	04	05	06	07
Project Name	Conditioned Floor Area (ft ²)	Number of Dwelling Units	Number of Bedrooms	Number of Zones	Number of Ventilation Cooling Systems	Number of Water Heating Systems
Danville ADU Contemporary 1Bedroom	840	1	1	1	0	1

ZONE INFORMATION						
01	02	03	04	05	06	07
Zone Name	Zone Type	HVAC System Name	Zone Floor Area (ft ²)	Avg. Ceiling Height	Water Heating System 1	Status
Accessory Unit	Conditioned	HVAC System 1	840	10	DHW System 1	New

OPAQUE SURFACES							
01	02	03	04	05	06	07	08
Name	Zone	Construction	Azimuth	Orientation	Gross Area (ft ²)	Window and Door Area (ft2)	Tilt (deg)
Front Wall	Accessory Unit	Exterior 2x6 Wall R-21	0	Front	350	98	90
Left Wall	Accessory Unit	Exterior 2x6 Wall R-21	90	Left	240	15	90
Back Wall	Accessory Unit	Exterior 2x6 Wall R-21	180	Back	350	22	90
Right Wall	Accessory Unit	Exterior 2x6 Wall R-21	270	Right	240	16	90
Ceiling Below Attic	Accessory Unit	R-38 Attic Ceiling	n/a	n/a	840	n/a	n/a

ATTIC							
01	02	03	04	05	06	07	08
Name	Construction	Type	Roof Rise (x in 12)	Roof Reflectance	Roof Emittance	Radiant Barrier	Cool Roof
Attic	Attic Roof	Ventilated	4	0.1	0.85	No	No

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FENESTRATION / GLAZING													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Name	Type	Surface	Orientation	Azimuth	Width (ft)	Height (ft)	Mult.	Area (ft ²)	U-factor	U-factor Source	SHGC	SHGC Source	Exterior Shading
FamilyRm 5050	Window	Front Wall	Front	0	5	5	1	25	0.3	NFRC	0.23	NFRC	Bug Screen
Entry 1680 Sidelets	Window	Front Wall	Front	0	1.5	8	2	24	0.3	NFRC	0.23	NFRC	Bug Screen
Front Door	Window	Front Wall	Front	0	3	8	1	24	0.32	NFRC	0.16	NFRC	Bug Screen
Bedroom 5050	Window	Front Wall	Front	0	5	5	1	25	0.3	NFRC	0.23	NFRC	Bug Screen
Kitchen 2630	Window	Left Wall	Left	90	2.5	3	1	7.5	0.3	NFRC	0.23	NFRC	Bug Screen
FamilyRm 2630	Window	Left Wall	Left	90	2.5	3	1	7.5	0.3	NFRC	0.23	NFRC	Bug Screen
Bathroom Back 2020	Window	Back Wall	Back	180	2	2	1	4	0.3	NFRC	0.23	NFRC	Bug Screen
DiningRm 6030	Window	Back Wall	Back	180	6	3	1	18	0.3	NFRC	0.23	NFRC	Bug Screen
Bedroom 4030	Window	Right Wall	Right	270	4	3	1	12	0.3	NFRC	0.23	NFRC	Bug Screen
Bathroom Right 2020	Window	Right Wall	Right	270	2	2	1	4	0.3	NFRC	0.23	NFRC	Bug Screen

OVERHANGS AND FINIS													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Window	Overhang					Left Fin				Right Fin			
	Depth	Dist Up	Left Extent	Right Extent	Flap Ht.	Depth	Top Up	Dist L	Bot Up	Depth	Top Up	Dist R	Bot Up
FamilyRm 5050	5	0	2	2	0	0	0	0	0	0	0	0	0
Entry 1680 Sidelets	7	0	2	2	0	0	0	0	0	0	0	0	0

Registration Number:

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OVERHANGS AND FINs													
01	02	03	04	05	06	07	08	09	10	11	12	13	14
Window	Overhang					Left Fin				Right Fin			
	Depth	Dist Up	Left Extent	Right Extent	Flap Ht.	Depth	Top Up	Dist L	Bot Up	Depth	Top Up	Dist R	Bot Up
Front Door	7	0	2	2	0	0	0	0	0	0	0	0	0
Bedroom 5050	1	0	2	2	0	0	0	0	0	0	0	0	0
Kitchen 2630	1	0	2	2	0	0	0	0	0	0	0	0	0
FamilyRm 2630	1	0	2	2	0	0	0	0	0	0	0	0	0
Bathroom Back 2020	1	0	2	2	0	0	0	0	0	0	0	0	0
DiningRm 6030	1	0	2	2	0	0	0	0	0	0	0	0	0
Bedroom 4030	1	0	2	2	0	0	0	0	0	0	0	0	0
Bathroom Right 2020	1	0	2	2	0	0	0	0	0	0	0	0	0

SLAB FLOORS							
01	02	03	04	05	06	07	08
Name	Zone	Area (ft ²)	Perimeter (ft)	Edge Insul. R-value and Depth	Edge Insul. R-value and Depth	Carpeted Fraction	Heated
Slab On Grade	Accessory Unit	840	118	none	0	80%	No

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OPAQUE SURFACE CONSTRUCTIONS							
01	02	03	04	05	06	07	08
Construction Name	Surface Type	Construction Type	Framing	Total Cavity R-value	Interior / Exterior Continuous R-value	U-factor	Assembly Layers
Exterior 2x6 Wall R-21	Exterior Walls	Wood Framed Wall	2x6 @ 16 in. O. C.	R-21	None / None	0.065	Inside Finish: Gypsum Board Cavity / Frame: R-21 / 2x6 Sheathing / Insulation: Wood Siding/sheathing/decking Exterior Finish: 3 Coat Stucco
Attic Roof	Attic Roofs	Wood Framed Ceiling	2x4 Top Chord of Roof Truss @ 24 in. O. C.	R-13	None / None	0.078	Roofing: Light Roof (Asphalt Shingle) Roof Deck: Wood Siding/sheathing/decking Cavity / Frame: R-13.0 / 2x4 Top Chrd Around Roof Joists: R-0.0 insul.
R-38 Attic Ceiling	Ceilings (below attic)	Wood Framed Ceiling	2x4 Bottom Chord of Truss @ 24 in. O. C.	R-38	None / None	0.025	Over Ceiling Joists: R-28.9 insul. Cavity / Frame: R-9.1 / 2x4 Btm Chrd Inside Finish: Gypsum Board

BUILDING ENVELOPE - HERS VERIFICATION				
01	02	03	04	05
Quality Insulation Installation (QII)	High R-value Spray Foam Insulation	Building Envelope Air Leakage	CFM50	CFM50
Not Required	Not Required	N/A	n/a	n/a

WATER HEATING SYSTEMS								
01	02	03	04	05	06	07	08	09
Name	System Type	Distribution Type	Water Heater Name	Number of Units	Solar Heating System	Compact Distribution	HERS Verification	Water Heater Name (#)
DHW System 1	Domestic Hot Water (DHW)	Standard	Heat Pump WH	1	n/a	None	n/a	Heat Pump WH (1)

Registration Number:

223-P010038747A-000-000-00000000-0000

Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:51:36

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Contemporary 1Bedroom

Calculation Date/Time: 2023-03-30T18:50:48-07:00

(Page 11 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Contemporary_1Bedroom.ribd22

WATER HEATERS - NEEA HEAT PUMP							
01	02	03	04	05	06	07	08
Name	# of Units	Tank Vol. (gal)	NEEA Heat Pump Brand	NEEA Heat Pump Model	Tank Location	Duct Inlet Air Source	Duct Outlet Air Source
Heat Pump WH	1	40	Rheem	RheemPROPH40T2R H37515	Outside	Outside	Outside

WATER HEATING - HERS VERIFICATION						
01	02	03	04	05	06	07
Name	Pipe Insulation	Parallel Piping	Compact Distribution	Compact Distribution Type	Recirculation Control	Shower Drain Water Heat Recovery
DHW System 1 - 1/1	Not Required	Not Required	Not Required	None	Not Required	Not Required

SPACE CONDITIONING SYSTEMS								
01	02	03	04	05	06	07	08	09
Name	System Type	Heating Unit Name	Heating Equipment Count	Cooling Unit Name	Cooling Equipment Count	Fan Name	Distribution Name	Required Thermostat Type
HVAC System 1	Heat pump heating cooling	PEAD-A24AA7	1	PEAD-A24AA7	1	HVAC Fan System 1	Distribution System 1	Setback

HVAC - HEAT PUMPS												
01	02	03	04	05	06	07	08	09	10	11	12	13
Name	System Type	Number of Units	Heating				Cooling			Zonally Controlled	Compressor Type	HERS Verification
			Efficiency Type	HSPF / HSPF2 / COP	Cap 47	Cap 17	Efficiency Type	SEER / SEER2	EER / EER / CEER			
PEAD-A24AA7	Central split HP	1	HSPF	10.8	26000	14800	EERSEER	19.6	11.7	Not Zonal	Single Speed	PEAD-A24AA7-hers-htpump

Registration Number:

223-P010038747A-000-000-00000000-0000

Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:51:36

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD
CF1R-PRF-01E
Project Name: Danville ADU Contemporary 1Bedroom

Calculation Date/Time: 2023-03-30T18:50:48-07:00

(Page 12 of 14)
Calculation Description: Title 24 Analysis

Input File Name: Danville ADU_Contemporary_1Bedroom.ribd22

HVAC HEAT PUMPS - HERS VERIFICATION								
01	02	03	04	05	06	07	08	09
Name	Verified Airflow	Airflow Target	Verified EER/EER2	Verified SEER/SEER2	Verified Refrigerant Charge	Verified HSPF/HSPF2	Verified Heating Cap 47	Verified Heating Cap 17
PEAD-A24AA7-hers-htpump	Required	350	Not Required	Required	Yes	Yes	Yes	Yes

HVAC - DISTRIBUTION SYSTEMS											
01	02	03	04	05	06	07	08	09	10	11	12
Name	Type	Design Type	Duct Ins. R-value		Duct Location		Surface Area		Bypass Duct	Duct Leakage	HERS Verification
			Supply	Return	Supply	Return	Supply	Return			
Distribution System 1	Unconditioned attic	Non-Verified	R-6	R-6	Attic	Attic	n/a	n/a	No Bypass Duct	Sealed and Tested	Distribution System 1-hers-dist

HVAC DISTRIBUTION - HERS VERIFICATION								
01	02	03	04	05	06	07	08	09
Name	Duct Leakage Verification	Duct Leakage Target (%)	Verified Duct Location	Verified Duct Design	Buried Ducts	Deeply Buried Ducts	Low-leakage Air Handler	Low Leakage Ducts Entirely in Conditioned Space
Distribution System 1-hers-dist	Yes	5.0	Not Required	Not Required	Not Required	Credit not taken	Not Required	No

HVAC - FAN SYSTEMS			
01	02	03	04
Name	Type	Fan Power (Watts/CFM)	Name
HVAC Fan System 1	HVAC Fan	0.45	HVAC Fan System 1-hers-fan

Registration Number:

223-P010038747A-000-000-00000000-0000

Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

CalCERTS inc.

CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:51:36

CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD**CF1R-PRF-01E****Project Name:** Danville ADU Contemporary 1Bedroom**Calculation Date/Time:** 2023-03-30T18:50:48-07:00**(Page 13 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Contemporary_1Bedroom.ribd22

HVAC FAN SYSTEMS - HERS VERIFICATION		
01	02	03
Name	Verified Fan Watt Draw	Required Fan Efficacy (Watts/CFM)
HVAC Fan System 1-hers-fan	Required	0.45

INDOOR AIR QUALITY (IAQ) FANS								
01	02	03	04	05	06	07	08	09
Dwelling Unit	Airflow (CFM)	Fan Efficacy (W/CFM)	IAQ Fan Type	Includes Heat/Energy Recovery?	IAQ Recovery Effectiveness - SRE	Includes Fault Indicator Display?	HERS Verification	Status
SFam IAQVentRpt 1-1	49	0.816326	Balanced	Yes	73	No	Yes	



Registration Number:

223-P010038747A-000-000-0000000-0000

Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

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Report Version: 2022.0.000

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CERTIFICATE OF COMPLIANCE - RESIDENTIAL PERFORMANCE COMPLIANCE METHOD

CF1R-PRF-01E

Project Name: Danville ADU Contemporary 1Bedroom**Calculation Date/Time:** 2023-03-30T18:50:48-07:00**(Page 14 of 14)****Calculation Description:** Title 24 Analysis**Input File Name:** Danville ADU_Contemporary_1Bedroom.ribd22

DOCUMENTATION AUTHOR'S DECLARATION STATEMENT	
1. I certify that this Certificate of Compliance documentation is accurate and complete.	
Documentation Author Name: Jason E Bennett	Documentation Author Signature: 
Company: DP Advanced Engineering	Signature Date: 2023-03-30 19:17:10
Address: 3381 Walnut Blvd. Ste. 220	CEA/ HERS Certification Identification (If applicable): R19-21-30042
City/State/Zip: Brentwood, CA 94513	Phone: 925-516-3502
	
RESPONSIBLE PERSON'S DECLARATION STATEMENT	
I certify the following under penalty of perjury, under the laws of the State of California: 1. I am eligible under Division 3 of the Business and Professions Code to accept responsibility for the building design identified on this Certificate of Compliance. 2. I certify that the energy features and performance specifications identified on this Certificate of Compliance conform to the requirements of Title 24, Part 1 and Part 6 of the California Code of Regulations. 3. The building design features or system design features identified on this Certificate of Compliance are consistent with the information provided on other applicable compliance documents, worksheets, calculations, plans and specifications submitted to the enforcement agency for approval with this building permit application.	
Responsible Designer Name: William Wood	Responsible Designer Signature: 
Company: William Wood Architects	Date Signed: 2023-03-31 08:33:08
Address: 301 Hartz Ave #203	License: C13874
City/State/Zip: Danville, CA 94526	Phone: 925-820-8233

Digitally signed by CalCERTS. This digital signature is provided in order to secure the content of this registered document, and in no way implies

Registration Provider responsibility for the accuracy of the information.



Registration Number:

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Registration Date/Time:

2023-03-31 08:33:08

HERS Provider:

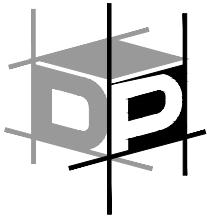
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CA Building Energy Efficiency Standards - 2022 Residential Compliance

Report Version: 2022.0.000

Schema Version: rev 20220901

Report Generated: 2023-03-30 18:51:36



ENVELOPE INSULATION

EXTERIOR WALLS:

R-21 IN 2x6

FOUNDATION:

SLAB, NOT INSULATED

ROOF OVER LIVING SPACE:

R-38 BETWEEN CEILING JOISTS

R-13 BETWEEN RAFTERS

WINDOW PERFORMANCE

MAXIMUM WINDOW PERFORMANCE VALUES (U-FACTOR / SHGC)

WINDOWS: .30/.23

FRENCH DOORS: .32/.16

DOMESTIC HOT WATER

RHEEM 40-GALLON HEAT PUMP WATER HEATER OR EQUIV.

SOLAR (STUDIO)

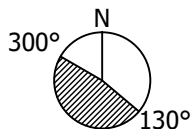
(NOT REQUIRED; CALCULATED SIZE IS <1.8 KW)

SOLAR (1 BEDROOM)

2.0 KW MINIMUM

ACCEPTABLE ORIENTATION:

(DEGREES EAST OF TRUE NORTH)



HERS VERIFICATIONS

- INDOOR AIR QUALITY VENTILATION
- KITCHEN RANGE HOOD
- MINIMUM AIRFLOW [350 CFM/TON]
- VERIFIED SEER [19.6]
- VERIFIED REFRIGERANT CHARGE
- FAN EFFICACY WATTS/CFM [0.45]
- VERIFIED HSPF [10.8]
- VERIFIED HEAT PUMP RATED HEATING CAPACITY
- DUCT LEAKAGE TESTING [5%]

SPACE HEATING & COOLING

HEAT PUMP (10.2 HSPF, 19.6 SEER, 11.7 EER)

MITSUBISHI PEAD-A24AA7 + PUZ-A24NHA7

AHRI # 201754661

DUCTS

SUPPLY AIR DUCTS IN ATTIC

DUCTS IN UNCONDITIONED SPACE TO HAVE R-6 INSULATION

INDOOR AIR QUALITY

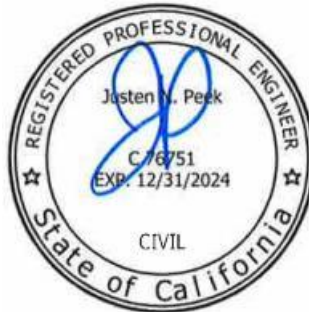
BROAN ERV100S ENERGY RECOVERY VENTILATOR OR EQUIV.

Structural Calculations

PROJECT: Town of Danville 1-Bedroom ADU
ADDRESS: Danville, California

JOB No.: W021420
DELTA: Permit Submittal
DATE: March 24, 2023

CLIENT: Town of Danville



GOVERNING CODE: California Building Code, 2022 Edition
CONSTRUCTION: One-Story Wood Framed Building

STRUCTURE:
Vertical Load System - Wood Framed Roof
Lateral Load System - Wood Framed Shear Walls



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

STRUCTURAL CALCULATIONS ARE BASED ON THE FOLLOWING CRITERIA, UNLESS NOTED OTHERWISE

STRUCTURAL MATERIALS:

Structural Steel:

W Shapes	ASTM A992, Fy = 50 ksi
Shapes (M, S, HP, C, MC, L)	ASTM A572, Grade 50
Pipe	ASTM A53, Grade B
Tube (HSS)	ASTM A500, Grade B
Plates and Bars	ASTM A572, Grade 50
Welding	Current AWS D1.1
Bolts - Unfinished	ASTM A307
Bolts - High Strength (HSB)	ASTM A325 SC/N/X
Threaded Rods	ASTM A449
Light Gage Steel Studs and Joists	ASTM A653, Grade 50
Welding (Light Gage)	Current AWS D1.3

Concrete:

Slab on Grade	2500 psi @ 28 Days
Foundation	2500 psi @ 28 Days
Tilt-Up Wall Panels	4000 psi @ 28 Days
Structural Concrete	3000 psi @ 28 Days
Reinforcing Steel (#3 Rebar)	ASTM A615, Grade 40
Reinforcing Steel (#4 & larger)	ASTM A615, Grade 60
Welding (Reinf.).....	Current AWS D1.4

Wood:

2" to 4" Thick x 2" and Wider	DF No. 2 or better
Joists and Planks	DF No. 2 or better
Beams and Stringers	DF No. 1 or better
Posts and Timbers	DF No. 1 or better
Glu-lam Beams (simple span)	2400F-V4 DF/DF
Glu-lam Beams (cantilevers)	2400F-V8 DF/DF
Sheathing	Exposure 1, Grade C-D, C-C



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

STRUCTURAL CALCULATIONS ARE BASED ON THE FOLLOWING CRITERIA, UNLESS NOTED OTHERWISE

LOADING SCHEDULE:

Roof Dead Load:

Max. Roof Pitch /12:

5

Roofing:	Concrete Tile	10.8 psf
Sheathing:	1/2" Plywood	1.8 psf
Insulation:	R38 Insulation Ceiling & R19 Insulation Roof Deck	1.0 psf
Framing:	Trusses @ 24" o.c.	2.5 psf
Ceiling:	5/8" Gyp. Installed on Bottom of Roof Framing	2.8 psf
Sprinklers:	Automatic Fire Sprinklers	1.0 psf
Misc:	HVAC + Miscellaneous	2.1 psf

22.0 psf

Photovoltaic Dead Load:

Non-concurrent with Live Load

3.0 psf

Roof Live Load:

Sloped Roof (Reducible)

20.0 psf

Exterior Wall Load:

Wall Type: 2x6 @ 16 in, 5/8" Gyp, Insulated, 7/8" Stucco

17.0 psf

Interior Wall Load:

Wall Type: 2x4 @ 16 in, (2) 5/8" Gyp, Insulated

7.0 psf



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SEISMIC LOADS: ASCE 7-16 SECTION 12.8 (EQUIV. LATERAL FORCE):

SITE INFORMATION:

S_s 2.500 ASCE 7-16 Sect. 11.4.2
 S_1 1.200 ASCE 7-16 Sect. 11.4.2
 Site Class..... D Per Geotech Report
 Risk Category II ASCE 7-16 Table 1.5-1
 Importance Factor 1.0 ASCE 7-16 Table 1.5-2

Site coefficients and adjusted maximum considered eq. spectral response accel. parameters

F_a 1.200 ASCE 7-16 Sect. 11.4.4
 F_v 1.700 ASCE 7-16 Sect. 11.4.4

Table 11.4-1 Short-Period Site Coefficient F_a

Site Class	$S_s \leq 0.25$	$S_s = 0.50$	$S_s = 0.75$	$S_s = 1.00$	$S_s = 1.25$	$S_s \geq 1.50$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.9	0.9	0.9	0.9	0.9	0.9
C	1.3	1.3	1.2	1.2	1.2	1.2
D	1.6	1.4	1.2	1.1	1.0	1.0
E	2.4	1.7	1.3	1.2	1.2	1.2
F	Site-Specific Response Analysis Required					

Table 11.4-2 Short-Period Site Coefficient F_v

Site Class	$S_1 \leq 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 = 0.50$	$S_1 \geq 0.60$
A	0.8	0.8	0.8	0.8	0.8	0.8
B	0.8	0.8	0.8	0.8	0.8	0.8
C	1.5	1.5	1.5	1.5	1.5	1.4
D	2.4	2.2	2	1.9	1.8	1.7
E	4.2	3.3	2.8	2.4	2.2	2
F	Site-Specific Response Analysis Required					

$S_{MS} = F_a S_s$ 3.000 (EQ. 11.4-1)
 $S_{M1} = F_v S_1$ 2.040 (EQ. 11.4-2)
 $S_{DS} = (2/3) S_{MS}$ 2.000 g (EQ. 11.4-3)
 $S_{D1} = (2/3) S_{M1}$ 1.360 g (EQ. 11.4-4)

Seismic Design Category Based on Short-Period Response Accelerations:

VALUE OF S_{DS}	1 OR 11	111	1V	
$S_{DS} < 0.167g$	A	A	A	S1 IS GREATER THAN 0.75 USE CATEGORY E PER CBC 1613.2.5
$0.167g \leq S_{DS} < 0.33g$	B	B	C	
$0.33g \leq S_{DS} < 0.50g$	C	C	D	
$0.50g \leq S_{DS}$	D	D	D	

Seismic Design Category Based on 1-Second Response Accelerations:

VALUE OF S_{D1}	1 OR 11	111	1V	
$S_{D1} < 0.067g$	A	A	A	S1 IS GREATER THAN 0.75 USE CATEGORY E PER CBC 1613.2.5
$0.067g \leq S_{D1} < 0.133g$	B	B	C	
$0.133g \leq S_{D1} < 0.20g$	C	C	D	
$0.20g \leq S_{D1}$	D	D	D	



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SEISMIC LOADS: ASCE 7-16 SECTION 12.8 (CONT.):

BUILDING INFORMATION:

Building Height, h_n 15.5 ft
 Mean Roof Height, H_m 12.8 ft
 Eave Height, h 10.0 ft

Building Depth:

Bldg Depth (Roof Level)..... 30.0 ft

Building Width:

Bldg Width (Roof Level)..... 35.0 ft

EQUIVALENT LATERAL FORCE PROCEDURE:

Seismic Base Shear:

R (ASCE 7-16 Table 12.2-1)..... 6.5
 C_t 0.020
 X 0.75
 $T_a = C_t(h_n)^X$ 0.16 sec

Seismic Response Coefficient:

$C_S = S_{DS}/(R/I)$ 0.308 (EQ. 12.8-2)
 $C_S = S_{D1}/T(R/I)$ 1.339 (EQ. 12.8-3)
 $C_S = 0.044S_{DS}I$ 0.088 (EQ. 12.8-5)
 $C_S = 0.5S_1/(R/I)$ 0.092 (EQ. 12.8-6)

Seismic Base Shear:

$V = C_S W$ **0.308 W** (EQ. 12.8-1)

BUILDING WEIGHTS:

SEISMIC BASE SHEAR:

Roof Loads:	AREA (sq ft)	WEIGHT (psf)	TOTAL (lb)
Roof Area	1082	22.0	23831
Roof Top Deck	0	15.0	0
Photovoltaic Area	1082	3.0	3246

Building DL = 40254 lb
 Seismic Coef. = 0.308
 $k = 1.00$
Base Shear = 12386 lb

Roof Loads:	LENGTH (ft)	WEIGHT (psf)	TOTAL (lb)
Linear Feet of Ext. Wall	118	17.0	8597
Linear Feet of Int. Wall	130	7.0	4581

*Note: Approximate Fenestration Percentage: 15 %
TOTAL ROOF LOAD (lb): 40254

CALCULATE SEISMIC SHEAR LOADS:

VERTICAL DISTRIBUTION OF SEISMIC FORCES					
Level	w_x (lb)	Top Plate Height (ft)	h_x (ft)	F_x (lb)	Unit Shear (psf)
ROOF	40254	10.1	12.8	12386	11.4
			$\sum w_i * h_i^k =$	513244	

REDUNDANCY FACTOR (ASCE 7-16 SECTION 12.3.4):

Seismic Design Cat: **USE CATEGORY E**
 Roof East/ West: **1.0**

*Note: Refer to calculations on following sheets
 Roof North/South: **1.3**

SUMMARY OF SEISMIC DESIGN LOADS:

Level	Direction	SEISMIC	UNIT	Redundancy	ASD DESIGN	ASD DESIGN
		FORCE (lb)	SHEAR (psf)	Factor	FORCE (lb)	LOAD (psf)
Roof	North/South	12386	11.4	1.3	11271	10.4
Roof	East/West	12386	11.4	1.0	8670	8.0



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

WIND ANALYSIS (ENCLOSED, PARTIALLY ENCLOSED AND OPEN BUILDINGS OF ALL HEIGHTS)

MAIN WIND-FORCE RESISTING SYSTEM:

Design Wind Speed..... 95 mph
 Wind Exposure..... C
 Site Elevation..... Unknown Feet
 Topographic factor, ASCE 7-10 Section 26.8.2 -
 K_{zt} (Assumed Flat Area)..... 1.00

BUILDING INFORMATION:

Building Height, h_n 15.5 ft
 Mean Roof Height, H_m 12.8 ft
 Eave Height, h 10.0 ft

Building Depth "B":

Bldg Depth (Roof Level)..... 30.0 ft
 Bldg Depth (Floor Level)..... 0.0 ft

Building Width "L":

Bldg Width (Roof Level)..... 35.0 ft
 Bldg Width (Floor Level)..... 0.0 ft

DETERMINE WIND LOAD PARAMETERS:

DIRECTIONALITY FACTOR (K_d): 0.85 ** See ASCE 7-16 Section 26.6 and Table 26.6-1
 TOPOGRAPHIC FACTOR (K_{zt}): 1.00 ** See ASCE 7-16 Section 26.8 and Figure 26.8-1
 GUST FACTOR (G): 0.85 ** See ASCE 7-16 Section 26.11.1
 ENCLOSURE CLASSIFICATION Enclosed ** See ASCE 7-16 Section 26.12
 INTERNAL PRESSURE (C_{pi}) 0.18 ** See ASCE 7-16 Section 26.12 and Table 26.13-1
 GROUND ELEVATION FACTOR (K_e) 1.00 ** See ASCE 7-16 Table 26.9-1, Note 2

Table 26.10-1 Velocity Pressure Coefficients, K_z

Height Above Ground (ft)	Exposure Category		
	B	C	D
15.0	0.57	0.85	1.03
20.0	0.62	0.90	1.08
25.0	0.66	0.94	1.12
30.0	0.70	0.98	1.16
35.0	0.73	1.01	1.19
40.0	0.76	1.04	1.22
45.0	0.79	1.07	1.25
50.0	0.81	1.09	1.27
55.0	0.83	1.11	1.29
60.0	0.85	1.13	1.31

VELOCITY COEFFICIENT (K_z): 0.85
 ** See ASCE 7-16 Table 26.10.1

DETERMINE VELOCITY PRESSURE:

$q_z = 0.00256 K_z K_{zt} K_d K_e V^2$: 16.69 psf



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

WIND ANALYSIS (ENCLOSED, PARTIALLY ENCLOSED AND OPEN BUILDINGS OF ALL HEIGHTS)

DETERMINE WALL PRESSURE:

Wall Pressure Coefficients, ASCE-7 (Figure 27..3-1)

Surface	L/B	Cp	Use With
Windward Wall	All Values	0.8	q _z
Leeward Wall	0-1	-0.5	q _h
	2	-0.3	
	>4	-0.2	
Side Wall	All Values	-0.7	q _h

Determine Cp Values

Level	Direction	L/B Values	Windward C _p Value	Leeward C _p Value
Roof	East/West	1.17	0.80	-0.467

Level	Direction	L/B Values	Windward C _p Value	Leeward C _p Value
Roof	North/South	0.86	0.80	-0.500

Windward Pressures:

Plan North/South

CASE 1: 14.36
CASE 2: 8.35

Plan East/West

CASE 1: 14.36
CASE 2: 8.35

Leeward Pressures:

Plan North/South

CASE 1: 4.09
CASE 2: 10.10

Plan East/West

CASE 1: 3.62
CASE 2: 9.63

SUMMARY OF WIND FORCE DESIGN LOADS

Level	Direction	WIND DESIGN	UNIT		ASD DESIGN	ASD DESIGN
		FORCE (lb)	SHEAR (plf)		FORCE (lb)	LOAD (plf)
Roof	North/South	4976	142		2986	85
Roof	East/West	4156	139		2494	83



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN: ROOF LEVEL

Shearwall Framing: 3/8" CDX ply with 8d Nailing

SHEAR LINE ID:	1	2
-----------------------	----------	----------

DETERMINATION OF LATERAL DESIGN LOADS:

Grid Line Trib. Area (sq ft):	481	601
Seismic Shear Load (psf):	8.0	8.0
Seismic Shear Force (lb):	3854	4816

Grid Line Trib. Width (ft):	12	18
Wind Design Load (plf):	83	83
Wind Design Force (lb):	997	1496

SHEARWALL LENGTHS:

Wall Length 1 (ft):	8.0	5.0
Wall Length 2 (ft):	8.0	5.0
Wall Length 3 (ft):	0.0	4.0
Wall Length 4 (ft):	0.0	0.0
Wall Length 5 (ft):	0.0	0.0

SHEARWALL RIGIDITY DESIGN:

Wall 1 Rigidity K_i (kip/in):	5.42	3.54
Wall 2 Rigidity K_i (kip/in):	5.42	3.54
Wall 3 Rigidity K_i (kip/in):		2.51
Wall 4 Rigidity K_i (kip/in):		
Wall 5 Rigidity K_i (kip/in):		

NAILING DESIGN BASED ON SDPWS SECTION 4.3.4 CAPACITY ADJ:

Wall 1 Design Shear (plf):	241	356
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 2 Design Shear (plf):	241	356
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 3 Design Shear (plf):		336
<i>Sec. 4.3.4.2 Capacity Reduction</i>		0.93
Wall 4 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 5 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		

Shear Capacity (plf):	260	490
-----------------------	-----	-----

Shear Designation #: **Type 1** **Type 3**

REDUNDANCY FACTOR ANALYSIS (ASCE 7-16 SECTION 12.3.4):

Wall 1 Capacity (%):	0.19	0.22
Wall 2 Capacity (%):	0.19	0.22
Wall 3 Capacity (%):		0.18
Wall 4 Capacity (%):		
Wall 5 Capacity (%):		

Redundancy Factor: **Use 1.0** **Use 1.0**



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN (CONT.): ROOF LEVEL

SHEAR LINE ID:	1	2
-----------------------	----------	----------

SHEARWALL OVERTURNING DESIGN:

Wall Height (ft):	10.1	10.1
Wall Weight (psf):	17.0	17.0
Roof Trib. Width (ft):	2	2
Roof Weight (psf):	22.0	22.0

SEISMIC UPLIFT FORCE (lb): Load Combo (ASCE 7-16): (0.6-0.14*SDS)D+0.7E

Wall Dead Load (plf):	69	69
Wall 1 Uplift (lb):	2153	3415
Wall 2 Uplift (lb):	2153	3415
Wall 3 Uplift (lb):		3034
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

WIND UPLIFT FORCE (lb): Load Combo (ASCE 7-16): 0.6D+0.6W

Wall Dead Load (plf):	129	129
Wall 1 Uplift (lb):	111	791
Wall 2 Uplift (lb):	111	791
Wall 3 Uplift (lb):		727
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

HOLDOWN TYPE:

Wall 1 Holdown Type:	HDU2	HDU4
Wall 2 Holdown Type:	HDU2	HDU4
Wall 3 Holdown Type:		HDU4
Wall 4 Holdown Type:		
Wall 5 Holdown Type:		

STORY DRIFT CHECK ($C_d=4$): Due to approximate nature of rigidity calculation methods, +/- 10% variation OK.

Wall 1 Defl. Check (in):	1.7	2.4
Wall 2 Defl. Check (in):	1.7	2.4
Wall 3 Defl. Check (in):		2.4
Wall 4 Defl. Check (in):		
Wall 5 Defl. Check (in):		
Wall 1 Drift Check (in):	OK	OK
Wall 2 Drift Check (in):	OK	OK
Wall 3 Drift Check (in):		OK
Wall 4 Drift Check (in):		
Wall 5 Drift Check (in):		

AREA VERIFICATION CHECK:

Seismic Trib Area Sum: 1082 sq ft

Trib Width Sum: 30 ft



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN: ROOF LEVEL

Shearwall Framing: 3/8" CDX ply with 8d Nailing

SHEAR LINE ID:	A	B
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DETERMINATION OF LATERAL DESIGN LOADS:

Grid Line Trib. Area (sq ft):	567	515
Seismic Shear Load (psf):	10.4	10.4
Seismic Shear Force (lb):	5906	5365

Grid Line Trib. Width (ft):	18	18
Wind Design Load (plf):	85	85
Wind Design Force (lb):	1493	1493

SHEARWALL LENGTHS:

Wall Length 1 (ft):	11.5	9.5
Wall Length 2 (ft):	0.0	0.0
Wall Length 3 (ft):	0.0	0.0
Wall Length 4 (ft):	0.0	0.0
Wall Length 5 (ft):	0.0	0.0

SHEARWALL RIGIDITY DESIGN:

Wall 1 Rigidity K_i (kip/in):	14.02	12.53
Wall 2 Rigidity K_i (kip/in):		
Wall 3 Rigidity K_i (kip/in):		
Wall 4 Rigidity K_i (kip/in):		
Wall 5 Rigidity K_i (kip/in):		

NAILING DESIGN BASED ON SDPWS SECTION 4.3.4 CAPACITY ADJ:

Wall 1 Design Shear (plf):	514	565
<i>Sec. 4.3.4.2 Capacity Reduction</i>	1.00	1.00
Wall 2 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 3 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 4 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		
Wall 5 Design Shear (plf):		
<i>Sec. 4.3.4.2 Capacity Reduction</i>		

Shear Capacity (plf):	600	600
-----------------------	-----	-----

Shear Designation #: Type 4 Type 4

REDUNDANCY FACTOR ANALYSIS (ASCE 7-16 SECTION 12.3.4):

Wall 1 Capacity (%):	0.45
Wall 2 Capacity (%):	
Wall 3 Capacity (%):	
Wall 4 Capacity (%):	
Wall 5 Capacity (%):	

Redundancy Factor: Use 1.0 Use 1.3



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SHEARWALL DESIGN (CONT.): ROOF LEVEL

SHEAR LINE ID:	A	B
-----------------------	----------	----------

SHEARWALL OVERTURNING DESIGN:

Wall Height (ft):	10.1	10.1
Wall Weight (psf):	17.0	17.0
Roof Trib. Width (ft):	0	0
Roof Weight (psf):	22.0	22.0

SEISMIC UPLIFT FORCE (lb): Load Combo (ASCE 7-16): (0.6-0.14*SDS)D+0.7E

Wall Dead Load (plf):	55	55
Wall 1 Uplift (lb):	4863	5434
Wall 2 Uplift (lb):		
Wall 3 Uplift (lb):		
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

WIND UPLIFT FORCE (lb): Load Combo (ASCE 7-16): 0.6D+0.6W

Wall Dead Load (plf):	103	103
Wall 1 Uplift (lb):	718	1096
Wall 2 Uplift (lb):		
Wall 3 Uplift (lb):		
Wall 4 Uplift (lb):		
Wall 5 Uplift (lb):		

HOLDOWN TYPE:

Wall 1 Holdown Type:	HDU5	HDU5
Wall 2 Holdown Type:		
Wall 3 Holdown Type:		
Wall 4 Holdown Type:		
Wall 5 Holdown Type:		

STORY DRIFT CHECK ($C_d=4$): Due to approximate nature of rigidity calculation methods, +/- 10% variation OK.

Wall 1 Defl. Check (in):	2.1	2.4
Wall 2 Defl. Check (in):		
Wall 3 Defl. Check (in):		
Wall 4 Defl. Check (in):		
Wall 5 Defl. Check (in):		
Wall 1 Drift Check (in):	OK	OK
Wall 1 Drift Check (in):		
Wall 1 Drift Check (in):		
Wall 1 Drift Check (in):		
Wall 5 Drift Check (in):		

AREA VERIFICATION CHECK:

Seismic Trib Area Sum: 1082 sq ft

Trib Width Sum: 35 ft



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

ROOF DIAPHRAGM DESIGN:

DIMENSIONS:

$$L = 35 \text{ ft}$$

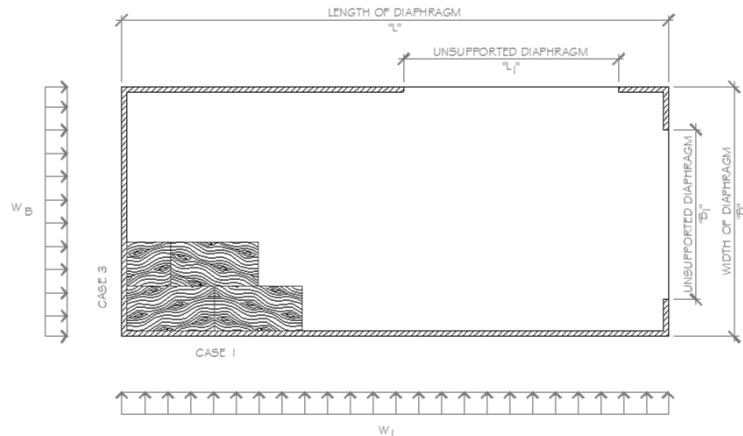
$$B = 30 \text{ ft}$$

$$B_1 = 0 \text{ ft}$$

$$L_1 = 0 \text{ ft}$$

FRAMING MATERIALS:

Plywood Panel Grade **CD, CC**
 Minimum Nominal Framing Size: **2 x**
 Nominal Plywood Thickness: **15/32**
 Common Nail Size: **8 d**
 Wall framing size: **2x4**
 Specific Gravity of Framing Members: **0.5**



LATERAL FORCE ALONG L SIDE:

$$W_L = 313 \text{ plf}$$

LATERAL FORCE ALONG B SIDE:

$$W_B = 280 \text{ plf}$$

ANALYSIS -

The diaphragm is considered flexible if its maximum lateral deformation is more than 2x the average shearwall deflection of the associated story.

Diaphragm Ratio: $L/B = 1.17 < 3$ *Diaphragm OK*

Shear_{MAX} along Side L: $V_L = W_B * B / 2 * L = 120 \text{ plf}$

Shear_{MAX} along Side B: $V_B = W_L * L / 2 * B = 182 \text{ plf}$

Chord Forces:

Side L: $T_L = C_L = W_L * L^2 / 8 * B = 1595 \text{ lb}$

**** Min. Blkg Req'd @ 96 in o.c.**

Side B: $T_B = C_B = W_B * B^2 / 8 * L = 901 \text{ lb}$

**** Min. Blkg Req'd @ 96 in o.c.**

Drag Forces:

Side L: $F_L = V_L * L_1 = 0 \text{ lb}$

**** Min. Strap Req'd - NONE**

Side B: $F_B = V_B * B_1 = 0 \text{ lb}$

**** Min. Strap Req'd - NONE**

DIAPHRAGM DEFLECTION -

** per APA: Supplement For Shearwall and Diaphragms*

$$D = \frac{5V_L^3}{8EAb} + \frac{V_L}{4Gt} + 0.188Le_n + \frac{S(D_cX)}{2b}$$

Chord Properties:

Area (in²): 5.25

$G_v t_v$ of ply (lb/in): 83500

E (psi): 1600000

e_n (in) = $(V_n/616)^{3.018}$: 0.045

ν (plf): 182

S (D_cX) (in): 2.50

$$D = 0.40 \text{ in}$$

$$D_{ALLOW} = 0.025 * \text{Story Height} = 3.8 \text{ in}$$

Deflection OK



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 45009

0

ROOF DIAPHRAGM DESIGN:

NAILING PATTERN -

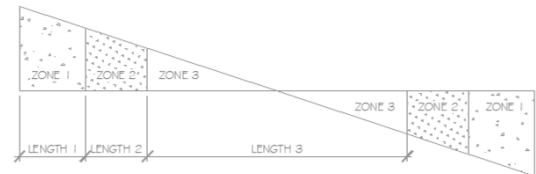
Panel Grade	Common Nail	Min. Pen. (in)	Min. Thick. (in)	Member Width (in)	Blocked Nail Spacing				Unblocked	
					Boundary / Other Edges					
					6 / 6	4 / 6	2.5 / 4	2 / 3	Case 1	Others
CD, CC	8 d	1 1/2	15/32	2	270	360	530	600	240	180

DIAPHRAGM NAILING DESIGN -

Along Length 'L'

Zone 1: 35 FT WIDE x 30 FT DEEP

UNBLOCKED 15/32 SHEATHING WITH 8 d COMMON NAILS
6.0 IN O.C. BOUNDARY/ 12 IN O.C. EDGES/ 12 IN O.C. FIELD

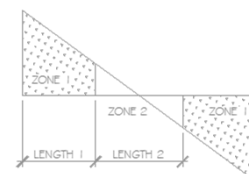
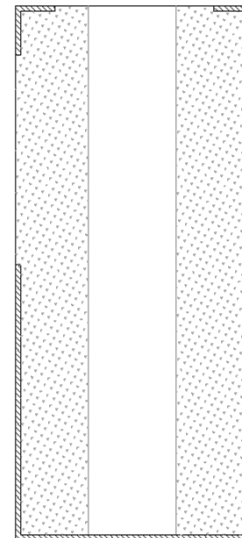


DIAPHRAGM NAILING DESIGN -

Along Length 'B'

Zone 1: 30 FT WIDE x 35 FT DEEP

UNBLOCKED 15/32 SHEATHING WITH 8 d COMMON NAILS
6.0 IN O.C. BOUNDARY/ 12 IN O.C. EDGES/ 12 IN O.C. FIELD



Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: Typical Header

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design

Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch

Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb + 1,350.0 psi

Fb - 1,350.0 psi

Fc - Prll 925.0 psi

Fc - Perp 625.0 psi

Fv 170.0 psi

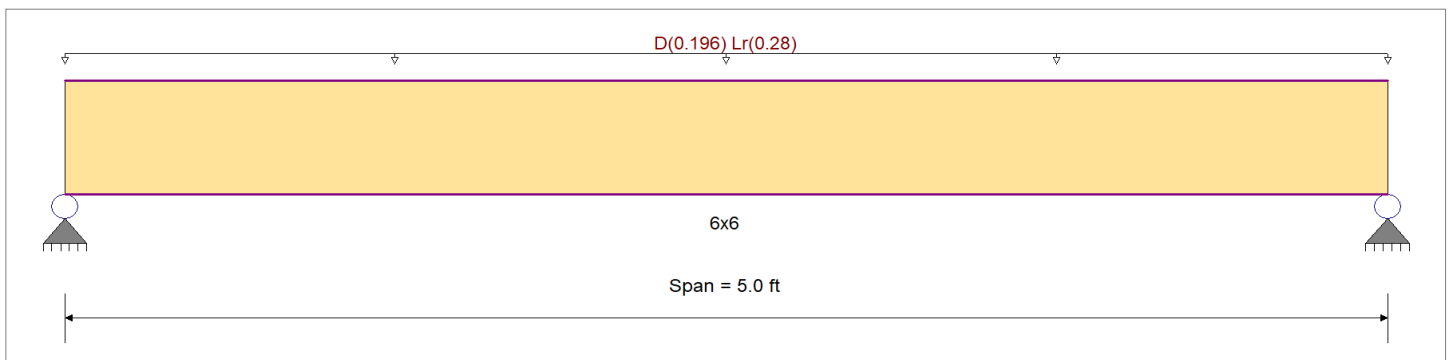
Ft 675.0 psi

E : Modulus of Elasticity

Ebend- xx 1,600.0ksi

Eminbend - xx 580.0ksi

Density 31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 14.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.387 : 1	Maximum Shear Stress Ratio	=	0.230 : 1
Section used for this span		6x6	Section used for this span		6x6
fb: Actual	=	652.59 psi	fv: Actual	=	48.90 psi
F'b	=	1,687.50 psi	F'v	=	212.50 psi
Load Combination		+D+Lr	Load Combination		+D+Lr
Location of maximum on span	=	2.500 ft	Location of maximum on span	=	0.000 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1
Maximum Deflection					
Max Downward Transient Deflection	0.032 in	Ratio = 1848 >=360	Span: 1 : Lr Only		
Max Upward Transient Deflection	0 in	Ratio = 0 <360	n/a		
Max Downward Total Deflection	0.056 in	Ratio = 1072 >=240	Span: 1 : +D+Lr		
Max Upward Total Deflection	0 in	Ratio = 0 <240	n/a		

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values				
		Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only																0.0	0.00	0.0	0.0
	Length = 5.0 ft	1	0.225	0.134	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.63	273.9	1,215.0	0.41	20.5	153.0	
+D+Lr						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.387	0.230	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.51	652.6	1,687.5	0.99	48.9	212.5	
+D+0.750Lr						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.331	0.197	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.29	557.9	1,687.5	0.84	41.8	212.5	
+0.60D						1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0	
	Length = 5.0 ft	1	0.076	0.045	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.38	164.4	2,160.0	0.25	12.3	272.0	

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: Typical Header

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0559	2.518		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	1.206	1.206
Max Upward from Load Combinations	1.206	1.206
Max Upward from Load Cases	0.700	0.700
D Only	0.506	0.506
+D+Lr	1.206	1.206
+D+0.750Lr	1.031	1.031
+0.60D	0.304	0.304
Lr Only	0.700	0.700

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC#: KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B1

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

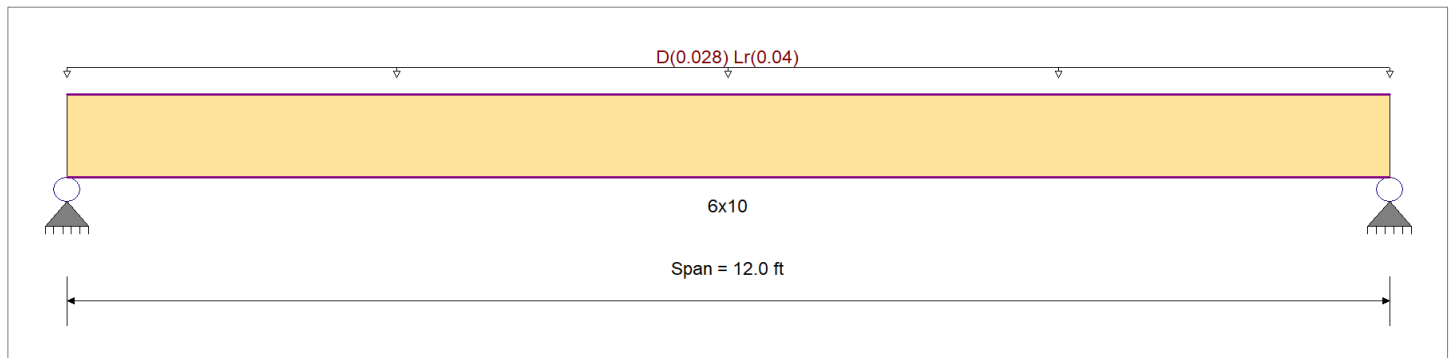
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb + 1,350.0 psi
Fb - 1,350.0 psi
Fc - Prll 925.0 psi
Fc - Perp 625.0 psi
Fv 170.0 psi
Ft 675.0 psi
E : Modulus of Elasticity
Ebend- xx 1,600.0ksi
Eminbend - xx 580.0ksi
Density 31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 2.0 ft

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.123	1	Maximum Shear Stress Ratio	=	0.056	1
Section used for this span		6x10		Section used for this span		6x10	
fb: Actual	=	207.11 psi		fv: Actual	=	11.87 psi	
F'b	=	1,687.50 psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	6.000 ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	
Maximum Deflection							
Max Downward Transient Deflection	0.030 in	Ratio =	4823 >=360	Span: 1 : Lr Only			
Max Upward Transient Deflection	0 in	Ratio =	0 <360	n/a			
Max Downward Total Deflection	0.059 in	Ratio =	2432 >=240	Span: 1 : +D+Lr			
Max Upward Total Deflection	0 in	Ratio =	0 <240	n/a			

Maximum Forces & Stresses for Load Combinations

Maximum Forces & Stresses of Load Combinations																		
Load Combination		Max Stress Ratios										Moment Values			Shear Values			
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v	
D Only															0.00	0.0	0.0	0.0
Length = 12.0 ft	1	0.085	0.038	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.71	102.7	1,215.0	0.20	5.9	153.0	
+D+Lr															0.0	0.00	0.0	0.0
Length = 12.0 ft	1	0.123	0.056	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.43	207.1	1,687.5	0.41	11.9	212.5	
+D+0.750Lr															0.0	0.00	0.0	0.0
Length = 12.0 ft	1	0.107	0.049	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.25	181.0	1,687.5	0.36	10.4	212.5	
+0.60D															0.0	0.00	0.0	0.0
Length = 12.0 ft	1	0.029	0.013	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.42	61.6	2,160.0	0.12	3.5	272.0	

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

(c) ENERCALC INC 1983-2022

DESCRIPTION: B1**Overall Maximum Deflections**

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0592	6.044		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	0.476	0.476
Max Upward from Load Combinations	0.476	0.476
Max Upward from Load Cases	0.240	0.240
D Only	0.236	0.236
+D+Lr	0.476	0.476
+D+0.750Lr	0.416	0.416
+0.60D	0.142	0.142
Lr Only	0.240	0.240

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B2

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

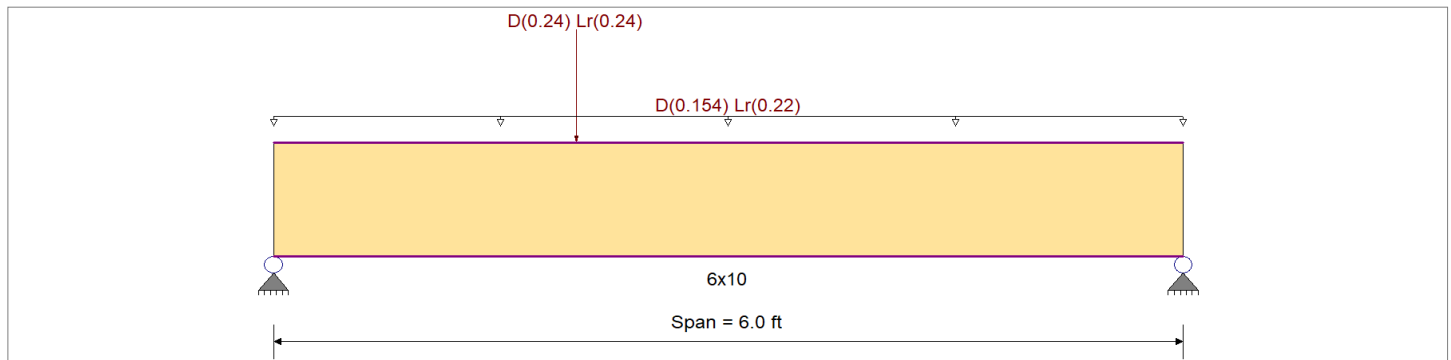
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	1,350.0 psi	E : Modulus of Elasticity	
Fb -	1,350.0 psi	Ebend- xx	1,600.0ksi
Fc - Prll	925.0 psi	Eminbend - xx	580.0ksi
Fc - Perp	625.0 psi		
Fv	170.0 psi		
Ft	675.0 psi	Density	31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 11.0 ft

Point Load : D = 0.240, Lr = 0.240 k @ 2.0 ft, (PL FROM B1)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.193	1	Maximum Shear Stress Ratio	=	0.158	1
Section used for this span		6x10		Section used for this span		6x10	
fb: Actual	=	325.96psi		fv: Actual	=	33.65 psi	
F'b	=	1,687.50psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	2.584ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.013 in	Ratio =	5619	>=360	Span: 1 : Lr Only
Max Upward Transient Deflection	0 in	Ratio =	0	<360	n/a
Max Downward Total Deflection	0.023 in	Ratio =	3120	>=240	Span: 1 : +D+Lr
Max Upward Total Deflection	0 in	Ratio =	0	<240	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values		
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only														0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.120	0.099	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.00	145.5	1,215.0	0.53	15.1	153.0
+D+Lr														0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.193	0.158	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	2.25	326.0	1,687.5	1.17	33.7	212.5
+D+0.750Lr														0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.166	0.137	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.94	280.8	1,687.5	1.01	29.0	212.5
+0.60D														0.0	0.00	0.0	0.0
Length = 6.0 ft	1	0.040	0.033	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	0.60	87.3	2,160.0	0.32	9.1	272.0

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B2**Overall Maximum Deflections**

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0231	2.956		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	1.476	1.316
Max Upward from Load Combinations	1.476	1.316
Max Upward from Load Cases	0.820	0.740
D Only	0.656	0.576
+D+Lr	1.476	1.316
+D+0.750Lr	1.271	1.131
+0.60D	0.394	0.346
Lr Only	0.820	0.740

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B3

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Load Combination Set : ASCE 7-16

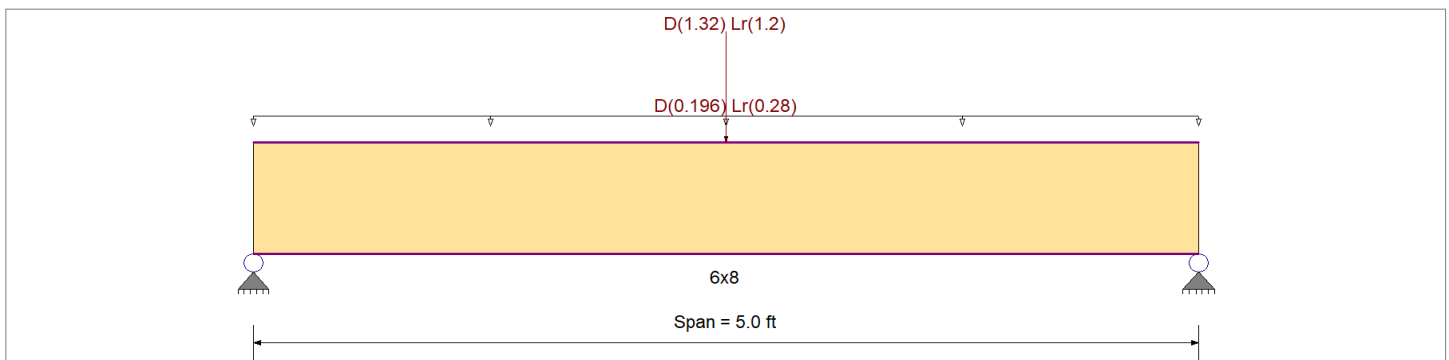
Material Properties

Analysis Method : Allowable Stress Design
Load Combination : ASCE 7-16

Wood Species : Douglas Fir-Larch
Wood Grade : No.1

Beam Bracing : Beam is Fully Braced against lateral-torsional buckling

Fb +	1,350.0 psi	E : Modulus of Elasticity	
Fb -	1,350.0 psi	Ebend- xx	1,600.0ksi
Fc - Prll	925.0 psi	Eminbend - xx	580.0ksi
Fc - Perp	625.0 psi		
Fv	170.0 psi		
Ft	675.0 psi	Density	31.210pcf



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loading

Uniform Load : D = 0.0140, Lr = 0.020 ksf, Tributary Width = 14.0 ft

Point Load : D = 1.320, Lr = 1.20 k @ 2.50 ft, (PL From G.T.)

DESIGN SUMMARY

Design OK

Maximum Bending Stress Ratio	=	0.643	1	Maximum Shear Stress Ratio	=	0.372	1
Section used for this span		6x8		Section used for this span		6x8	
fb: Actual	=	1,085.77 psi		fv: Actual	=	78.96 psi	
F'b	=	1,687.50 psi		F'v	=	212.50 psi	
Load Combination		+D+Lr		Load Combination		+D+Lr	
Location of maximum on span	=	2.500ft		Location of maximum on span	=	0.000 ft	
Span # where maximum occurs	=	Span # 1		Span # where maximum occurs	=	Span # 1	

Maximum Deflection

Max Downward Transient Deflection	0.030 in	Ratio =	1976	>=360	Span: 1 : Lr Only
Max Upward Transient Deflection	0 in	Ratio =	0	<360	n/a
Max Downward Total Deflection	0.059 in	Ratio =	1016	>=240	Span: 1 : +D+Lr
Max Upward Total Deflection	0 in	Ratio =	0	<240	n/a

Maximum Forces & Stresses for Load Combinations

Load Combination		Max Stress Ratios										Moment Values			Shear Values		
Segment Length	Span #	M	V	CD	CM	C _t	CLx	C _F	C _{fu}	C _i	C _r	M	fb	F'b	V	fv	F'v
D Only														0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.439	0.248	0.90	1.00	1.00	1.00	1.000	1.00	1.00	1.00	2.29	533.0	1,215.0	1.05	38.0	153.0
+D+Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.643	0.372	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	4.67	1,085.8	1,687.5	2.17	79.0	212.5
+D+0.750Lr					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.562	0.323	1.25	1.00	1.00	1.00	1.000	1.00	1.00	1.00	4.07	947.6	1,687.5	1.89	68.7	212.5
+0.60D					1.00	1.00	1.00	1.000	1.00	1.00	1.00			0.0	0.00	0.0	0.0
Length = 5.0 ft	1	0.148	0.084	1.60	1.00	1.00	1.00	1.000	1.00	1.00	1.00	1.37	319.8	2,160.0	0.63	22.8	272.0

Wood Beam

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: B3

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+Lr	1	0.0590	2.518		0.0000	0.000

Vertical Reactions

Support notation : Far left is #1

Values in KIPS

Load Combination	Support 1	Support 2
Max Upward from all Load Conditions	2.472	2.472
Max Upward from Load Combinations	2.472	2.472
Max Upward from Load Cases	1.300	1.300
D Only	1.172	1.172
+D+Lr	2.472	2.472
+D+0.750Lr	2.147	2.147
+0.60D	0.703	0.703
Lr Only	1.300	1.300

Wood Column

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: 6x6 POST

Code References

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Analysis Method	Allowable Stress Design			Wood Section Name	6x6			
End Fixities	Top & Bottom Pinned			Wood Grading/Manuf.	Graded Lumber			
Overall Column Height	9 ft			Wood Member Type	Sawn			
(Used for non-slender calculations)								
Wood Species	Douglas Fir-Larch			Exact Width	5.50 in	Allow Stress Modification Factors		
Wood Grade	No.2			Exact Depth	5.50 in	Cf or Cv for Bending	1.0	
Fb +	750.0 psi	Fv	170.0 psi	Area	30.250 in^2	Cf or Cv for Compression	1.0	
Fb -	750.0 psi	Ft	475.0 psi	Ix	76.255 in^4	Cf or Cv for Tension	1.0	
Fc - Prll	700.0 psi	Density	31.210 pcf	Iy	76.255 in^4	Cm : Wet Use Factor	1.0	
Fc - Perp	625.0 psi			Incising Factors :		Ct : Temperature Fact	1.0	
E : Modulus of Elasticity . . .	x-x Bending	y-y Bending	Axial	for Bending	0.80	Cfu : Flat Use Factor	1.0	
	Basic	1,300.0	1,300.0	for Elastic Modulus	0.95	Kf : Built-up columns	1.0 NDS 15.3.2	
	Minimum	470.0	470.0	Use Cr : Repetitive ?				No
Brace condition for deflection (buckling) along columns :								
				X-X (width) axis :	Unbraced Length for buckling ABOUT Y-Y Axis = 9 ft, K			
				Y-Y (depth) axis :	Unbraced Length for buckling ABOUT X-X Axis = 9 ft, K			

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 59.006 lbs * Dead Load Factor

AXIAL LOADS . . .

Axial Load at 9.0 ft, D = 0.660, Lr = 0.820 k

DESIGN SUMMARY

Bending & Shear Check Results

PASS Max. Axial+Bending Stress Ratio =	0.09102 : 1	Maximum SERVICE Lateral Load Reactions . .	
Load Combination	+D+Lr	Top along Y-Y	0.0 k
Governing NDS Formula	Comp Only, f_c/F_c'	Bottom along Y-Y	0.0 k
Location of max.above base	0.0 ft	Top along X-X	0.0 k
At maximum location values are .		Bottom along X-X	0.0 k
Applied Axial	1.539 k	Maximum SERVICE Load Lateral Deflections . . .	
Applied Mx	0.0 k-ft	Along Y-Y	0.0 in at 0.0 ft above base
Applied My	0.0 k-ft	for load combination : n/a	
Fc : Allowable	558.95 psi	Along X-X	0.0 in at 0.0 ft above base
		for load combination : n/a	
PASS Maximum Shear Stress Ratio =	0.0 : 1	Other Factors used to calculate allowable stresses . . .	
Load Combination	+0.60D	Bending	Compression
Location of max.above base	9.0 ft	Tension	
Applied Design Shear	0.0 psi		
Allowable Shear	217.60 psi		

Load Combination Results

Load Combination	C _D	C _P	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
D Only	0.900	0.866	0.05444	PASS	0.0 ft	0.0	PASS	9.0 ft
+D+Lr	1.250	0.799	0.09102	PASS	0.0 ft	0.0	PASS	9.0 ft
+D+0.750Lr	1.250	0.799	0.07890	PASS	0.0 ft	0.0	PASS	9.0 ft
+0.60D	1.600	0.728	0.02186	PASS	0.0 ft	0.0	PASS	9.0 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		Y-Y Axis Reaction		Axial Reaction	My - End Moments		Mx - End Moments	
	@ Base	@ Top	@ Base	@ Top		@ Base	@ Top	@ Base	@ Top
D Only					0.719				
+D+Lr					1.539				
+D+0.750Lr					1.334				

Wood Column

Project File: W021420 - Town of Danville ADU_Enercalc (1-Bd).ec6

LIC# : KW-06015505, Build:20.23.2.14

DP ADVANCED ENGINEERING INC.

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DESCRIPTION: 6x6 POST

Maximum Reactions

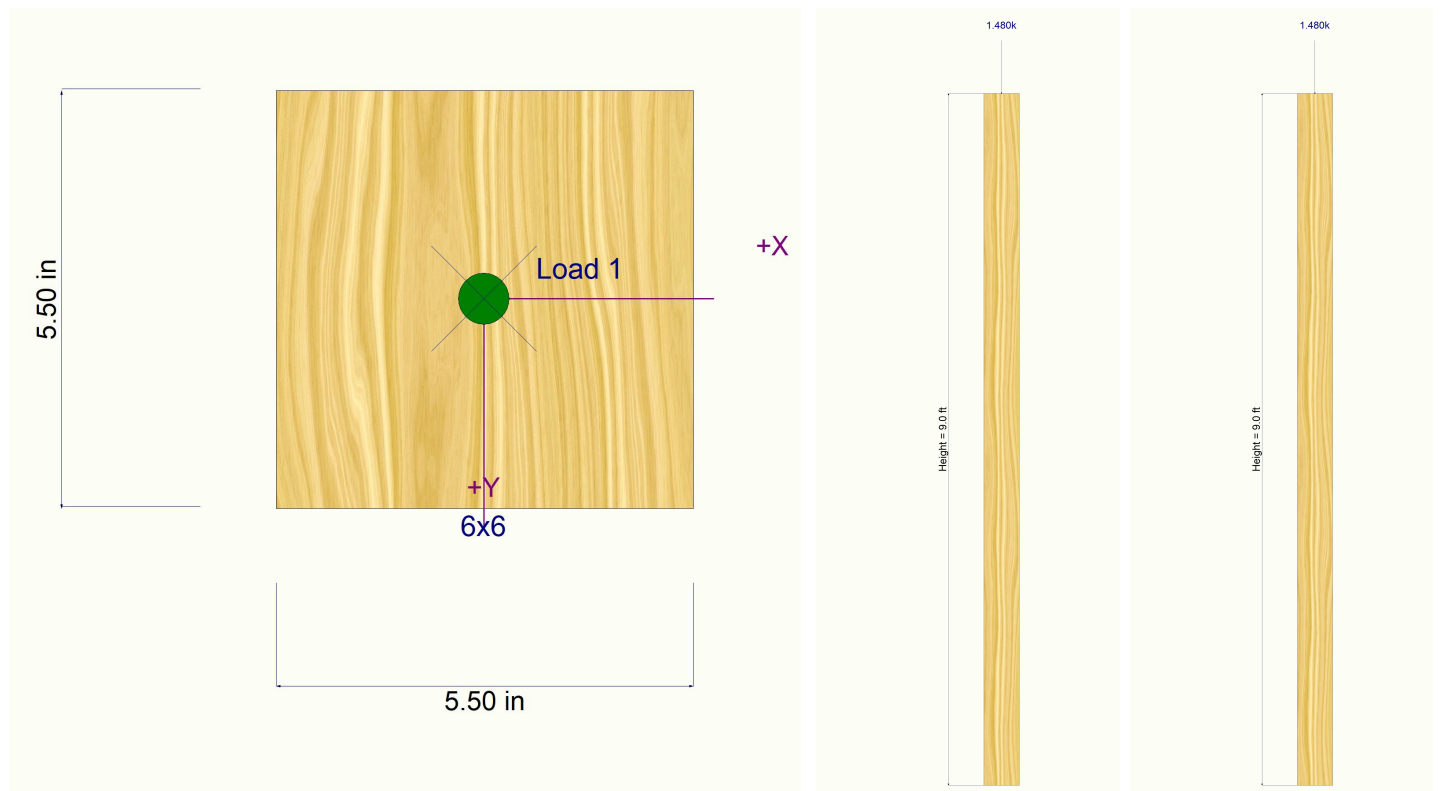
Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		k	Y-Y Axis Reaction		Axial Reaction	My - End Moments		k-ft	Mx - End Moments	
	@ Base	@ Top		@ Base	@ Top	@ Base	@ Base	@ Top		@ Base	@ Top
+0.60D						0.431					
Lr Only						0.820					

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
D Only	0.0000 in	0.000ft	0.000 in	0.000 ft
+D+Lr	0.0000 in	0.000ft	0.000 in	0.000 ft
+D+0.750Lr	0.0000 in	0.000ft	0.000 in	0.000 ft
+0.60D	0.0000 in	0.000ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000ft	0.000 in	0.000 ft

Sketches





PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

SPREAD FOOTING FOUNDATION ANALYSIS:

Allowable Stress:

fy (ksi): 60
f'c (psi): 2500

Footing Reinf:

(2) #4 in Top
(2) #4 in Bottom

As used: 0.4

<i>Grid Line:</i>	<i>1</i>	<i>2</i>	<i>A</i>	<i>B</i>
<u>Footing Design Criteria:</u>				
Allow. Bearing Pres. (psf):	1500	1500	1500	1500
Footing Width (in):	12	12	12	12
Footing Depth (in):	18	18	18	18
Unsupported Length (ft):	5.0	5.0	5.0	5.0
<u>Uniform Loads:</u>				
Roof Trib (ft)	13.0	13.0	5.0	5.0
Roof DL (psf)	22.0	22.0	22.0	22.0
Roof LL (psf)	20.0	20.0	20.0	20.0
Floor Trib (ft)	0.0	0.0	0.0	0.0
Floor DL (psf)	15.0	15.0	15.0	15.0
Floor LL (psf)	40.0	40.0	40.0	40.0
Lower Floor Trib (ft)	0.0	0.0	0.0	0.0
Lower Floor DL (psf)	15.0	15.0	15.0	15.0
Lower Floor LL (psf)	40.0	40.0	40.0	40.0
Deck Trib (ft)	0.0	0.0	0.0	0.0
Deck DL (psf)	15.0	15.0	15.0	15.0
Deck LL (psf)	60.0	60.0	60.0	60.0
Wall Trib (ft)	10.0	10.0	10.0	10.0
Wall DL (psf)	17.0	17.0	17.0	17.0
TOTAL DL (plf):	456	456	280	280
TOTAL Lr (plf):	260	260	100	100
TOTAL LL (plf):	0	0	0	0
<u>Point Load:</u>				
Max. Point DL (lbs)	1320	1320	0	0
Max. Point Lr (lbs)	1200	1200	0	0
Max. Point LL (lbs)	0	0	0	0
Max. HD Load (lbs)	2153	3415	4863	5434
<u>Soil Analysis:</u>				
q (psf):	1331	1331	851	951
q/ q _{ALLOW} :	0.89	0.89	0.57	0.63
M_{max} (ft-lb):	7391 ft-lb	7391 ft-lb	5360 ft-lb	5897 ft-lb
V_{max} (lb):	4161 lbs	4161 lbs	2460 lbs	2675 lbs
Allowable M_{max} (ft-lb):	25253 ft-lb	25253 ft-lb	25253 ft-lb	25253 ft-lb
Allowable V_{max} (lb):	7395 lbs	7395 lbs	7395 lbs	7395 lbs
Footing A(s) req'd:	0.40	0.40	0.40	0.40
	O.K.	O.K.	O.K.	O.K.



PROJECT: Town of Danville 1-Bedroom ADU

CLIENT: Town of Danville

JOB NO.: W021420

CALCS BY: J. Peek

DATE: 3/24/2023

PIER AND GRADE BEAM FOUNDATION ANALYSIS:

Allowable Stress:

fy (ksi): 60
f'c (psi): 2500

Grade Beam Reinf:

(3) #5 in Top
(3) #5 in Bottom

As used: 0.93

Grid Line:	1	2	A	B
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Pier Design Criteria:

Allow. Skin Friction (psf):	250	250	250	250
Pier Diameter (in):	16	16	16	16
Neglect Pier Depth (ft):	2	2	2	2

Grade Beam Design Criteria:

Grade Beam Width (in):	12	12	12	12
Grade Beam Depth (in):	18	18	18	18
Grade Beam Span (ft):	8.0	8.0	8.0	8.0

Uniform Loads:

Roof Trib (ft)	13.0	13.0	5.0	5.0
Roof DL (psf)	22.0	22.0	22.0	22.0
Roof LL (psf)	20.0	20.0	20.0	20.0
Floor Trib (ft)	0.0	0.0	0.0	0.0
Floor DL (psf)	15.0	15.0	15.0	15.0
Floor LL (psf)	40.0	40.0	40.0	40.0
Lower Floor Trib (ft)	0.0	0.0	0.0	0.0
Lower Floor DL (psf)	15.0	15.0	15.0	15.0
Lower Floor LL (psf)	40.0	40.0	40.0	40.0
Deck Trib (ft)	0.0	0.0	0.0	0.0
Deck DL (psf)	15.0	15.0	15.0	15.0
Deck LL (psf)	60.0	60.0	60.0	60.0
Wall Trib (ft)	10.0	10.0	10.0	10.0
Wall DL (psf)	17.0	17.0	17.0	17.0
TOTAL DL (plf):	681	681	505	505
TOTAL Lr (plf):	260	260	100	100
TOTAL LL (plf):	0	0	0	0

Point Load:

Max. Point DL (lbs)	1320	1320	0	0
Max. Point Lr (lbs)	1200	1200	0	0
Max. Point LL (lbs)	0	0	0	0
Max. HD Load (lbs)	2153	3415	4863	5434

Grade Beam Analysis:

M_{max} (ft-lb):	16877 ft-lb	16877 ft-lb	10959 ft-lb	11817 ft-lb
V_{max} (lb):	6686 lbs	6686 lbs	3651 lbs	3866 lbs
Allowable M_{max} (ft-lb):	56104 ft-lb	56104 ft-lb	56104 ft-lb	56104 ft-lb
Allowable V_{max} (lb):	7395 lbs	7395 lbs	7395 lbs	7395 lbs

Grade Beam A(s) req'd:	0.93	0.93	0.93	0.93
	O.K.	O.K.	O.K.	O.K.

Pier Analysis:

Design Pier Load (lbs):	10051	10051	5598	5898
Req'd Pier Depth (ft)	11.60	11.60	7.35	7.64
Use Pier Depth (ft):	12	12	8	8